

BLD 2012-1408  
BTR - 2014-0074  
33-5023-040-0140



VILLAGE OF PALMETTO BAY  
9705 E. HIBISCUS STREET  
PALMETTO BAY, FLORIDA 33157  
- (305) 259-1234 Fax: (305) 259-1290

Approved  
Edward Silva, Village Manager  
Date

REQUEST FOR PUBLIC RECORDS

Requests are filled in accordance with the provisions of Chapters 119 and 257, Florida Statutes.

DATE: 12-3-18  
NAME: Eduardo Rodriguez  
COMPANY: Cutler Bay Solar Solutions  
ADDRESS: 641 NW 132nd Place  
PHONE: 786-556-5574 FAX: \_\_\_\_\_  
EMAIL: edusolar designs@gmail.com

REQUEST (Attach additional page, if necessary): Copies of the following documents:

Copy of structural roof plans  
for property 6745 SW 139th St,  
Palmetto Bay, FL, 33158. Should  
show roof truss locations and  
directions.

\*\*\*\*\*

FOR USE BY VILLAGE STAFF ONLY

TRACKING NO.: 2018-394

DATE FORWARDED: 12/5/18

ASSIGNED DEPT: \_\_\_\_\_

DATE REQUEST FILLED: 12/7

NUMBER OF COPIES: 33

ESTIMATED TIME (IF APPLICABLE): \_\_\_\_\_

ESTIMATED COST: \_\_\_\_\_

HOW WAS REQUEST FILLED? \_\_\_\_\_

IF NOT FILLED, REASON: \_\_\_\_\_

BY: [Signature]

**Melissa Dodge**

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**From:** noreply@civicplus.com  
**Sent:** Monday, December 03, 2018 6:14 PM  
**To:** Missy Arocha; Melissa Dodge  
**Subject:** Online Form Submittal: Public Records Requests

Public Records Requests

PUBLIC RECORDS  
REQUEST FORM

[Public Records Policy](#)

Date	12/3/2018
Name	Eduardo Rodriguez
Company	Cutler Bay Solar Solutions
Phone Number	7865565574
Email Address	<a href="mailto:edusolardesigns@gmail.com">edusolardesigns@gmail.com</a>
Address	641 NW 132nd PL
Preferred Delivery Method	Electronic (via email- email address required)
Are you a member of the media?	No
Public Record Request(s):	I would like to have the Structural roof plans of the following property: 6745 SW 139th St Palmetto Bay, FL 33158 It should show the roof truss locations and directions

Thank You

*Please note that there may be a fee associated with your request depending on the extent of the information being requested. If so, the Office of the Village Clerk will contact you about any associated fees. If you have any further questions, please email Village Clerk Arocha at [marocha@palmettobay-fl.gov](mailto:marocha@palmettobay-fl.gov)*

Email not displaying correctly? [View it in your browser.](#)



(27)

**PLANS PROCESSING**

Process Number: BID-20121468

Description of Work: Anchor Yarns

Property Address: 6745 SW 139 St

2010 FLORIDA BUILDING  
MARCH 10, 2012

Department	Date In	Date Out	Total Business Days	Approved	Denied	Comments
Zoning						
Building	5/3/12	5/7/12	2	✓		✓
Structural	5-8-12 5-15-12	5/8/12 5-15-12	1	✓	✓	✓ * CEDEY RUN !
Electrical						
Mechanical						
Plumbing						
Planning & Zoning						
Public Works						
ADA Review						
Final Signature	5-22-12	5-22-12	1	✓		
Code Enforcement Verification	5-22-12	5-22-12	1	✓		
Pricing	5-22-12					
Call for Pick up						

Permit #: BLD-2012-1468  
 Master permit #: BRF-2012-0516

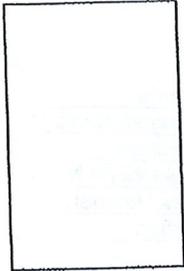
Permit type: bmis - MISCELLANEOUS REPAIR RE: ADDRESS: 6745 SW 139 ST  
 Routing queue: b7 - FLAT ROOF /LOW SLOPE PALMETTO BAY, FL 33157

Group # - Name	Action Code	Action Description	Completion Date	Completion Code	Completed By	Comments
1 - PERMIT COUNTER	intake	APPLICATION INTAKE	5/3/2012		jvillegas	
1 - PERMIT COUNTER	cuff	COLLECT UPFRONT FEE	5/3/2012		jvillegas	
2 - PLANS PROCESSING	routeplans	ROUTE PLANS FOR REVIEW	5/3/2012		jvillegas	
3 - BUILDING REVIEW	brev	BUILDING REVIEW	5/8/2012	apprve	aramos	
3 - BUILDING REVIEW	srev	STRUCTURAL REVIEW	5/9/2012	denied	mcankat	PLEASE PROVIDE MORE DETAILED INFORMATION SUCH AS A) TRUSS SPAN B) CALCULATED UPLIFT & HORIZONTAL FORCES AT THE CONNECTION BASED ON WIND DESIGN AT THE TIME OF CONSTRUCTED C) ORIGINAL STRAP W/ 2-3 NAILS. PLEASE IDENTIFY IT W/ APPROX. UPLIFT CAPACITY. AND IF THERE IS ANY HORIZONTAL RESISTANCE CAPACITY D) ATTACH COPY OF THE SIMPSON HGAM 10 (PRODUCT APPROVAL) COMPARE TOTAL RESISTING CAPACITY AGAINST REQUIRED.
3 - BUILDING REVIEW	srev	STRUCTURAL REVIEW	5/22/2012	apprve	mcankat	
4 - PLANS PROCESSING	compcomm	COMPILE COMMENTS	5/9/2012		djeune	
5 - PERMIT COUNTER	verifycont	VERIFY CONTRACTOR	5/23/2012	apprve	jvillegas	
6 - PLANS PROCESSING	notifycust	NOTIFY CUSTOMER	5/23/2012		jvillegas	
7 - PERMIT COUNTER	collissue	COLLECT FEES/ISSUE PERMIT	5/23/2012	compl	jvillegas	
8 - INSPECTIONS	100	SETBACK	6/28/2012		aramos	
8 - INSPECTIONS	700	NOC	6/28/2012		aramos	
8 - INSPECTIONS	701	FOUNDATION	6/28/2012		aramos	
8 - INSPECTIONS	707	SLAB 1 ST FLOOR	6/28/2012		aramos	
8 - INSPECTIONS	727	FRAMING	6/28/2012		aramos	
8 - INSPECTIONS	788	ALUMIN ROOF INSTALLATION	6/28/2012		aramos	
8 - INSPECTIONS	121	FINAL ZONING	6/28/2012		aramos	
8 - INSPECTIONS	752	ACCESSIBILITY	6/28/2012		aramos	
8 - INSPECTIONS	758	FINAL STRUCTURAL BUILDING	6/28/2012	apprve	aramos	OK - SUBMITTED P.E. LETTER - A. RAMOS



# Village of Palmetto Bay Permit Application

Department of Building & Capital Projects  
 9705 E. Hibiscus Street  
 Palmetto Bay, Florida 33157  
 Phone: (305) 259-1250 Fax: (305) 259-1291 Inspections: (305) 259-1253



**GENERAL INFORMATION:** Please read these instructions carefully before submitting the work for review.

This application must be completed and signed by both the property owner and qualifier. Both of these signatures must be notarized. Please print legibly or type in order not to delay your application. For roofing permits, in addition to this permit you must also fill out a roofing permit application. Express permits require an additional fee and will only be accepted between the hours of 8:00 A.M. and 10:00 A.M., Monday through Friday. All other permits/plans must be dropped off before 4:30 p.m. for regular processing. During the processing of your application you may be asked to submit additional information. There may be additional permits and reviews required from other governmental agencies not affiliated with Palmetto Bay.

**APPLICATION:**

<b>Clerk's Initials</b> EJ	<b>Plan Process Number</b> BLD-2012-1468	<b>Master Permit Number</b> BRF-2012 0516	<b>Subsidiary Permit Number(s)</b>	<b>Expiration Date</b>
<b>Job Address:</b> 6745 SW 139 St	Address	Unit number	City Palmetto Bay	State FL
			Zip Code 33158	

<b>Folio Number:</b> 3350230400140	<b>Linear Feet:</b> 400	<b>Units:</b>	<b>Stories:</b> 1
<b>Lot:</b>	<b>Block:</b>	<b>Value of Proposed Work:</b> 7,500	<b>Est. Bldg. Value:</b> <del>7,500</del> 543,533
<b>Subdivision:</b>	<b>PB:</b>	<b>Tax Assessed Value:</b>	
<b>Current Use of Property:</b> Residence	<b>PG:</b>	<b>Flood Zone:</b>	<b>Base Floor Elev.:</b>
<b>Proposed Use of Property:</b> Residence		<b>Homeowner's Association:</b>	
<b>Description of Work:</b> ANCHOR TRUSSES	<i>I affirm that there <input type="checkbox"/> are or <input type="checkbox"/> are no restrictive covenants associated with the underlying property that would affect the pending application. Failure to disclose this information shall result in the immediate revocation of any type of permit or certificate of use/occupancy.</i>		
<b>Zoning:</b>	<b>Square Feet:</b>		
<b>Tenant Information:</b>	<b>Unit Number:</b>		

Check Permit Type		Check Permit Change		Check Type of Improvement			
Building	X	Change of Contractor		New Construction		Deck/Concrete flatwork	
Electrical		Permit Renewal		Exterior Alteration		Window Replacement	
Mechanical		Plan Revision		Interior Alteration		Shutters	
Plumbing		Permit Extension		Attached Addition		Garage Doors	
LPGX		Supplement		Detached Addition		Storage Shed	
Roofing		Re-inspection Fee		Repair		Railings	
Fence				Repair due to Fire		Stairs	
Sign				Demolish		Windows/Doors	
Public Works				Screen Enclosure		Roofing	
Other				Driveway		Re-Roof	
				Fence		Seal-cote	
				Pool		Other	

5/23/12

Architect Information	Engineer Information
Name:	Name:
License Number:	License Number:
Address:	Address:
Telephone Number:	Telephone Number:
Fax Number:	Fax Number:
Other:	Other:

Property Owner	Contractor
Name: <u>Greg Herskowitz</u>	Company Name: <u>Gables Generic Construction</u>
Address:	Qualifer: <u>Leonard M. Rioseco</u>
Home Telephone:	License Number: <u>CGC 1520279</u>
Business Telephone:	Address:
Other Telephone:	Telephone Number:
Fax Number:	Fax Number:
Does Property have Homestead Exemption	Phone Number for Pick Up: <u>305 244 5588</u> <u>-Ken</u>

Bond Company (if applicable)	Mortgage Lender
Name:	Name:
Address:	Address:
City: State: Zip	City: State: Zip

Classification Of Proposed Work		
Residential <input type="checkbox"/>	Duplex <input type="checkbox"/>	Townhouse <input type="checkbox"/> Multi Family <input type="checkbox"/> Commercial <input type="checkbox"/> Industrial <input type="checkbox"/> Other <input type="checkbox"/>
Effective Code _____	Occupancy _____	Construction Type _____
Zoning:	Variance Number:	Remarks:

**OWNER AFFIDAVIT**

Application is hereby made to obtain a permit to do the work and installation as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work will be performed to meet the standards of all laws regulating construction in this jurisdiction. I understand that a separate permit must be secured for ELECTRICAL, MECHANICAL, PLUMBING, SIGNS, WELLS, POOLS, ROOFING, SHUTTERS, WINDOWS, FURNACES, BOILERS, HEATERS, TANKS, FENCE, DRIVEWAYS and AIR CONDITIONERS, ETC. In addition to the requirements of this permit, there may be additional restrictions found in the Public Records, and there may be additional permits required from other governmental entities.

I, the owner of the property, have disclosed all information related to any work that has been performed in the prior twelve months to the Building Division as part of this application. Further, I am fully aware that if the cumulative cost of work to my home or business under this and any other permit equals or exceeds fifty percent of the fair market value of the structure, the entire structure must meet the present federal flood criteria for finished floor elevation. I am also fully aware that if the cost of work to my home or business under this and any other permit equals or exceeds fifty per cent of the replacement cost of the structure, then the entire structure must conform to current code requirements of the Florida Building Code.

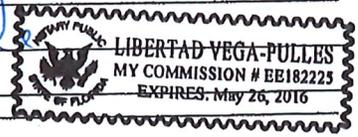
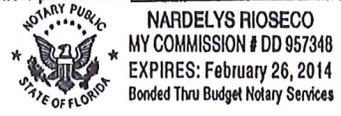
I, the owner of the property, understand that a permit application is subject to denial and a validated permit or permit card is subject to revocation or modification based upon applicable deeds, covenants, declarations, easements and any other legal restriction. By issuing a permit, the Village of Palmetto Bay makes no representation as to the existence or validity of any property restriction.

I, the owner of the property certify that all the foregoing information is accurate and that all work will be done in compliance with all applicable laws regulating construction and zoning.

**WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.**

Signature of Owner: GREG HERSKOWITZ  
 State of Florida, County of Miami Dade  
 Sworn to (or affirmed) and subscribed before this 27<sup>th</sup> day of April, 2012, by (print name) Nardelys Rioseco  
 Notary Name Nardelys Rioseco  
 Personally known  or I.D. \_\_\_\_\_  
 Type of Identification produced: \_\_\_\_\_

Signature of Qualifier: [Signature]  
 State of Florida, County of Miami Dade  
 Sworn to (or affirmed) and subscribed before this 0 day of May, 2012, by (print name) \_\_\_\_\_  
 Notary Name [Signature]  
 Personally known or I.D. \_\_\_\_\_  
 Type of Identification produced: \_\_\_\_\_



**IMPORTANT NOTICES**

- Do not begin work without receiving you validated permit and permit card. Applying for a permit does not grant you the right to commence construction. Construction can only occur during the hours of 7:00 a.m. to 7:00 p.m., Monday thru Saturday and from the hours of 9:00 a.m. to 6:00 p.m. on Sundays and Federal holidays.
- All construction sites must be maintained in a clean and orderly condition free from construction debris. Failure to do so will result in a fine and a suspension of inspections until said property is cleared.
- Streets and neighboring properties shall be kept free from dirt and debris.
- Swales must be protected from damage by equipment or vehicles and sidewalks cannot be blocked.
- Portable Toilets for construction jobs require a separate permit. If toilet is not available the inspection will not be performed.
- Water cannot be discharged into the right of way or storm drains without the approval of the Public Works Department.
- No equipment or materials can be stored on the right of way; they must only be stored on your property.
- Florida Department of Health approval is required for applications involving Septic Tanks. Department of Environmental Resources Management (DERM) and /or Miami-Dade Water and Sewer Department approval is required for applications involving sewers and water. The tree section of the Department of Environmental Resources Management (DERM) approval is required on all landscape plans and on all tree removal applications.

**CHECKLIST (OFFICE USE ONLY)**

- |   |   |   |
|---|---|---|
| <input type="checkbox"/> OWNER-BUILDER FORM<br>(Attached)                             | <input type="checkbox"/> HEALTH DEPARTMENT APPROVAL<br>(Septic/Sewer)             | <input checked="" type="checkbox"/> PERMIT CLERK REVIEW<br>By: <u>ef</u>  |
| <input type="checkbox"/> FIRE DEPARTMENT APPROVAL<br>(Commercial/multi (family only)) | <input type="checkbox"/> IMPACT FEE<br>(New Construction)                         | <input checked="" type="checkbox"/> Complete Application  |
| <input type="checkbox"/> CONCURRENCY<br>(New Construction)                            | <input type="checkbox"/> SCHOOL REVIEW<br>(New Construction)                      | <input type="checkbox"/> Current liability ins.   |
| <input type="checkbox"/> PROOF OF OWNERSHIP<br>(Attached)                             | <input type="checkbox"/> DERM REVIEW<br>(New Construction/Additions/Tree Removal) | <input type="checkbox"/> Worker's Comp.   |
| <input type="checkbox"/> CONDO ASSOCIATION APPROVAL                                   | <input type="checkbox"/> PUBLIC WORKS   | <input type="checkbox"/> Cont. Lic. Check   |
| <input type="checkbox"/> UPFRONT FEES AMOUNT: _____                                   |   | <input type="checkbox"/> OTHER<br>(Specify and Attach)  |
|   |   | <input type="checkbox"/> FLORIDA DEPARTMENT OF<br>BUSINESS AND<br>PROFESSIONAL REGULATION<br>APPROVAL (RESTAURANTS) |

**PERMIT FEES (OFFICE USE ONLY)**

Scanning Fees Small (1.85 per sheet)	27.75	Art in Public Places	
Scanning Fees Large (3.50 per sheet)		Certificate of Use and Occupancy	
Village of Palmetto Bay Permit Fees	115.50	Concurrency Fee (7.35%)	
Miami-Dade County Fees (sq. ft. x \$65//1000x0.60)		Technology Fee (6.3%)	
Flood Zone Review		Zoning Inspection Fee (157.50 per application)	
Radon-Inspector State Educational Fund and DCA State fee	4.00	Administration Fee	
Code Enforcement Fine		Express Fee ( 25.00)	
Certificate of Completion		Public Works Fee	
Construction Sign Fee		Landscape Review Fee (175.00 per hour)	
Roll-off Waste Container Fee (105.00 per container site)		Special Review Fee (89.25 per hour)	
Rework Fee		Other	

\$ 147.25

**PLAN REVIEWER APPROVAL AREA (OFFICE USE ONLY)**

SECTION	REVIEWER APPROVAL'S NAME	APPROVAL DATE	REJECTED DATE NUMBER 1	REJECTED DATE NUMBER 2	REJECTED DATE NUMBER 3
COMMUNITY DEVELOPMENT					
ZONING					
ELECTRICAL					
MECHANICAL					
PLUMBING					
FIRE					
ROOFING					
PUBLIC WORKS					
PLANNING					
STRUCTURAL	<i>mc</i>	<i>05/22/2012</i>	<i>05/08/12</i>	<i>mc</i>	
BUILDING	<i>mc</i>	<i>5/2/12</i>			
BUILDING OFFICIAL					

**REWORKS:** A fee of \$105.50 may be charged for failure to make required corrections previously indicated. The fee shall be charged after the initial review plus one follow up review per trade. Please note that Florida Statutes 553.80 section 2(b) states "with respect to evaluation of design professionals documents, if a local government finds it necessary, in order to enforce compliance with the Florida Building Code and issue a permit, to reject design documents required by the code three or more times for failure to correct a code violation specifically noted in each rejection, including but not limited to egress, fire protection, structural stability, energy, accessibility, lighting, ventilation, electrical, mechanical, plumbing, and gas systems, or other requirements identified by rule of the Florida Building Commission adopted pursuant to Chapter 120, the local government shall impose, each time after third such review the plans are rejected for that code violation a fee of four times the amount of the proportion of the permit fee attributed to plans review".

**ISSUING OFFICIAL**

FINAL PLAN REVIEWED AND PREPARED FOR ISSUANCE BY: \_\_\_\_\_ DATE: \_\_\_\_\_

**CONDITIONS OF APPROVAL**

**PLAN TRACKING**

Plans Checked out	Date	Clerk	Plans Checked in	Date	Clerk
<i>[Signature]</i>	<i>5/10/12</i>	<i>[Signature]</i>			



Palmetto Bay Building Department  
Comment Sheet

Department: STRUCTURAL

Inspector: MUSTAFA CANKAT

Date: 05/08/2012

PLEASE PROVIDE MORE DETAILED INFORMATION SUCH AS

A) TRUSS SPAN

B) CALCULATED UPLIFT & HORIZONTAL FORCES AT THE CONNECTION BASED ON WIND DESIGN AT THE TIME OF CONSTRUCTED

C) ORIGINAL STRAP W/ 2-3 NAILS. PLEASE IDENTIFY IT W/ APPROXIMATE UPLIFT CAPACITY, AND IF THERE IS ANY HORIZONTAL ~~STR~~ RESISTANCE CAPACITY.

D) ATTACH COPY OF THE SIMPSON HGAM 10 (PRODUCT APPROVAL) COMPARE TOTAL RESISTING CAPACITY ~~AND~~ AGAINST REQUIRED.



**Florida International Engineering and Testing Lab LLC**

16701SW 117 Avenue, Miami, FL 33177

Telephone: (305) 378-1991-Fax: (305) 378-1997

Miami-Dade Lab Certification # 07-0612.11-State of Florida ca #27273

May 22, 2010

Village of Palmetto Bay  
Building & Zoning Dept.  
9705 E Hibiscus Dr.  
Palmetto Bay, FL 33157

Re: ROOF RETROFIT. Section 553.844 Florida Statutes.

Address: 6745 SW 139<sup>th</sup> St., Palmetto Bay, FL.

Areas inspected:

Attic was visited by means of ceiling hatch in garage and observations were made thru destructive openings at corners of two roof areas.

Observations:

Type of Roof: Hipped.

Sheathing: plywood-5/8" CDX

Framing: 2x8 Wood rafter/trusses- 24" o.c.

Existing truss anchoring to masonry wall with 2-3 nails on one side of wood member to be supplemented by providing additional 10d nails thru strap hole opening for a total of 4 nails to side and Simpson gussets (right angle) Type HGAM 10 with 4-3-1/2"x1/4" tap cons into tie beam and 4- 1-1/2" wood screws as provided by Simpson HGAM 10 kit at opposite side of wood member for a minimum uplift capacity of 500 lbs +, as mandated by roof-to-wall retrofit requirements.

Priority areas, if required, shall be corners and longest span trusses.

These results are based on observations of a limited scope pursuant to the requirements of Section 553.844 Florida Statutes sub-section 101 (roof to wall connections), as conditions may allow, and prescriptive methods for compliance as per sub-section 201.3 shall be used to limit the scope of work to the 15% limit. Further analysis intended to analyze the transfer of loads, metal plate connectors, etc., are not a part of this report.

Respectfully submitted,

Mr. Vinayagar M. Balakrishnan V, P.E.

Fla. Reg. No.: 63107

### ROOF WIND LOAD CALCULATIONS PER ASCE 7

I. Given Data:

Job address: 6745 SW 139 St., Palmetto Bay, Fl.  
Roof Type: Sloped Deck type: Wood  
Building height: <15 Feet

II. Design Criteria:

Exposure: C  
Wind velocity (V): 146 mph  
Topography Factor ( $K_{zt}$ ): 1.0  
Importance factor (I): 1.0  
Directionality factor ( $K_d$ ): 0.85 (rigid structure-use Gust 1)  
Exposure factor ( $K_z$ ): 0.85  
Internal pressure coefficient ( $GC_{pi}$ ): +/-0.18  
Tributary area: 10 Square Feet (all zones)

III. Velocity Pressure( $Q_z$ ):

$$q_z = 0.00256(K_{zt})(K_d)(K_z)(V^2)(I)$$
$$q_z = -39.43 \text{ psf}$$

IV. Perimeter Area(A):

0.1 X minimum width or 0.4 X height, but not less than three feet or 0.04 X minimum width.  
Therefore, A= 3 ft.

V. Design Pressure (P) for Roof System

$$P = q_z(GC_p \pm GC_{pi})$$

$$P_1(\text{Field}) = 39.43 (-1.0-0.18) = -46.52 \text{ psf}$$

$$P_2(\text{Perimeter}) = 39.43 (-1.8-0.18) = -78.07 \text{ psf}$$

$$P_3(\text{Corners}) = 39.43 (-2.8-0.18) = -117.50 \text{ psf}$$

Vinayagar M Balakrishnan V, P.E.  
5/22/12

Vinayagar M Balakrishnan V, P.E.  
Florida Registration #63107

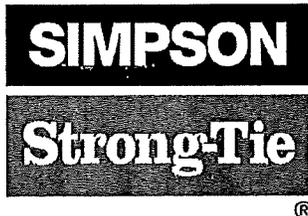
# SIM200802

Used for Florida State Wide Product Approval #

## FL11473

Products on this Report which are approved:

<u>Product</u>	<u>FL#</u>	<u>Product</u>	<u>FL#</u>
DETAL20	11473.1	LGUM210-2-SDS	11473.12
FGTR	11473.2	LGUM210-3-SDS	11473.13
FGTRE	11473.2	LGUM210-4-SDS	11473.14
FGTRHL	11473.2	LGUM26-2-SDS	11473.12
FGTRHR	11473.2	LGUM26-3-SDS	11473.13
HETA12	11473.3	LGUM26-4-SDS	11473.14
HETA16	11473.3	LGUM28-2-SDS	11473.12
HETA20	11473.3	LGUM28-3-SDS	11473.13
HETA24	11473.4	LGUM28-4-SDS	11473.14
HETA40	11473.4	LGUM410-SDS	11473.15
HETAL12	11473.5	LGUM46-SDS	11473.15
HETAL16	11473.5	LGUM48-SDS	11473.15
HETAL20	11473.5	LTA1	11473.16
HGAM10	11473.6	META12	11473.17
HGUM5.25	11473.7	META14	11473.17
HGUM5.50	11473.7	META16	11473.17
HGUM7.00	11473.8	META18	11473.17
HGUM7.25	11473.8	META20	11473.18
HGUM9.00	11473.8	META22	11473.18
HHETA12	11473.9	META24	11473.18
HHETA16	11473.9	META40	11473.18
HHETA20	11473.9	MSTAM24	11473.19
HHETA24	11473.10	MSTAM36	11473.19
HHETA40	11473.10	MSTCM40	11473.19
HM9	11473.6	MSTCM60	11473.19
HTSM16	11473.11	MTSM16	11473.2
HTSM20	11473.11	MTSM20	11473.2



®

SIMPSON STRONG-TIE COMPANY, INC



**Jax Apex Technology, Inc.**  
FBPE CA NO. 7547  
4745 Sutton Park Court, Suite 402  
Jacksonville, FL 32224/ 904/821-5200

Evaluation reports are the opinion of the engineer who prepared the report, based on the findings, and in no way constitute or imply approval by a local building authority. The engineer, in review of the data submitted, finds that, in his opinion, the product, material, system, or method of construction specifically identified in this report conforms with or is a suitable alternate to that specified in the Florida Building Code, SUBJECT TO THE LIMITATIONS IN THIS REPORT

Jeffrey P. Arneson, an employee of Jax Apex Technology, Inc. (Apex Technology), is the authorized evaluating engineer of this report. Apex Technology is the prime professional, as defined in Florida Rule 61G-30.002, authorized to sell the engineering services performed by Jeffrey P. Arneson, and is in no way acting, nor attempting to act, as an approved evaluation entity. Neither Jeffrey P. Arneson, nor any other employee of Apex Technology, has performed calculations or testing for the products listed in this report. This evaluation is based solely upon the review, under the direct supervision of Jeffrey P. Arneson, of testing and/or calculations submitted by the manufacturer.

The capacities listed in this report are based on the limiting capacities as determined from the substantiating data. We reviewed the substantiating data to a degree that allowed us to determine whether or not the work performed is consistent with the intended use of the product, and that the methods used are in compliance with, or meet the intent of, the Florida Building Code. All test reports were prepared by an approved testing laboratory.

**REPORT NO.:** SIM200802

**CATEGORY:** Structural Components  
**SUB CATEGORY:** Metal Connectors

**SUBMITTED BY:**

SIMPSON STRONG-TIE COMPANY, INC.  
5956 W. LAS POSITAS BOULEVARD  
PLEASANTON, CA 94588

**1. CERTIFICATION OF INDEPENDENCE:**

Jeffrey P. Arneson, the Florida engineer who prepared this report, and Apex Technology have no financial interest in the manufacturing, sales, or distribution of the products included in this report. Jeffrey P. Arneson and Apex Technology comply with all criteria as stated in Florida Administrative Code Chapter 9B-72.110.

**2. PRODUCT NAME**

Truss to Wall Connectors  
MTSM16, MTSM20, HTSM16, HTSM20, HM9, HGAM10

#### Embedded Truss Anchors

META12, META14, META16, META18, META20, META22, META24, META40,  
HETA12, HETA16, HETA20, HETA24, HETA40, HETAL12, HETAL16, HETAL20,  
HHETA12, HHETA16, HHETA20, HHETA24, HHETA40, LTA1, DETAL

#### Wood to Masonry Straps

MSTAM24, MSTAM36, MSTCM40, MSTCM60

#### Girder Tiedowns

FGTR, FGTR, FGTRHL, FGTRHR

#### Wood to Masonry Hangers

LGUM26-2, LGUM28-2, LGUM210-2, LGUM26-3, LGUM28-3, LGUM210-3,  
LGUM26-4, LGUM28-4, LGUM210-4, LGUM46, LGUM48, LGUM410,  
HGUM5.25, HGUM5.50, HGUM7.00, HGUM7.25, HGUM9.00

### 3. SCOPE OF EVALUATION

Load Evaluation as a Structural Component using the requirements of the *Florida Building Code, Building*.

### 4. DESCRIPTION:

**4.1 MTSM16 and MTSM20 Twist Straps for Wood to Masonry.** The MTSM16 and MTSM20 are used to anchor wood trusses, rafters, or beams to masonry or concrete walls. The MTSM fastens to the wood member with 10d common nails, and fastens to the wall with either  $\frac{1}{4} \times 2\frac{1}{4}$ " Titen Masonry Screws for a masonry wall, or  $\frac{1}{4} \times 1\frac{3}{4}$ " Titen Masonry Screws for a concrete wall. These connectors are manufactured from 16 gauge steel meeting ASTM A653 SS Grade 33. The galvanized coating complies with the G90 requirements of ASTM A653. Twist strap fastener schedules, dimensions and allowable loads are shown in Table 1. See Figure 1 for additional details of twist straps for masonry.

**4.2 HTSM16 and HTSM20 Twist Straps for Wood to Masonry.** The HTSM16 and HTSM20 are used to anchor wood trusses, rafters, or beams to masonry or concrete walls. The HTSM fastens to the wood member with 10d common nails, and fastens to the wall with either  $\frac{1}{4} \times 2\frac{1}{4}$ " Titen Masonry Screws for a masonry wall, or  $\frac{1}{4} \times 1\frac{3}{4}$ " Titen Masonry Screws for a concrete wall. These connectors are manufactured from 14 gauge steel meeting ASTM A653 SS Grade 50, Class 1. The galvanized coating complies with the G90 requirements of ASTM A653. Twist strap fastener schedules, dimensions and allowable loads are shown in Table 1. See Figure 1 for additional details of twist straps for masonry.

**4.3 HM9 Hurricane Tie.** The HM9 is used to anchor wood trusses, rafters, or beams to masonry or concrete walls. The HM9 fastens to the wood member with Simpson  $\frac{1}{4} \times 1\frac{1}{2}$ " SDS screws (provided with the part), and fastens to the wall with either  $\frac{1}{4} \times 2\frac{1}{4}$ " Titen Masonry Screws for a masonry wall, or  $\frac{1}{4} \times 1\frac{3}{4}$ " Titen Masonry Screws for a concrete wall. The HM9 is manufactured from 18 gauge steel meeting ASTM A653 SS Grade 33. The galvanized coating complies with the G90 requirements of ASTM A653. Hurricane tie fastener schedule, dimensions and allowable loads are shown in Table 1. See Figure 2 for additional details of the HM9.

**4.4 HGAM10 Hurricane Gusset Angle.** The HGAM10 is used to anchor wood trusses, rafters, or beams to masonry or concrete walls. The HGAM10 fastens to the wood member with Simpson  $\frac{1}{4} \times 1\frac{1}{2}$ " SDS screws (provided with the part), and fastens to the wall with  $\frac{1}{4} \times 2\frac{1}{4}$ " Titen Masonry Screws. Allowable loads are shown in Table 2. The HGAM10 is manufactured from 14 gauge steel meeting ASTM A653 SS Grade 33. The galvanized coating complies with the G90 requirements of ASTM A653. Angle

fastener schedule, dimensions and allowable loads are shown in Table 1. See Figure 3 for additional details of the HGAM10.

**4.5 META, HETA, HETAL, HHETA Embedded Truss Anchors.** Embedded Truss Anchors are used to anchor a wood member (usually a truss) to a masonry or concrete wall. Embedded truss anchors fasten to a single-ply wood truss with 10d×1½" nails or to a multiple-ply truss with 16d common nails. They are embedded in the masonry or concrete wall to a depth indicated on the side of the anchor (4" for META, HETA, and HETAL, and 5 1/16" for HHETA). The strap portion of the anchor is 1½" wide. The anchors are manufactured from steel meeting ASTM A653 SS Grade 50, Class 1, with the exception of the truss seat of the HETAL which is manufactured from steel meeting ASTM A653 SS Grade 33. Steel thickness is as specified in Table 9. The galvanized coating complies with the G90 requirements of ASTM A653. Embedded truss anchor fastener schedule, dimensions and allowable loads are shown in Table 2 for single installations and Table 3 for double installations. See Figures 4 and 6 for additional details of single and double embedded truss anchors.

**4.6 LTA1 Lateral Truss Anchor.** The LTA1 is used to anchor wood trusses, rafters, or beams to masonry or concrete walls. The LTA1 fastens to the wood member with 10d×1½" common nails and has legs which are embedded into the wall system. Allowable loads are shown in Table 2. The LTA1 is manufactured from 18 gauge steel meeting ASTM A653 SS Grade 33. The galvanized coating complies with the G90 requirements of ASTM A653. Truss anchor fastener schedule, dimensions and allowable loads are shown in Table 2. See Figure 5 for additional details of the LTA1.

**4.7 DETAL20 Double Embedded Truss Anchor.** The DETAL is a high capacity connector used to anchor single-ply wood trusses or rafters to masonry or concrete walls. The DETAL fastens to the wood members with 10d×1½" nails. They are embedded in the masonry or concrete wall to a depth of 4½ inches. The strap portion of the anchor is 1½" wide. The strap anchors are manufactured from steel meeting ASTM A653 SS Grade 50, Class 1, and the truss seat is manufactured from steel meeting ASTM A653 SS Grade 33. The strap anchors are 16 ga. steel and the seat is 18 ga. steel. The galvanized coating complies with the G90 requirements of ASTM A653. Embedded truss anchor fastener schedule, dimensions and allowable loads are shown in Table 3. See Figure 6 for additional details of the DETAL.

**4.8 MSTAM, MSTCM Wood to Masonry Strap Tie.** The MSTAM and MSTCM Strap Tie models are used to provide a tension connection between wood members and a masonry or concrete structure. The MSTAM Straps are 1½" wide for use on 1½" and larger members. They are installed with 10d common nails to the wood and either ¼×2¼" Titen Masonry Screws to masonry, or ¼×1¾" Titen Masonry Screws to concrete. The MSTCM Strap is 3" wide for use on doubled 2-by or single 4-by and larger members. They are installed with 16d sinker nails to the wood and either ¼×2¼" Titen Masonry Screws to masonry, or ¼×1¾" Titen Masonry Screws to concrete. The MSTCM Strap has countersunk nail slots for a lower nailing profile and coined edges for safer handling. The straps are manufactured from steel meeting ASTM A653 SS Grade 50, Class 1, of a thickness as specified in Table 4. The galvanized coating complies with the G90 requirements of ASTM A653. Masonry strap fastener schedule, dimensions and allowable loads are shown in Table 4. See Figure 7 for additional details of wood to masonry straps.

**4.9 FGTR, FGTRH, FGTRHL, FGTRHR Face Mount Girder Tie Down.** The FGTR is a non-pitch specific girder tie down that can be used in new construction or retrofit applications to tie down a girder truss or beam to a concrete or masonry wall. The

FGTR can be installed in a single application or can be doubled to achieve a higher uplift capacity. The FGTR fastens to the truss with Simpson Strong-Tie SDS 1/4" wood screws, and fastens to the masonry or concrete wall with Simpson Strong-Tie 1/2" diameter Titen HD fasteners, which are supplied with the connector. The FGTR uses a strap that is oriented with its flat dimension parallel to the truss for placement at the end of walls when the truss is parallel to the wall. The FGTRHL and FGTRHR are designed with the flat dimension of the strap at a 45 degree angle to the truss for anchorage of hip trusses. The FGTR straps are manufactured from 7 gauge ASTM A-1011 Grade 33 steel having  $F_y=33\text{ksi}$  and  $F_u=52\text{ksi}$  and the plates are made from 8 gauge ASTM A-1011 Grade 33 steel having  $F_y=33\text{ksi}$  and  $F_u=52\text{ksi}$ . They have a gray powder coat finish. Girder tie down fastener schedule, dimensions and allowable loads are shown in Table 5. See Figure 8 for additional details of face mount girder tie down connectors.

**4.10 LGUM, HGUM Masonry Girder Hangers.** LGUM and HGUM girder hangers are high capacity joist hangers that are used to connect wood girders and beams to masonry or concrete walls. The LGUM and HGUM use Simpson Strong-Tie Titen HD anchors to attach to the masonry or concrete wall, and Strong-Drive Screws (which are provided) to attach the beam to the hanger. To install the Titen HD anchors, drill holes of the same diameter as the anchor into the masonry or concrete. Holes should be 1/2" deeper than the specified Titen HD length. The SDS screws are installed best with a low-speed 1/2" drill and a 3/8" hex head driver. Predrilling holes for SDS screws is not required. The LGUM is manufactured from galvanized steel complying with ASTM A 653 SS Grade 40 with minimum yield and tensile strengths of 40 and 55 ksi (275 and 379 MPa), respectively. The HGUM is manufactured from galvanized steel complying with ASTM A 653 SS Grade 33 with minimum yield and tensile strengths of 33 and 45 ksi (228 and 310 MPa), respectively. The galvanized coating complies with the G90 requirements of ASTM A 653. The steel thicknesses are 0.099" (2.51 mm) for the LGUM, and 0.173" (4.39 mm) for the HGUM. Girder hanger fastener schedule, dimensions and allowable loads are shown in Table 6. See Figure 9 for additional details of masonry girder hangers.

## 5. MATERIALS

**5.1 Steel.** Steel specifications for each product listed in this evaluation report shall be as indicated in the previous section.

**5.2 Wood.** Wood members to which these connectors are fastened shall be solid sawn lumber, glued-laminated lumber, or structural composite lumber having dimensions consistent with the connector dimensions shown in Tables 1 through 6. Unless otherwise noted, lumber shall be Southern Pine or Douglas Fir-Larch having a minimum specific gravity of 0.50. Where indicated by SPF, lumber shall be Spruce-Pine-Fir having a minimum specific gravity of 0.42.

**5.3 Nails and Bolts.** Unless noted otherwise, nails shall be common nails. Nails shall comply with ASTM F 1667 and shall have the minimum bending yield strengths  $F_{yb}$ :

Common Nail Pennyweight	Nail Shank Diameter (inch)	$F_{yb}$ (psi)
10d	0.148	90,000
16d sinker	0.148	90,000
16d	0.162	90,000

Fasteners for galvanized connectors in pressure-preservative treated wood shall be hot-dipped zinc coated galvanized steel with coating weights in accordance with ASTM A153. Fasteners for stainless steel connectors shall be stainless steel.

5.4 **Concrete/Masonry.** Concrete and Masonry design specifications shall be the stricter of the specifications by the engineer of record, the Florida Building Code minimum standards, the following, or as noted in the report:

Material	Specification	Minimum Compressive Strength
Concrete, f'c	-	2500 psi
Masonry, f'm	ASTM E447	1500 psi
Masonry Unit	ASTM C90	1900 psi
Mortar	ASTM C270 Type S	1800 psi (or by proportions)
Grout	ASTM C476	2000 psi (or by proportions)

## 6. INSTALLATION

Installation shall be in accordance with this report and the most recent edition of the Simpson Strong-Tie *Wood Construction Connectors* catalog. The information in this report supercedes any conflicting information between information provided in this report and the catalogue.

## 7. SUBSTANTIATING DATA

Test data submitted by Testing Engineers Inc. and Product Testing, Inc., and signed and sealed calculations performed by Jeremy Gilstrap, P.E., and Samuel Hensen, P.E., performed in accordance with the 2007 Florida and Residential Building Codes.

Product	Test Number	Date Tested
MTSM	B845, H756	2/27/90, 12/6/00
HTSM	02-3667	1/30/02
HM9 Uplift	02-3793	5/15/02
HM9 F1 Direction	02-3793	5/15/02
HM9 F2 Direction	02-3793	5/15/02
HGAM10 Uplift	02-3884	7/29/02
HGAM10 F1 Direction	H046	3/25/99
HGAM10 F2 Direction	H141	6/22/99
META Uplift	02-3674, 02-3802, 02-3861, 04-4675	6/4/02, 6/8/02, 7/24/02, 2/8/04
META F1	02-3674, 02-3802	6/4/02, 6/8/02
META F2	02-3674, 02-3802, 02-3861	6/4/02, 6/8/02, 7/24/02
HETA Uplift	02-3803, 02-3862, 04-4676	6/10/02, 7/26/02, 2/8/04
HETA F1	02-3803	6/10/02
HETA F2	02-3803, 02-3862	6/10/02, 7/26/02,
HHETA Uplift	02-3676, 02-3863, 04-4674	6/4/02, 7/29/02, 2/7/04
HHETA F1	02-3676	6/4/02
HHETA F2	02-3676, 02-3863	6/4/02, 7/29/02
HETAL Uplift	02-3803, 02-3862, 04-4676	6/10/02, 7/26/02, 2/8/04
HETAL F1	D793	3/17/94

Product	Test Number	Date Tested
HETAL F2	D844	3/28/94
DETAL Uplift	O797	3/28/08
DETAL F1	O795, O799	5/12/08, 3/27/08
DETAL F2	O796, O798	6/05/08, 3/28/08
LTA1 Uplift	02-3616	2/13/02
LTA1 F1	02-3616	2/13/02
LTA1 F2	02-3616	2/13/02
MSTAM24 Uplift	02-3795	5/17/02, 5/17/02
MSTAM36 Uplift	02-3795	5/17/02, 5/17/02
MSTCM40 Uplift	02-3796	5/31/02
MSTCM60 Uplift	N471	1/26/07
FGTR Uplift	04-5004, 04-5005	10/6/04, 10/6/04
FGTRE Uplift	04-5010	10/29/04
FGTRHL/R Uplift	04-4915	10/13/04
LGUM Down	M202, M 203, M204, M222, M224	7/13/06, 7/13/06, 7/13/06, 7/14/06, 8/03/06
LGUM Uplift	M211, M212, M213	8/18/06, 8/18/06, 8/21/06
HGUM Down	M207, M209, M216, M217	9/11/06, 9/11/06, 10/20/06, 10/20/06
HGUM Uplift	M729, M731	8/3/06, 8/04/06

## 8. FINDINGS

Upon review of the data submitted by Simpson Strong-Tie, it is my opinion that the models as described in this report conform with or are a suitable alternative to the standards and sections in the 2007 Florida Building Code, Building, and the Florida Building Code, Residential code editions listed in section 10 of this report, subject to the limitations below. Maximum allowable loads shall not exceed the allowable loads listed in this report.

## 9. LIMITATIONS:

1. Maximum allowable loads shall not exceed the allowable loads listed in this report. Allowable loads listed in this report are based on allowable stress design. The loads in this report are not applicable to Load and Resistance Factor Design.
2. Capacity of wood members is not covered by this report. Capacity of wood members must be checked by the building designer.
3. Allowable loads for more than one direction for a single connection cannot be added together. A design load that can be divided into components in the directions given must be evaluated as follows:  
Design Uplift/Allowable Uplift + Design Lateral Parallel to Plate/Allowable Lateral Parallel to Plate + Design Lateral Perp. to Plate/Allowable Lateral Perp. to Plate < 1.0

## 10. CODE REFERENCES

### Florida Building Code, Building 2007 Edition

Section 104.11	Alternate Materials and Methods
Chapter 1714.2	Load Test Procedure Specified
Chapter 21	Masonry
Chapter 22	Steel
Chapter 23	Wood

Florida Building Code, Residential 2007 Edition

R101.2.1	Scope
R4407	HVHZ Masonry
R4408	HVHZ Steel
R4409	HVHZ Wood

11. ALLOWABLE LOADS:

The tables that follow reference the allowable loads for the aforementioned products.

TABLE 1 ALLOWABLE LOADS AND FASTENERS FOR TRUSS TO MASONRY OR CONCRETE WALL CONNECTORS							
Model No.	Ga	Length (in.)	Fasteners			Allowable Uplift Loads (160)	
			Truss/Rafter	CMU (Titen)	Concrete (Titen)	Southern Pine/Douglas Fir-Larch	Spruce-Pine-Fir
MTSM16	16	16	7-10d	4-1/4x2 1/4	4-1/4x1 1/4	875	755
MTSM20	16	20	7-10d	4-1/4x2 1/4	4-1/4x1 1/4	875	755
HTSM16	14	16	8-10d	4-1/4x2 1/4	4-1/4x1 1/4	1175	1010
HTSM20	14	20	10-10d	4-1/4x2 1/4	4-1/4x1 1/4	1175	1010
HM9 <sup>2</sup>	18	-	4-SDS 1/4x1 1/2	5-1/4x2 1/4	5-1/4x1 1/4	805	690
HGAM10 <sup>3,b</sup>	14	-	4-SDS 1/4x1 1/2	4-1/4x2 1/4	4-1/4x2 1/4	850	850

Notes:

1. Loads include a 60% load duration increase on the fastener capacity for wind loading where allowed by the Florida Building Code. Loads do not include a stress increase on the strength of the steel. No further increases are permitted. Reduce loads where other loads govern.
2. HM9 allowable F1 load shall be 635 lbs (DFL/SYP) & 545 lbs (SFP), and allowable F2 load shall be 200 lbs (DFL/SYP) & 170 lbs (SPF).
3. HGAM10 allowable F1 load shall be 1005 lbs (DFL/SYP) & 870 lbs (SFP), and allowable F2 load shall be 1105 lbs (DFL/SYP) & 950 lbs (SPF).
4. Allowable loads for the HGAM10 are for one connector. A minimum rafter thickness of 2 1/2" must be used when framing anchors are installed on each side of the joist or truss.

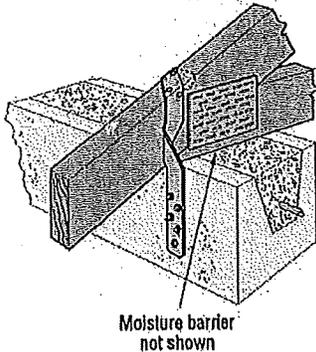


Figure 1  
Typical MTSM/HTSM Application

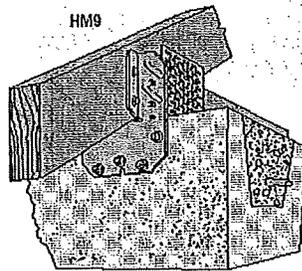


Figure 2  
Typical HM9 Installation

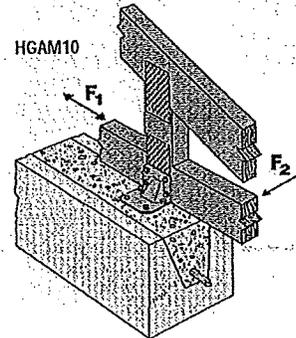


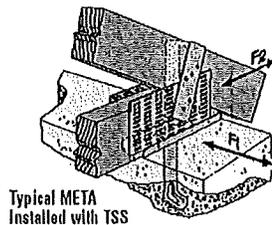
Figure 3  
Typical HGAM10 Installation

**TABLE 2 ALLOWABLE LOADS AND FASTENERS**

Model No.	Ga	H	Uplift				Lateral Loads	
			1-Ply So. Pine Truss		2 or 3-Ply So. Pine Truss		F <sub>1</sub> (parallel to wall)	F <sub>2</sub> (perpen. to wall)
			Fasteners	Load	Fasteners	Load		
META12	18	8	7-10d×1½	1450	6-16d	1450	340	725 <sup>a</sup>
META14		10	7-10d×1½	1450	6-16d	1450	340	725 <sup>a</sup>
META16		12	7-10d×1½	1450	6-16d	1450	340	725 <sup>a</sup>
META18		14	7-10d×1½	1450	6-16d	1450	340	725
META20		16	6-10d×1½	1270	5-16d	1245	340	725
			7-10d×1½	1450	6-16d	1450	340	725
META22		18	7-10d×1½	1450	6-16d	1450	340	725
META24		20	7-10d×1½	1450	6-16d	1450	340	725
META40		36	7-10d×1½	1450	6-16d	1450	340	725
HETA12	16	8	7-10d×1½	1520	7-16d	1780	340	725
HETA16		12	9-10d×1½	1810	8-16d	1810	340	725
HETA20		16	8-10d×1½	1735	7-16d	1780	340	725
			9-10d×1½	1810	8-16d	1810	340	725
HETA24		20	9-10d×1½	1810	8-16d	1810	340	725
HETA40		36	9-10d×1½	1810	8-16d	1810	340	725
HHETA12	14	8	7-10d×1½	1565	7-16d	1820	340 <sup>b</sup>	815
HHETA16		12	10-10d×1½	2235	9-16d	2235	340 <sup>b</sup>	815
HHETA20		16	9-10d×1½	2010	8-16d	2080	340 <sup>b</sup>	815
			10-10d×1½	2235	9-16d	2235	340 <sup>b</sup>	815
HHETA24		20	10-10d×1½	2235	9-16d	2235	340 <sup>b</sup>	815
HHETA40		36	10-10d×1½	2235	9-16d	2235	340 <sup>b</sup>	815
HETAL12	16	7	10-10d×1½	1085	10-16d	1270	415	1100
HETAL16		11	14-10d×1½	1810	13-16d	1810	415	1100
HETAL20		15	14-10d×1½	1810	13-16d	1810	415	1100
LTA1	18	3 1/8	12-10d×1½	1420	12-10d×1½	1420	485	1425

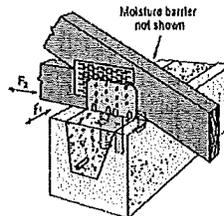
**Notes:**

1. Loads include a 60% load duration increase on the fastener capacity in wood for wind loading where allowed by the Florida Building Code. Loads do not include a stress increase on the strength of the steel. No further increases are permitted. Reduce loads where other loads govern.
2. Five nails must be installed into the truss seat of the HETAL
3. Parallel-to-plate load towards face of HETAL is 1975 lbs.
4. Minimum F<sub>c</sub> is 2,000psi
5. It is acceptable to use a reduced number of fasteners in a product provided that there is a reduction in load capacity. The load per nail can be approximated by dividing the allowable load by the number of fasteners. This concept applies to all member sizes. There should be a minimum of 4 nails installed in the strap. Lateral loads do not apply when fewer than 7 fasteners are used with the HETA and HHETA anchors or less than 6-16d or 7-10d×1½" fasteners are used with the META anchor.
6. The HHETA allowable F<sub>1</sub> load can be increased to 435 pounds if the strap is wrapped over the truss and a minimum of 12 nails are installed.



**Figure 4**

**META/HETA/HHETA Typical Installation**



**Figure 5**

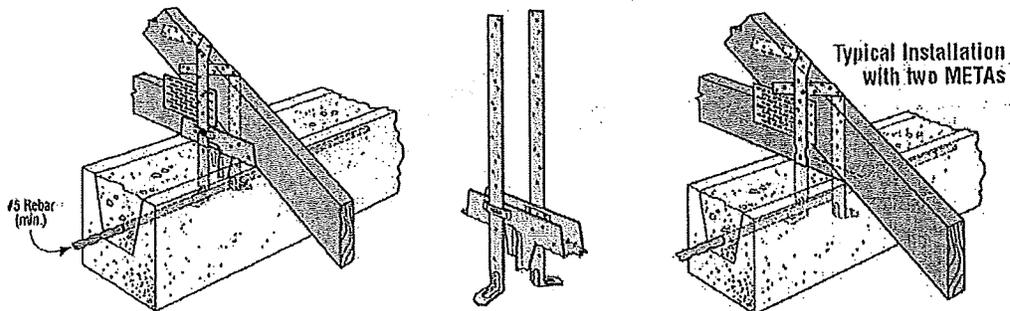
**LTA1 Typical Installation**

Simpson Strong-Tie

Model No.	Qty.	Application	Uplift Loads				Lateral Loads	
			1-Ply Southern Pine Truss		2 or 3 Ply Southern Pine Truss		F <sub>1</sub> (parallel to wall)	F <sub>2</sub> (perpen. to wall)
			Fasteners	Load	Fasteners	Load		
DETAL20	1	CMU	18-10d×1½	2480	-	-	2000 <sup>6</sup>	1370
		Concrete	18-10d×1½	2480	-	-	2000	1505
META	2	CMU	10-10d×1½	1985	14-16d	1900	1210	1160
		Concrete	10-10d×1½	1985	14-16d	2565	1210	1160
HETA	2	CMU	10-10d×1½	2035	12-16d	2500	1225	1520
		Concrete	10-10d×1½	2035	12-16d	2700	1225	1520
HHETA	2	CMU	10-10d×1½	2035	12-16d	2500	1225	1520
		Concrete	10-10d×1½	2035	14-16d	3350	1225	1520

**Notes:**

1. Loads include a 60% load duration increase on the fastener capacity in wood for wind loading where allowed by the Florida Building Code. Loads do not include a stress increase on the strength of the steel. No further increases are permitted. Reduce loads where other loads govern.
2. Minimum  $f'_c$  is 2,500 psi.
3. Install with spoons facing outward and spaced no more than 1/8" wider than the truss width.
4. Install half of the required number of fasteners in each strap, except for the DETAL20. For DETAL20, install six nails in each strap and six nails in the truss seat.
5. Lateral loads for META, HETA, and HHETA anchors apply only to 2- or 3-ply applications with anchors spaced a minimum of 3" apart. For single ply applications use lateral loads in Table 2. DETAL lateral load apply to single-ply application.
6. DETAL20 Lateral Loading in the F<sub>1</sub> direction anchored in CMU greater than 1,790 lb. may result in deflection up to 5/32" in the F<sub>1</sub> direction.



**Figure 6  
DETAL and Double META/HETA/HHETA Application**

TABLE 4 MASONRY STRAPS ALLOWABLE LOADS, FASTENERS AND DIMENSIONS									
Model No.	Ga.	Dimensions (inches)		Simpson Strong-Tie Titen Screws		DF/SP (160)		SPF (160)	
		W	L	CMU	Concrete	Nails	Load	Nails	Load
MSTAM24	18	1¼	24	5-¼×2¼	5-¼×1¾	8-10d	1500	9-10d	1500
MSTAM36	16	1¼	36	8-¼×2¼	8-¼×1¾	10-10d	1870	11-10d	1870
MSTCM40	16	3	40¼	14-¼×2¼	14-¼×1¾	22-16d Sinker	4220	26-16d Sinker	4220
MSTCM60	16	3	59½	14-¼×2¼	14-¼×1¾	26-16d Sinker	4220	26-16d Sinker	4220

Notes:

1. Loads include a 60% load duration increase on the fastener capacity in wood for wind loading where allowed by the Florida Building Code. Loads do not include a stress increase on the strength of the steel. No further increases are permitted. Reduce loads where other loads govern.
2. Minimum edge distance is 1½" for Titen Masonry Screws.
3. Minimum  $f'_m = 1500$  psi and minimum  $f'_c = 2500$  psi.
4. Products shall be installed such that the Titen screws are not exposed to moisture.

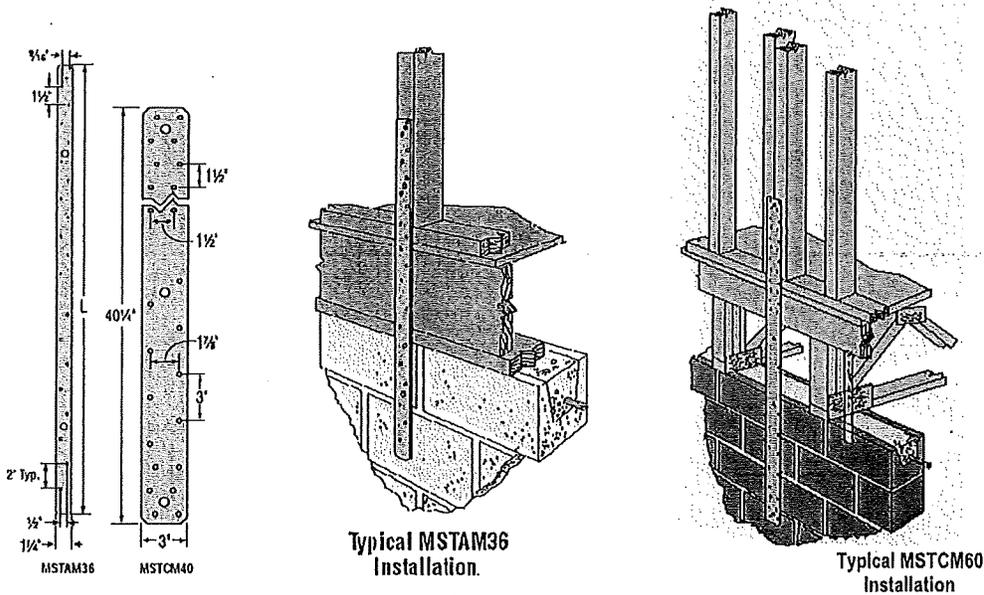


Figure 7  
MSTAM/MSTCM Typical Dimensions and Installation

TABLE 5 FGTR SERIES ALLOWABLE LOADS AND FASTENERS				
Model No.	Qty.	Fasteners		Allowable Uplift Load (160)
		To Block and Concrete Wall	To Truss	
FGTR	1	2-Titen HD 1/2x5"	18-SDS1/4x3	5000 <sup>6</sup>
	2	4-Titen HD 1/2x5"	36-SDS1/4x3	9400
FGTRHL/R	1	2-Titen HD 1/2x5"	18-SDS1/4x3	3850
FGTRE	1	2-Titen HD 1/2x5"	18-SDS1/4x3	4685 <sup>6</sup>
FGTRE+FGTR	1 Each	4-Titen HD 1/2x5"	36-SDS1/4x3	5000 <sup>6</sup>

Notes

1. Loads include a 60% load duration increase on the fastener capacity in wood for wind loading where allowed by the Florida Building Code. Loads do not include a stress increase on the strength of the steel. No further increases are permitted. Reduce loads where other loads govern.
2. Products shall be attached to grouted and reinforced CMU walls or reinforced concrete walls that are designed to transfer the uplift loads to the foundation.
3. Minimum edge distance for the Titen HD is 4"
4. THD's should be spaced in every other hole on the part
5. Attached members must be designed to resist the applied loads
6. Products used for corner applications shall be limited to 4685 lbs allowable
7. Loads are governed by the grouted wall capacity based on testing of the products attached to the corner of a block wall at an average ultimate load of 14,800 lbs. The connector has been tested attached to a steel column to an allowable load of 11,400 lbs which can be used for design provided the wall is designed by the engineer of record to transfer the uplift forces.

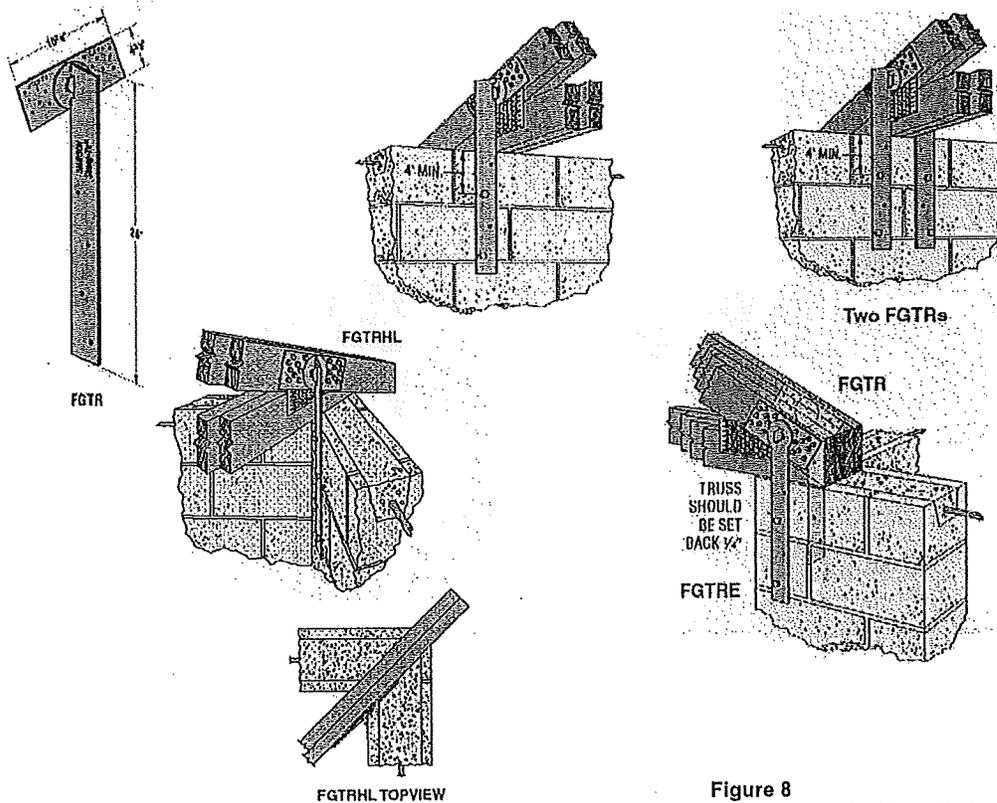


Figure 8  
FGTR/FGTRE/FGTRHL/FGTRH Typical Installation



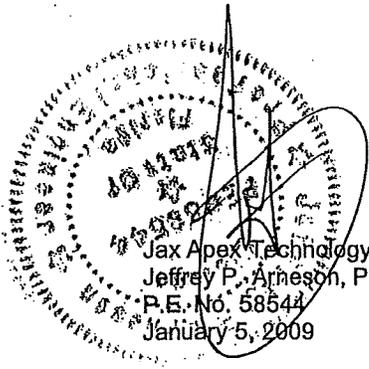
**12. IDENTIFICATION**

Each connector covered by this report shall be stamped with the manufacturer's name and/or trademark and the product name.

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Jax Apex Technology, Inc.  
Jeffrey P. Arneson, P.E.  
P.E. No. 58544  
January 5, 2009



**Florida International Engineering and Testing Lab LLC**  
16701SW 117 Avenue, Miami, FL 33177  
Telephone: (305) 378-1991-Fax: (305) 378-1997

Miami-Dade Lab Certification # 07-0612.11-State of Florida ca #27273

April 20, 2012

ROOF RETROFIT. Section 553.844 Florida Statutes.

Address: 6745 SW 139<sup>th</sup> St., Palmetto Bay, Fl.

Areas inspected:

Attic was visited by means of ceiling hatch in garage and observations were made thru destructive openings at corners of two roof areas.

Observations:

Type of Roof: Hipped.

Sheathing: plywood-5/8" CDX

Framing: 2x4 Wood rafter/trusses- 24" o.c.

Existing truss anchoring to masonry wall with 2-3 nails on one side of wood member to be supplemented by providing additional 10d nails thru strap hole opening for a total of 4 nails to side and Simpson gussets (right angle) Type HGAM 10 with 4-3-1/2"x1/4" tap cons into tie beam and 4-1-1/2" wood screws as provided by Simpson HGAM 10 kit at opposite side of wood member for a minimum uplift capacity of 500 lbs +, as mandated by roof-to-wall retrofit requirements.

Priority areas, if required, shall be corners and longest spanned trusses.

Respectfully submitted,

Mr. Vinayagar M. Balakrishnan V, P.E.

Fla. Reg. No.: 63107

SEE REVISED  
LETTER  
DATED 5/22/2012



**Florida International Engineering and Testing Lab LLC**

16701SW 117 Avenue, Miami, FL 33177

Telephone: (305) 378-1991-Fax: (305) 378-1997

Miami-Dade Lab Certification # 07-0612.11-State of Florida ca #27273

April 20, 2019

ROOF RETROFIT. Section 553.844 Florida Statutes.

Address: 6745 SW 139<sup>th</sup> St., Palmetto Bay, Fl.

Areas inspected:

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Priority areas, if required, shall be corners and longest spanned trusses.

Respectfully submitted,

Mr. Vinayagar M. Balakrishnan V, P.E.

Fla. Reg. No.: 63107

6/28/2012

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Inspection Schedule for aramos  
Scheduled for June 28, 2012

<u>Permit#</u>	<u>Address</u>	<u>Folio</u>	<u>Contractor</u>
BLD-2012-1468	6745 SW 139 ST	3350230400140	GABLES GROVE CONSTRUCTION MANAGEMENT INC (305) 266-5095
		<u>Owner</u>	
		GREG & JENNIFER HERSKOWITZ	

Description  
ANCHOR TRUSSES REPAIR (2010 F.B.C.)

---

Type: 758

FINAL STRUCTURAL BUILDING

Time: 08:00

Notes:

Comments:

OK - SUBMITTED P.E. LETTER

Date:

6/28

Signature:

AK



**Florida International Engineering and Testing Lab LLC**

16701SW 117 Avenue, Miami, FL 33177  
Telephone: (305) 378-1991-Fax: (305) 378-1997

*Miami-Dade Lab Certification # 07-0612.11-State of Florida ca #27273*

June 15, 2012

Village of Palmetto Bay  
Building & Zoning Dept.  
9705 E Hibiscus Dr.  
Palmetto Bay, FL 33157

Re: LETTER OF COMPLIANCE FOR ROOF RETROFIT. Section 553.844 Florida Statutes.

Job Address: 6745 SW 139<sup>th</sup> St., Palmetto Bay, FL.

Permit No.: BLD 2012-1468

Areas inspected:

Roof to wall connections were observed by means of roof openings during the time of supplemental installation.

Observations:

Dade County approved Right angle Simpson Gusset Type HGAM 10 were installed at each end of truss and attached to concrete tie beam with 4-3-1/2"x1/4" Tap cons and Simpson HGAM 10 kit wood fasteners providing a minimum uplift capacity of 500 lbs., as per mandate specified in sub section 201 of Section 553.844 Florida Statutes. In addition, existing straps (where original holes permit) have been re-nailed w/10d nails. Supplemental installation to the entire house surpasses the 15% expenditure Statutory requirements and complies with the intent of the Florida Building Code, 2010.

These results are based on observations of a limited scope pursuant to the compliance of requirements under Section 553.844 Florida Statutes.

Respectfully submitted,

*VMRV*  
*6-15-12*

Mr. Vinayagar M. Balakrishnan V, P.E.

Fla. Reg. No.: 63107







38

PLANS PROCESSING

Process Number: PTR 2014-0074  
Description of Work: TRUSSES  
Property Address: 6745 SW 139

Department	Date In	Date Out	Total Business Days	Approved	Denied	Comments
Zoning						
Building	9/17/14	9/17/14	1	✓		
Structural	9/15/14	9/16/2014		✓		
Electrical						
Mechanical						
Plumbing						
Planning & Zoning						
Public Works						
ADA Review						
Code Enforcement Verification	9/17/14			✓		
Final Signature						
Pricing						
Call for Pick up						

Permit #: BTR-2014-0074  
Master permit #: BLD-2014-5499

Permit type: btruss - TRUSS SHOP DRAWINGS RE  
Routing queue: b12 - SHOP DRAWINGS- RES/COMM

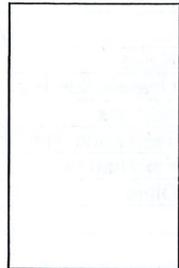
Address: 6745 SW 139 ST  
PALMETTO BAY, FL 33157

Group # - Name	Action Code	Action Description	Completion Date	Completion Code	Completed By	Comments
1 - PERMIT COUNTER	intake	APPLICATION INTAKE	9/15/2014		ahewlett	
1 - PERMIT COUNTER	cuff	COLLECT UPFRONT FEE	9/15/2014		ahewlett	
2 - PLANS PROCESSING	routeplans	ROUTE PLANS FOR REVIEW	9/15/2014		ahewlett	
3 - BUILDING REVIEW	brev	BUILDING REVIEW	9/17/2014	apprve	aramos	BUILDING APPROVED
4 - STRUCTURAL REVIEW	srev	STRUCTURAL REVIEW	9/17/2014	apprve	mcanakat	STRUCTURAL APPROVED
5 - FIRE DEPARTMENT REV	compcomm	COMPILE COMMENTS	9/18/2014		jvillegas	
6 - PLANS PROCESSING	notifycust	NOTIFY CUSTOMER	9/18/2014		jvillegas	
7 - PERMIT COUNTER	verifycont	VERIFY CONTRACTOR	9/18/2014		jvillegas	
8 - PLANS PROCESSING	notifycust	NOTIFY CUSTOMER	9/18/2014		jvillegas	
9 - PERMIT COUNTER	collissue	COLLECT FEES/ISSUE PERMIT	9/18/2014		jvillegas	
10 - INSPECTIONS	721	ROOF TRUSSES/BRACING	10/7/2014	apprve	aramos	
10 - INSPECTIONS	773	ANCHOR AND STRAPS (ENG CALCI	10/7/2014	apprve	aramos	
10 - INSPECTIONS	758	FINAL STRUCTURAL BUILDING	6/25/2015	apprve	jgarcell	
10 - INSPECTIONS	758	FINAL STRUCTURAL BUILDING	6/25/2015	apprve	jgarcell	APPROVED INSPECTED ON 10/07/2014



# Village of Palmetto Bay Permit Application

Department of Building & Capital Projects  
 9705 E. Hibiscus Street  
 Palmetto Bay, Florida 33157  
 Phone: (305) 259-1250 Fax: (866) 927-5576 Inspections: (305) 259-1253



**GENERAL INFORMATION:** Please read these instructions carefully before submitting the work for review

This application must be completed and signed by both the property owner and qualifier. Both of these signatures must be notarized. Please print legibly or type in order not to delay your application. For roofing permits, in addition to this permit you must also fill out a roofing permit application. Express permits require an additional fee and will only be accepted between the hours of 8:00 A.M. and 10:00 A.M., Monday through Friday. All other permits/plans must be dropped off before 4:30 p.m. for regular processing. During the processing of your application you may be asked to submit additional information. There may be additional permits and reviews required from other governmental agencies not affiliated with Palmetto Bay.

**APPLICATION:**

Clerk's Initials	Plan Process Number	Master Permit Number	Subsidiary Permit Number(s)	Expiration Date
<i>[Signature]</i>	BTR2014-071	2014-5499		

Job Address: 6745 SW 139 st mia FL 3  
 Address Unit number City State Zip Code

Folio Number: <u>3350230400140</u>	Linear Feet: _____ Units: _____ Stories: _____
Lot: _____ Block: _____	Value of Proposed Work: _____ Est. Bldg. Value: _____
Subdivision: _____ PB: _____ PG: _____	Tax Assessed Value: _____
Current Use of Property: _____	Flood Zone: _____ Base Floor Elev.: _____
Proposed Use of Property: _____	Homeowner's Association: _____
Description of Work: <u>Trusses</u>	I affirm that there <input type="checkbox"/> are or <input type="checkbox"/> are no restrictive covenants associated with the underlying property that would affect the pending application. Failure to disclose this information shall result in the immediate revocation of any type of permit or certificate of use/occupancy.
Zoning: _____ Square Feet: _____	
Tenant Information: _____ Unit Number: _____	

Check Permit Type		Check Permit Change		Check Type of Improvement			
Building		Change of Contractor		New Construction		Deck/Concrete flatwork	
Electrical		Permit Renewal		Exterior Alteration		Window Replacement	
Mechanical		Plan Revision		Interior Alteration		Shutters	
Plumbing		Permit Extension		Attached Addition		Garage Doors	
LPGX		Supplement		Detached Addition		Storage Shed	
Roofing		Re-inspection Fee		Repair		Railings	
Fence				Repair due to Fire		Stairs	
Sign				Demolish		Windows/Doors	
Public Works				Screen Enclosure		Roofing	
Other				Driveway		Re-Roof	
				Fence		Seal-cote	
				Pool		Other	

*[Handwritten signature]*  
 2/10/14 1

Architect Information	Engineer Information
Name:	Name:
License Number:	License Number:
Address:	Address:
Telephone Number:	Telephone Number:
Fax Number:	Fax Number:
Other:	Other:

Property Owner	Contractor
Name:	Company Name: <i>Chelle Construction</i>
Address:	Qualifier: <i>Richard Lehman</i>
Home Telephone:	License Number: <i>CGC 062070</i>
Business Telephone:	Address: <i>19151 SW 108 AVE</i>
Other Telephone:	Telephone Number: <i>305 218 3278</i>
Fax Number:	Fax Number: <i>305 233 6424</i>
Does Property have Homestead Exemption	Phone Number for Pick Up: <i>305 218 3278</i>

Bond Company (if applicable)	Mortgage Lender
Name:	Name:
Address:	Address:
City: State: Zip	City: State: Zip

Classification Of Proposed Work		
Residential <input type="checkbox"/>	Duplex <input type="checkbox"/>	Townhouse <input type="checkbox"/> Multi Family <input type="checkbox"/> Commercial <input type="checkbox"/> Industrial <input type="checkbox"/> Other <input type="checkbox"/>
Effective Code _____	Occupancy _____	Construction Type _____
Zoning:	Variance Number:	Remarks:

**OWNER AFFIDAVIT**

Application is hereby made to obtain a permit to do the work and installation as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work will be performed to meet the standards of all laws regulating construction in this jurisdiction. I understand that a separate permit must be secured for ELECTRICAL, MECHANICAL, PLUMBING, SIGNS, WELLS, POOLS, ROOFING, SHUTTERS, WINDOWS, FURNACES, BOILERS, HEATERS, TANKS, FENCE, DRIVEWAYS and AIR CONDITIONERS, ETC. In addition to the requirements of this permit, there may be additional restrictions found in the Public Records, and there may be additional permits required from other governmental entities.

I, the owner of the property, have disclosed all information related to any work that has been performed in the prior twelve months to the Building Division as part of this application. Further, I am fully aware that if the cumulative cost of work to my home or business under this and any other permit equals or exceeds fifty percent of the fair market value of the structure, the entire structure must meet the present federal flood criteria for finished floor elevation. I am also fully aware that if the cost of work to my home or business under this and any other permit equals or exceeds fifty per cent of the replacement cost of the structure, then the entire structure must conform to current code requirements of the Florida Building Code.

I, the owner of the property, understand that a permit application is subject to denial and a validated permit or permit card is subject to revocation or modification based upon applicable deeds, covenants, declarations, easements and any other legal restriction. By issuing a permit, the Village of Palmetto Bay makes no representation as to the existence or validity of any property restriction.

I, the owner of the property certify that all the foregoing information is accurate and that all work will be done in compliance with all applicable laws regulating construction and zoning.

**WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.**

Signature of Owner \_\_\_\_\_

State of Florida, County of \_\_\_\_\_

Sworn to (or affirmed) and subscribed before this \_\_\_\_ day of \_\_\_\_\_, 20\_\_\_\_.

by (print name) \_\_\_\_\_

Notary Name \_\_\_\_\_

Personally known  or I.D. \_\_\_\_\_

Type of identification produced: \_\_\_\_\_

Signature of Qualifier *[Signature]* \_\_\_\_\_

State of Florida, County of *Miami Dade* \_\_\_\_\_

Sworn to (or affirmed) and subscribed before this *14* day of *Sept*, 20\_\_\_\_.

by (print name) \_\_\_\_\_

Notary Name *[Signature]* \_\_\_\_\_

Personally known or I.D. *1050-771-19-1050* \_\_\_\_\_

Type of identification \_\_\_\_\_



**IMPORTANT NOTICES**

- Do not begin work without receiving you validated permit and permit card. Applying for a permit does not grant you the right to commence construction. Construction can only occur during the hours of 7:00 a.m. to 6:00 p.m., Monday thru Friday and from the hours of 9:00 a.m. to 5:00 p.m. on Saturdays. No construction activity is permitted on Sunday
- All construction sites must be maintained in a clean and orderly condition free from construction debris. Failure to do so will result in a fine and a suspension of inspections until said property is cleaned.
- Streets and neighboring properties shall be kept free from dirt and debris.
- Swales must be protected from damage by equipment or vehicles and sidewalks cannot be blocked.
- Portable Toilets for construction jobs require a separate permit. If toilet is not available the inspection will not be performed.
- Water cannot be discharged into the right of way or storm drains without the approval of the Public Works Department.
- No equipment or materials can be stored on the right of way; they must only be stored on your property.
- Florida Department of Health approval is required for applications involving Septic Tanks. Department of Environmental Resources Management (DERM) and /or Miami-Dade Water and Sewer Department approval is required for applications involving sewers and water. The tree section of the Department of Environmental Resources Management (DERM) approval is required on all landscape plans and on all tree removal applications.

**CHECKLIST (OFFICE USE ONLY)**

- |   |   |  |
|---|---|--|
| <input type="checkbox"/> OWNER-BUILDER FORM<br>(Attached)                             | <input type="checkbox"/> HEALTH DEPARTMENT APPROVAL<br>(Septic/Sewer)             | <input type="checkbox"/> PERMIT CLERK REVIEW<br>By: _____  |
| <input type="checkbox"/> FIRE DEPARTMENT APPROVAL<br>(Commercial/multi (family only)) | <input type="checkbox"/> IMPACT FEE<br>(New Construction)                         | <input type="checkbox"/> Complete Application<br><input type="checkbox"/> Current liability ins.<br><input type="checkbox"/> Worker's Comp.<br><input type="checkbox"/> Cont. Lic. Check |
| <input type="checkbox"/> CONCURRENCY<br>(New Construction)                            | <input type="checkbox"/> SCHOOL REVIEW<br>(New Construction)                      |  |
| <input type="checkbox"/> PROOF OF OWNERSHIP<br>(Attached)                             | <input type="checkbox"/> DERM REVIEW<br>(New Construction/Additions/Tree Removal) | <input type="checkbox"/> OTHER<br>(Specify and Attach)   |
| <input type="checkbox"/> CONDO ASSOCIATION APPROVAL                                   | <input type="checkbox"/> PUBLIC WORKS   | <input type="checkbox"/> FLORIDA DEPARTMENT OF<br>BUSINESS AND<br>PROFESSIONAL REGULATION<br>APPROVAL (RESTAURANTS)  |
| <input type="checkbox"/> UPFRONT FEES AMOUNT: _____                                   |   |  |

**PERMIT FEES (OFFICE USE ONLY)**

Scanning Fees Small (1.85 per sheet)	Art in Public Places	
Scanning Fees Large (3.50 per sheet)	Certificate of Use and Occupancy	
Village of Palmetto Bay Permit Fees	Concurrency Fee (7.35%)	
Miami-Dade County Fees (sq. ft. x \$65//1000x0.60)	Technology Fee (6.3%)	
Flood Zone Review	Zoning Inspection Fee (157.50 per application)	
Radon-Inspector State Educational Fund and DCA State fee	Administration Fee	
Code Enforcement Fine	Express Fee ( 25.00)	
Certificate of Completion	Public Works Fee	
Construction Sign Fee	Landscape Review Fee (175.00 per hour)	
Roll-off Waste Container Fee (105.00 per container site)	Special Review Fee (89.25 per hour)	
Rework Fee	Other	

**PLAN REVIEWER APPROVAL AREA (OFFICE USE ONLY)**

SECTION	REVIEWER APPROVAL'S NAME	APPROVAL DATE	REJECTED DATE NUMBER 1	REJECTED DATE NUMBER 2	REJECTED DATE NUMBER 3
COMMUNITY DEVELOPMENT					
ZONING					
ELECTRICAL					
MECHANICAL					
PLUMBING					
FIRE					
ROOFING					
PUBLIC WORKS					
PLANNING					
STRUCTURAL	<i>JMC</i>	<i>9/16/2014</i>			
BUILDING	<i>JAL</i>	<i>9/17/14</i>			
BUILDING OFFICIAL					

**REWORKS:** A fee of \$105.50 may be charged for failure to make required corrections previously indicated. The fee shall be charged after the initial review plus one follow up review per trade. Please note that Florida Statutes 553.80 section 2(b) states "with respect to evaluation of design professionals documents, if a local government finds it necessary, in order to enforce compliance with the Florida Building Code and issue a permit, to reject design documents required by the code three or more times for failure to correct a code violation specifically noted in each rejection, including but not limited to egress, fire protection, structural stability, energy, accessibility, lighting, ventilation, electrical, mechanical, plumbing, and gas systems, or other requirements identified by rule of the Florida Building Commission adopted pursuant to Chapter 120, the local government shall impose, each time after third such review the plans are rejected for that code violation a fee of **four times the amount** of the proportion of the permit fee attributed to plans review".

**ISSUING OFFICIAL**

FINAL PLAN REVIEWED AND PREPARED FOR ISSUANCE BY: \_\_\_\_\_

DATE: *9/17/14*

**CONDITIONS OF APPROVAL**

**PLAN TRACKING**

Plans Checked out	Date	Clerk	Plans Checked in	Date	Clerk



# NOTICE OF COMMENCEMENT

A RECORDED COPY MUST BE POSTED ON THE JOB SITE AT TIME OF FIRST INSPECTION

PERMIT NO. \_\_\_\_\_ TAX FOLIO NO. \_\_\_\_\_

STATE OF FLORIDA:  
COUNTY OF MIAMI-DADE:

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

Space above reserved for use of recording office

1. Legal description of property and street/address: \_\_\_\_\_
2. Description of improvement: \_\_\_\_\_
3. Owner(s) name and address: \_\_\_\_\_  
Interest in property: \_\_\_\_\_  
Name and address of fee simple titleholder: \_\_\_\_\_
4. Contractor's name, address and phone number: \_\_\_\_\_
5. Surety: (Payment bond required by owner from contractor, if any)  
Name, address and phone number: \_\_\_\_\_  
Amount of bond \$: \_\_\_\_\_
6. Lender's name and address: \_\_\_\_\_
7. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a)7., Florida Statutes,  
Name, address and phone number: \_\_\_\_\_
8. In addition to himself, Owners designates the following person(s) to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes.  
Name, address and phone number: \_\_\_\_\_
9. Expiration date of this Notice of Commencement: \_\_\_\_\_  
(the expiration date is 1 year from the date of recording unless a different date is specified)

**WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART 1, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.**

Signature(s) of Owner(s) or Owner(s)' Authorized Officer/Director/Partner/Manager

By \_\_\_\_\_ By \_\_\_\_\_  
Print Name \_\_\_\_\_ Print Name \_\_\_\_\_  
Title/Office \_\_\_\_\_ Title/Office \_\_\_\_\_

STATE OF FLORIDA  
COUNTY OF MIAMI-DADE

The foregoing instrument was acknowledged before me this \_\_\_\_\_ day of \_\_\_\_\_

By \_\_\_\_\_

Individually, or  as \_\_\_\_\_ for \_\_\_\_\_  
 Personally known, or  produced the following type of identification: \_\_\_\_\_

Signature of Notary Public: \_\_\_\_\_  
Print Name: \_\_\_\_\_  
(SEAL)

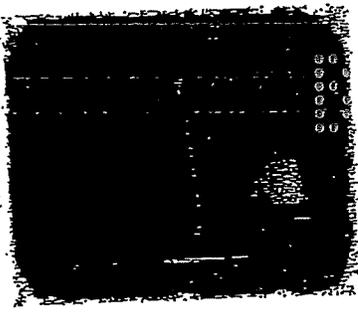
### VERIFICATION PURSUANT TO SECTION 92.525, FLORIDA STATUTES

Under penalties of perjury, I declare that I have read the foregoing and that the facts stated in it are true, to the best of my knowledge and belief.

Signature(s) of Owner(s) or Owner(s)'s Authorized Officer/Director/Partner/Manager who signed above:

By \_\_\_\_\_ By \_\_\_\_\_

# Miami-Dade County Clerk's Office



## COUNTY RECORDER

Business Hours: 9:00 a.m. - 4:00 p.m.  
(305) 275-1155, press 6

22 N.W. 1st Street  
Miami, Florida 33128

Harvey Ruvin, Clerk

### Records Searches

All recorded documents can be researched in the records library by the party's name, last name first (ex: Public, John Q), Clerk's File Number (CFN), legal description (subdivision name, lot, block, plat book and page) and Official Records Book and Page. The Records staff can perform the search of the records, or the public can be assisted with the search. The fee for searches is \$2.00 per name per year.

You can also perform a search via the internet for most recorded documents. Earlier documents are in the process of being made available on the web. A Premier service is available for a fee and enables more complex searches.

The web address is:  
[www.miamidade.gov/public-records](http://www.miamidade.gov/public-records)

### Search Request by Mail

If you would like to request a search by mail, you need to include the full name you would like searched and the legal description or address, if you are looking for a deed or mortgage.

If you would like a search for a satisfaction of mortgage or lien, please send the party's name and the original book and page of the mortgage or lien.

We do not perform any searches over the telephones.

### Important Notice:

The various credit bureaus search for liens and judgments recorded in our offices and link these documents to your credit history. If you have been advised by an agency that you have an item recorded against your name and you do not know what this item is, you may obtain the book and page from them to request a copy of the item from the Records Library by mail or the web.

Americans With Disabilities Act of 1990: TDD/TTY users may contact the telephone number(s) listed via the Florida Relay Service at 1-800-955-8771, or by dialing 711. This document is available in alternate formats upon request.

CLK/CT 905 Rev. 10/10

### How do I record a document?

You have three options for recording your documents in the Official Records;

1. You can bring your original documents in person, along with the appropriate fees, to the main office located at the Courthouse East Building  
Recording Department  
22 NW, 1st Street, First floor  
Miami, Florida 33128

2. You can mail your original documents along with the appropriate fees. Please mail documents for recording to the address listed below:  
Miami-Dade County Recorder  
P.O. Box 011711  
Flagler Station  
Miami, Florida 33101

3. You can eRecord your document through one of our approved vendors. With this option, you retain your original document and the recorded information is available the next day. Additional information on this option, including links to the vendors, is available on the web at [www.miamidadeclerk.com/Recording.asp](http://www.miamidadeclerk.com/Recording.asp).

### Records Library

Courthouse East Building, 2nd floor  
Copies of all recorded documents, along with the indexes, are maintained in the Records Library.

Recorded documents consist of: deeds, mortgages, satisfactions, releases and assignments of mortgages, claims of liens, UCC's (financial statements), affidavits, judgments, as well as civil court orders.

The cost for obtaining a copy of a document is \$1.00 per page and \$2.00 to certify. This can be done by mail, walk-in or via the web.

Certified Copies can be ordered via the web. This option is available after the image is displayed.

The website address is:  
[www.miamidade.gov/public-records](http://www.miamidade.gov/public-records)

# Office of the County Recorder

The responsibilities of the Clerk are established by the State Constitution, the Florida State Statutes, County Ordinances, and Regulations and Judicial Orders. In this capacity he serves as:

- Ex-Officio Clerk of the Board of County Commissioners
- County Recorder
- County Auditor
- Custodian of all County Funds
- Custodian of all records filed with the Court

In his role as County Recorder, the Clerk shall record specific kinds of instruments presented for recording, upon payment of the service charges prescribed by law. These documents are presented for recording by the public as well as the court system and include, but are not limited to: deeds, mortgages, liens, affidavits, subdivision plats, judgments, declarations of domicile, satisfactions and releases, powers of attorney and financing statements. Fees, Mortgage Taxes and Real Estate Transfer Taxes are computed and collected prior to recording.

The Clerk of the Circuit Court is the official recorder of all instruments recorded in the county pursuant to Chapter 28 of the Florida Statutes. The Clerk is required to record all instruments in one general series of books called the "Official Records."

The Recording Division of the Clerk's office is responsible for the recording, protecting, preserving, and disseminating of Miami-Dade County's official records. This is designed to create a permanent record of these important documents.

## Requirements for Recording:

There are several requirements that a document must have before being accepted for recording. All documents must be original or court certified. They must be signed and notarized.

## Frequently Recorded Documents and their requirements

1. **Deed:**
  - Original document
  - Miami-Dade County Property Only
  - Address of both parties (Grantor & Grantee)
  - Must be signed by seller
  - Two witnesses to the signature
  - Notarized and property acknowledged
  - Must have a prepared by statement
  - Prepared by: Individual name & address
  - Documentary Stamps at the rate of .60 cents for each \$100 of consideration or fractional part thereof. (\$6.00 per \$1,000).
  - Surtax is required on transfers of interest in any real property other than a single-family residence, at the rate of .45 cents for each \$100 of consideration or fractional part thereof (or \$4.50 per \$1,000).
2. **Mortgage**
  - An original document
  - Signed and notarized.
  - A copy of promissory note must be attached or the amount of the note should be written on the face of the Mortgage.
  - Signature of the borrower.
  - Addresses of both lender and borrower.
  - Prepared by: Individual name and address.
  - Florida documentary stamps and intangible tax.
  - Documentary stamps at the rate of .35 cents on each \$100 or fraction thereof on the amount of the note (or \$3.50 per \$1,000).
  - Intangible tax is required on real estate mortgages at the rate of 2 mills (or \$2 per \$1000) on the amount of the note.
3. **Satisfaction of Mortgage or Release**
  - Book and page of the original mortgage that is being satisfied or released.
  - Signature by the holder of the mortgage.
  - Notarized and properly acknowledged.
  - Prepared by: Individual name and address.

## 4. Assignment or mortgage

- The book and page of the mortgage that is being assigned.
- Signature of the assignor.
- Notarized
- Prepared by: Individual name and address

## 5. Claim of Lien

- Original document
- Signed by the Lienor and notarized.
- Legal description or property address
- Prepared by: Individual name and address

## 6. Declaration of Domicile

- Original document
- Signed and notarized
- Recording fee of \$10.00
- Copies are \$1.00 per page plus \$2.00 for the certification.

## 7. Notice of Commencement

- Original document
- Signed by owner and notarized
- Prepared by: Individual name and address

## Fees:

The acceptable methods of payment are: cashier's check, money order, certified check or credit card (in person only) Master Card or Visa.

Checks should be made payable to: Clerk of the Circuit Court.

The Recording fee for any document is \$10.00 for the first page and \$8.50 for each additional page. Copies are \$1.00 per page plus \$2.00 for the certification.

When presenting or mailing documents for recording, please submit/enclose self-addressed stamped envelope for the return of the original recorded document.

## Our mailing address is:

Office of County Recorder  
Recording Department  
P.O. Box 01-1711, Flagler Station  
Miami, Florida 33101



Dade Truss Company, Inc.  
 6401 NW 74th Avenue  
 Miami, FL 33166  
 Tel: (305) 592-8245  
 Fax: (305) 477-8092

Re : T407-75346: HERSKOWITZ

Site Information: Project Customer: CHELLE CONSTRUCTION  
 Lot/Block:  
 Site Name:  
 Site Address: 6745 139 STREET  
 St: Zip: PALMETTO BAY

Job Name: HERSKOWITZ ADDITION  
 Model: Set Of Trusses

**General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):**

Design Code: FRC2010/TPI2007 Design Program: 7.51 Jan 20 2014 Roof Load: 0.0 psf Floor Load: N/A  
 Wind Code: MWFRS(Directional)/C-C hybrid Wind ASCE 7-10 Wind Speed: 176 mph

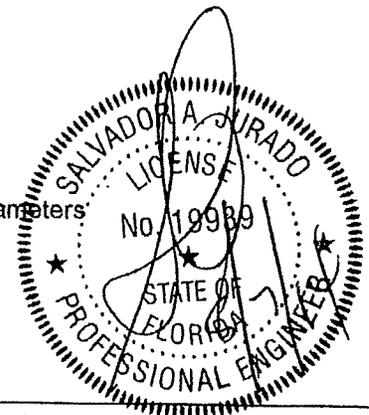
This package includes 14 individual, dated Truss Design Drawings and 0 Additional Drawings.  
 With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Date	Truss ID#	Seal#
1	08/05/14	CJ9	A0039102
2	08/05/14	H1	A0039103
3	08/05/14	H2	A0039104
4	08/05/14	HG1	A0039105
5	08/05/14	J1	A0039106
6	08/05/14	J3	A0039107
7	08/05/14	J5	A0039108
8	08/05/14	J7	A0039109
9	08/05/14	T1	A0039110
10	08/05/14	T1A	A0039111
11	08/05/14	V10	A0039112
12	08/05/14	V14	A0039113
13	08/05/14	V18	A0039114
14	08/05/14	V6	A0039115

The truss drawing(s) referenced above have been prepared using MiTek Industries, Inc. engineering software under my direct supervision based on the parameters provided by Dade Truss Company, Inc. of Miami, FL.

Truss Design Engineer's Name: Salvador A Jurado P.E.  
 STATE OF FLORIDA REG. NO 19939 - 6401 NW 74TH AVE, MIAMI, FL 33166

NOTE: The seal on these drawings indicate acceptance of professional engineering responsibility for the truss components shown per ANSI/TPI 1-2007 Section 2.



Job 29875-1	Truss CJ9	Truss Type CORNER JACK	Qty 2	Ply 1	CHELLE CONST,HERSKOWITZ ADDITION A0039102
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Job Reference (optional)  
7.510 s 5 Jan 20 2014 MiTek Industries, Inc. Tue Aug 05 16:46:26 2014 Page 1  
ID:UR1HQWE65MRpZ4QKsmTPLY97ctL-xJf1r1LdQ9DP7ayTeHQ\_ji1LMTo3FRTRFLyqs7R

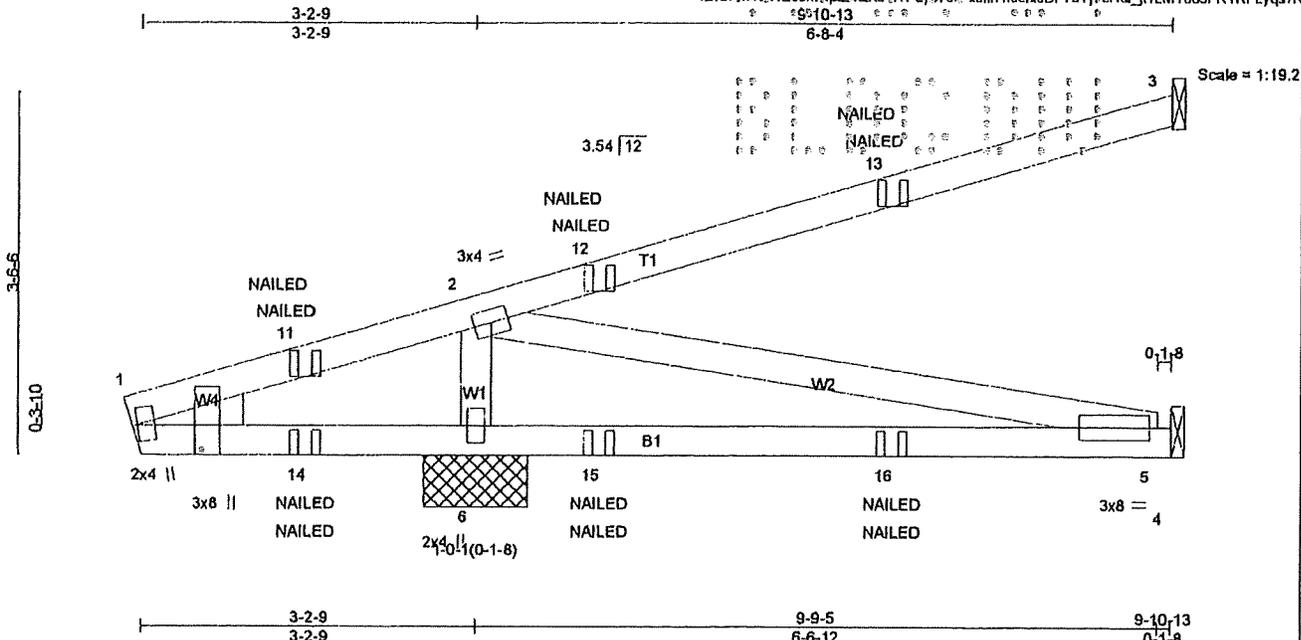


Plate Offsets (X,Y): [1:0-3-6,Edge], [1:0-2-0,0-0-9], [5:0-2-6,0-1-8]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 30.0	Plates Increase 1.33	TC 0.47	Vert(LL) -0.05	5-6	>999	240	MT20	244/190
TCDL 15.0	Lumber Increase 1.33	BC 0.37	Vert(TL) 0.07	5-6	>999	180		
BCLL 0.0	Rep Stress Inor NO	WB 0.10	Horz(TL) -0.01	3	n/a	n/a		
BCDL 10.0	Code FRC2010/TPI2007	(Matrix-M)					Weight: 43 lb	FT = 0%

**LUMBER**  
TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.2  
WEBS 2x4 SP No.3  
WEDGE  
Left: 2x4 SP No.3  
**BRACING**  
TOP CHORD  
Structural wood sheathing directly applied or 6-0-0 oc purlins.  
BOT CHORD  
Structural wood sheathing directly applied or 6-0-0 oc bracing.  

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

**TOP CHORD**  
1-11=0/316, 2-11=0/322  
**BOT CHORD**  
1-14=-299/0, 6-14=-299/0,  
6-15=-325/0, 15-16=-325/0,  
5-16=-325/0  
**WEBS**  
2-6=-351/187, 2-5=0/331

**NOTES**  
1) Wind: ASCE 7-10; Vult=175mph (3-second gust) Vasd=136mph; HVHZ; TCDL=9.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional); cantilever left exposed; Lumber DOL=1.60 plate grip DOL=1.60  
2) This truss is not designed to support a ceiling and is not intended for use where aesthetics are a consideration.  
3) Plates checked for a plus or minus 0 degree rotation about its center.  
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.  
5) Refer to girder(s) for truss to truss connections.  
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5 except (jt=lb) 3=130, 6=340.  
7) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.  
8) "NAILED" indicates 3-10d (0.148"x3") or 2-12d (0.148"x3.25") toe-nails. For more details refer to MiTek's ST-TOENAIL Detail.

9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).  
**LOAD CASE(S)**  
Standard  
1) Dead + Roof Live (balanced): Lumber Increase=1.33, Plate Increase=1.33  
Uniform Loads (plf)  
Vert: 1-3=-90, 4-7=-20  
Concentrated Loads (lb)  
Vert: 11=188(F=94, B=94) 12=198(F=99, B=99) 13=102(F=51, B=51) 14=2(F=1, B=1) 15=186(F=93, B=93) 16=53(F=27, B=27)

**REACTIONS (lb/size)**

3	=	176/Mechanical
6	=	203/1-0-1
5	=	-14/Mechanical
Max Horz		
6	=	188(LC 8)
Max Uplift		
3	=	-130(LC 8)
6	=	-340(LC 6)
5	=	-54(LC 14)
Max Grav		
3	=	176(LC 1)
6	=	203(LC 1)
5	=	38(LC 3)

**FORCES (lb)**  
Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

Job 29875-1	Truss H1	Truss Type HIP TR	Qty 1	Ply 1	CHELLE CONST./HERSKOWITZ ADDITION A0039103
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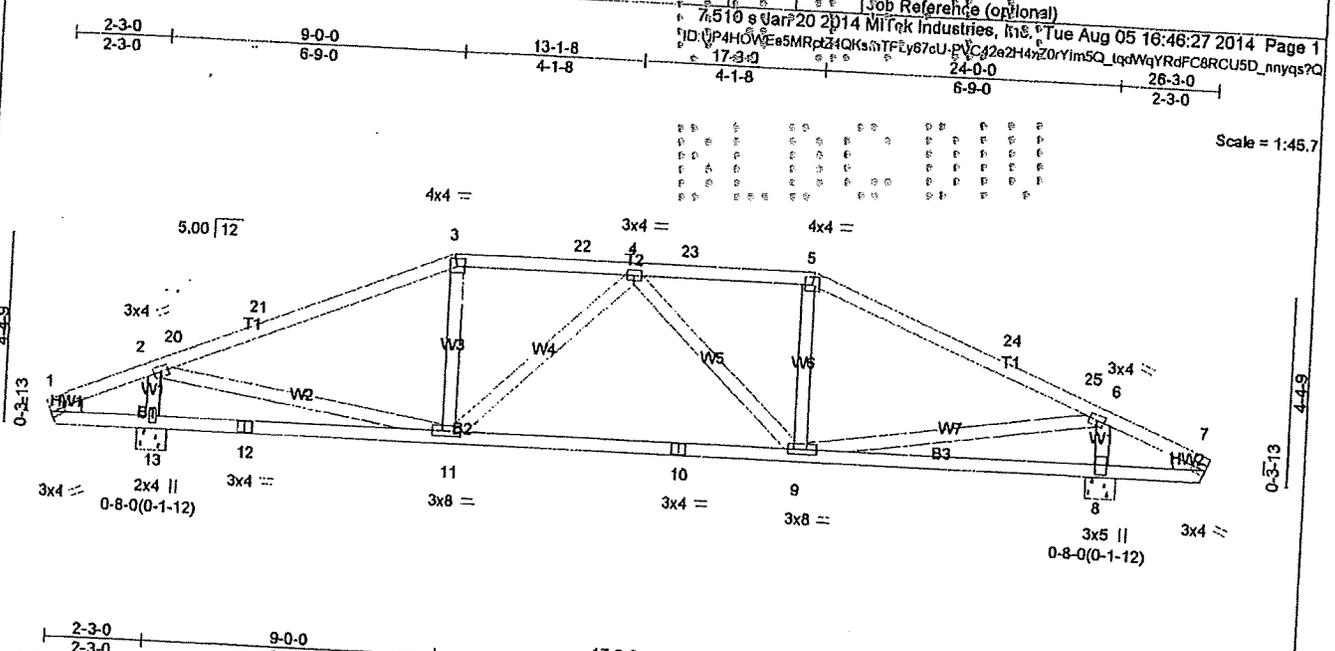


Plate Offsets (X,Y): [3:0-2:0,0-2:4], [5:0-2:0,0-2:4], [6:0-2:12,0-1:8], [9:0-1:8,0-1:8], [11:0-3:0,0-1:8]	24-0-0 6-9-0	26-3-0 2-3-0
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<b>LOADING (psf)</b> TCLL 30.0 TCDL 15.0 BCLL 0.0 BCDL 10.0	<b>SPACING</b> Plates Increase 1.33 Lumber Increase 1.33 Rep Stress Incr YES Code FRC2010/TP12007	<b>CSI</b> TC 0.73 BC 0.59 WB 0.57 (Matrix-M)	<b>DEFL</b> Vert(LL) 0.10 Vert(TL) -0.26 Horz(TL) 0.03	<b>PLATES</b> MT20	<b>GRIP</b> 244/190	Weight: 134 lb FT = 0%
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**LUMBER**  
TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.2  
WEBS 2x4 SP No.3  
WEDGE  
Left: 2x4 SP No.3,  
Right: 2x4 SP No.3  
**BRACING**  
TOP CHORD  
Structural wood sheathing directly applied or 3-0-9 oc  
purlins.  
BOT CHORD  
Structural wood sheathing directly applied or 10-0-0  
oc bracing, Except:  
7-6-8 oc bracing: 9-11.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

<b>REACTIONS (lb/size)</b>	
13 =	1467/0-8-0
8 =	1467/0-8-0
Max Horz	
13 =	-162(LC 8)
Max Uplift	
13 =	-567(LC 10)
8 =	-449(LC 10)
Max Grav	
13 =	1467(LC 1)
8 =	1467(LC 1)

**FORCES (lb)**  
Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
**TOP CHORD**  
2-20=-1609/737, 20-21=-1587/754,

**TOP CHORD**  
2-20=-1609/737, 20-21=-1587/754,  
3-21=-1480/784, 3-22=-1366/818,  
4-22=-1366/818, 4-23=-1372/817,  
5-23=-1372/817, 5-24=-1484/784,  
24-25=-1591/755, 6-25=-1613/737  
**BOT CHORD**  
12-13=-126/337, 11-12=-126/337,  
10-11=-602/1541, 9-10=-602/1541  
**WEBS**  
2-13=-1360/943, 2-11=-597/1233,  
4-11=-365/175, 6-8=-1357/956,  
4-9=-358/178, 6-9=-616/1232

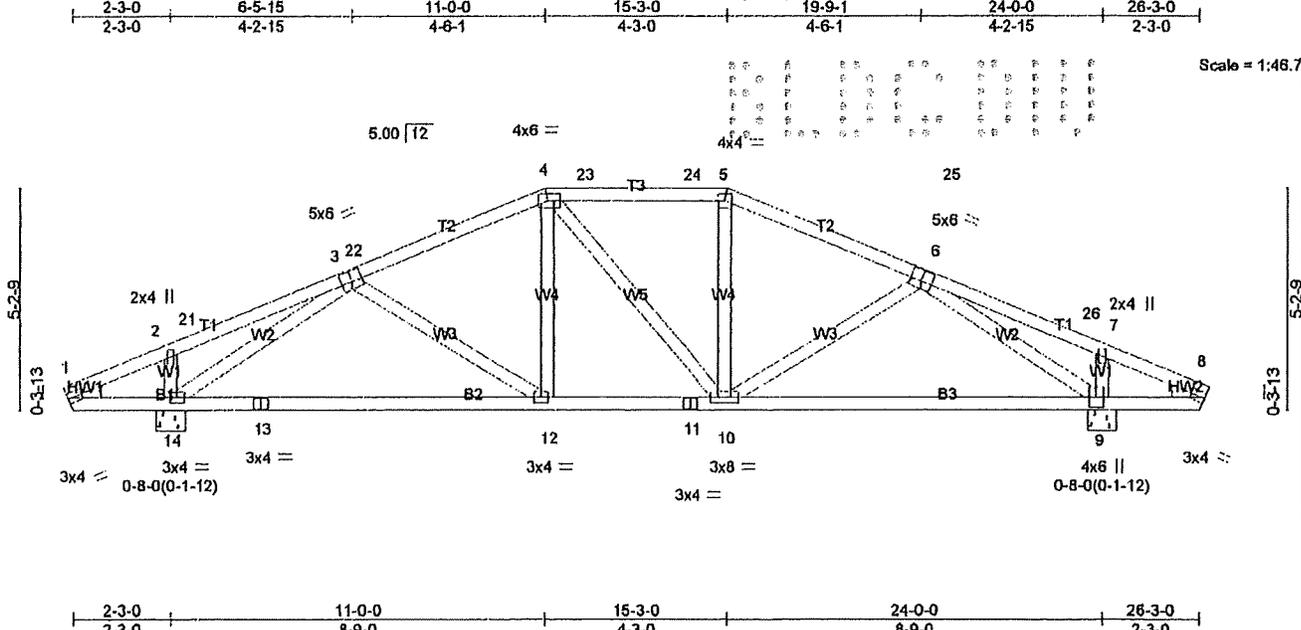
**NOTES**  
1) Unbalanced roof live loads have been considered for this design.  
2) Wind: ASCE 7-10; Vult=175mph (3-second gust) Vasd=136mph; HVHZ; TCDL=9.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=26ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) 0-1-5 to 2-10-11, Interior(1) 2-10-11 to 4-9-7, Exterior(2) 4-9-7 to 21-5-9, Interior(1) 21-5-9 to 23-4-5 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60  
3) Provide adequate drainage to prevent water ponding.  
4) Plates checked for a plus or minus 0 degree rotation about its center.  
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 13=567, 8=449.  
7) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

**LOAD CASE(S)**  
Standard

Job 29875-1	Truss H2	Truss Type HIP TR	Qty 1	Ply 1	CHELLE CONST./HERSKOWITZ ADDITION A0039104
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Job Reference (optional)  
7.510 e Jan 20 2014 Mitek Industries, Inc. r1ue Augr05 16:46:29 2014 Page 1  
ID:UP4HOWE5MHPZ4CK3nTPLy67U-MuKqTKYyPk4ss9DrILv2bGhFlJgzXVxPI5sgyqs7C



LOADING (psf)		SPACING 2-0-0	CSI	DEFL in (loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plates Increase 1.33	TC 0.37	Vert(LL) -0.13 12-14	>999	240	MT20	244/190
TCDL	15.0	Lumber Increase 1.33	BC 0.68	Vert(TL) -0.35 12-14	>743	180		
BCLL	0.0	Rep Stress Incr YES	WB 0.86	Horz(TL) 0.05 9	n/a	n/a		
BCDL	10.0	Code FRC2010/TPI2007	(Matrix-M)					
							Weight: 140 lb	FT = 0%

**LUMBER**  
TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.2  
WEBS 2x4 SP No.3  
WEDGE  
Left: 2x4 SP No.3,  
Right: 2x4 SP No.3  
**BRACING**  
TOP CHORD  
Structural wood sheathing directly applied or 4-6-15 oc purlins.  
BOT CHORD  
Structural wood sheathing directly applied or 6-0-0 oc bracing.

**REACTIONS (lb/size)**  
14 = 1467/0-8-0  
9 = 1467/0-8-0  
Max Horz  
14 = -197(LC 8)  
Max Uplift  
14 = -567(LC 10)  
9 = -449(LC 10)  
Max Grav  
14 = 1467(LC 1)  
9 = 1467(LC 1)

**FORCES (lb)**  
Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
TOP CHORD  
1-2=-410/103, 2-21=-277/0,

**TOP CHORD**  
1-2=-410/103, 2-21=-277/0,  
3-21=-251/119, 3-22=-1429/714,  
4-22=-1417/746, 4-23=-1242/758,  
23-24=-1242/758, 5-24=-1242/758,  
5-25=-1417/744, 6-25=-1429/713,  
6-26=-273/118, 7-26=-299/0,  
7-8=-433/102  
**BOT CHORD**  
1-14=-41/395, 13-14=-449/1283,  
12-13=-449/1283, 11-12=-365/1242,  
10-11=-365/1242, 9-10=-443/1261,  
8-9=-40/421  
**WEBS**  
2-14=-336/309, 3-14=-1603/1046,  
6-9=-1602/1064, 7-9=-336/311

**NOTES**  
1) Unbalanced roof live loads have been considered for this design.  
2) Wind: ASCE 7-10; Vult=175mph (3-second gust) Vasd=136mph; HVHZ; TCDL=9.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=26ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) 0-1-5 to 2-10-11, Interior(1) 2-10-11 to 6-9-7, Exterior(2) 6-9-7 to 19-5-9, Interior(1) 19-5-9 to 23-4-5 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60  
3) Provide adequate drainage to prevent water ponding.  
4) Plates checked for a plus or minus 0 degree rotation about its center.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.  
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 14=567, 9=449.  
7) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

**LOAD CASE(S)**  
Standard

Job 29875-1	Truss HG1	Truss Type HIP GIRDER TR	Qty 1	Ply 1	CHELLE CONST./HERSKOWITZ ADDITION A0039105
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Job Reference (optional)  
7.510 s Jan 20 2014 MITek Industries, Inc. The Aug 05 16:46:31 2014 Page 1  
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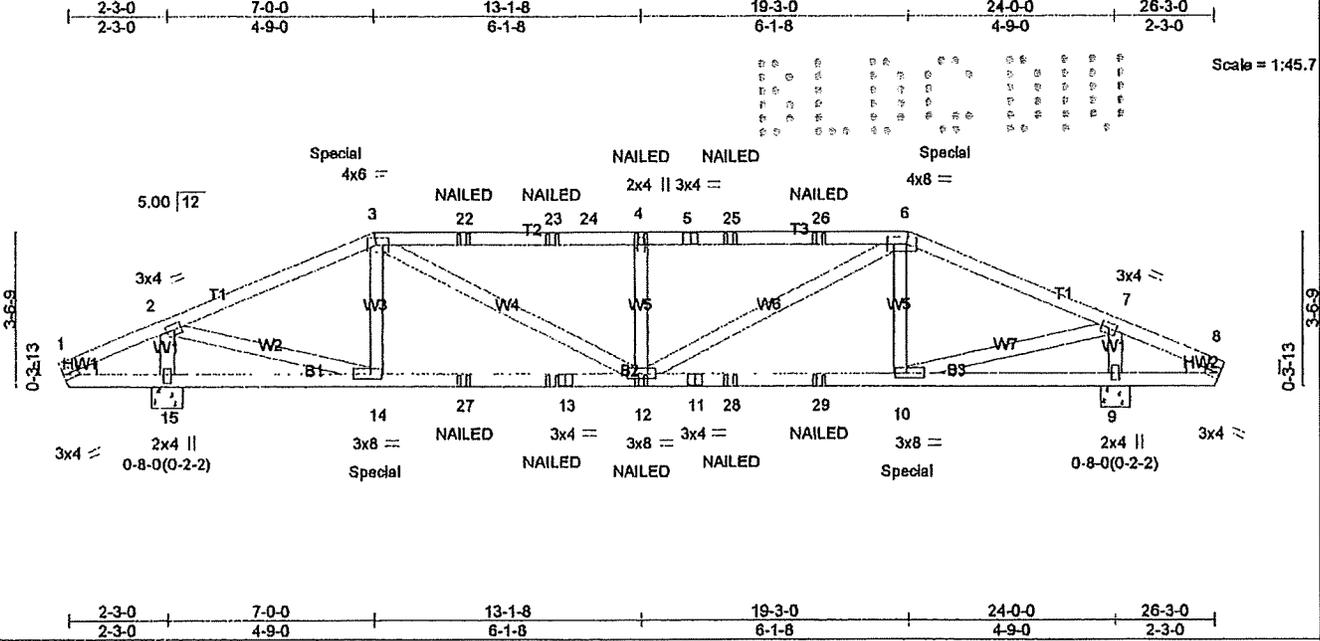


Plate Offsets (X,Y): [3:0-3-8,0-2-0], [6:0-5-4,0-2-0], [10:0-3-0,0-1-8], [14:0-3-4,0-1-8], [15:0-2-8,0-1-0]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 30.0	Plates Increase 1.33	TC 0.53	Vert(LL) 0.13	12	>999	240	MT20	244/190
TCDL 15.0	Lumber Increase 1.33	BC 0.63	Vert(TL) -0.25	12-14	>999	180		
BCLL 0.0	Rep Stress Incr NO	WB 0.72	Horz(TL) 0.04	9	n/a	n/a		
BCDL 10.0	Code FRC2010/TPI2007	(Matrix-M)					Weight: 132 lb	FT = 0%

**LUMBER**  
**TOP CHORD** 2x4 SP No.2 \*Except\*  
 T2,T3: 2x4 SP 2250F 1.9E  
**BOT CHORD** 2x4 SP No.2  
**WEBS** 2x4 SP No.3  
**WEDGE**  
 Left: 2x4 SP No.3,  
 Right: 2x4 SP No.3  
**BRACING**  
**TOP CHORD**  
 Structural wood sheathing directly applied or 3-9-13  
 oc purlins.  
**BOT CHORD**  
 Structural wood sheathing directly applied or 6-0-0 oc  
 bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

**REACTIONS (lb/size)**

15	=	1805/0-8-0
9	=	1805/0-8-0
Max Horz	=	-127(LC 6)
Max Uplift	=	-871(LC 8)
9	=	-751(LC 8)
Max Grav	=	1805(LC 1)
15	=	1805(LC 1)
9	=	1805(LC 1)

**FORCES (lb)**  
 Max. Comp./Max. Ten. - All forces 250 (lb) or less  
 except when shown.  
**TOP CHORD**  
 2-3=-2079/941, 3-22=-2808/1329,  
 22-23=-2808/1329,  
 23-24=-2808/1329,  
 4-24=-2808/1329, 4-5=-2808/1329,  
 5-25=-2808/1329,  
 25-26=-2808/1329,  
 6-26=-2808/1329, 6-7=-2102/978  
**BOT CHORD**  
 14-27=-729/1915, 13-27=-729/1915,  
 12-13=-729/1915,  
 11-12=-744/1857, 11-28=-744/1857,  
 28-29=-744/1857,  
 10-29=-744/1857  
**WEBS**  
 2-15=-1711/846, 2-14=-888/2018,  
 3-14=-412/271, 3-12=-471/1163,  
 4-12=-902/538, 6-12=-444/1158,  
 6-10=-401/259, 7-10=-861/2027,  
 7-9=-1705/789

**TOP CHORD**  
 2-3=-2079/941, 3-22=-2808/1329,  
 22-23=-2808/1329,  
 23-24=-2808/1329,  
 4-24=-2808/1329, 4-5=-2808/1329,  
 5-25=-2808/1329,  
 25-26=-2808/1329,  
 6-26=-2808/1329, 6-7=-2102/978  
**BOT CHORD**  
 14-27=-729/1915, 13-27=-729/1915,  
 12-13=-729/1915,  
 11-12=-744/1857, 11-28=-744/1857,  
 28-29=-744/1857,  
 10-29=-744/1857  
**WEBS**  
 2-15=-1711/846, 2-14=-888/2018,  
 3-14=-412/271, 3-12=-471/1163,  
 4-12=-902/538, 6-12=-444/1158,  
 6-10=-401/259, 7-10=-861/2027,  
 7-9=-1705/789

**NOTES**  
 1) Unbalanced roof live loads have been considered for this design.  
 2) Wind: ASCE 7-10; Vult=175mph (3-second gust) Vasd=136mph; HVHZ; TCDL=9.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=26ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional); cantilever left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60  
 3) Provide adequate drainage to prevent water ponding.  
 4) Plates checked for a plus or minus 0 degree rotation about its center.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.  
 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 15=871, 9=751.  
 7) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.  
 8) "NAILED" Indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails. For more details refer to MiTek's ST-TOENAIL Detail.  
 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 323 lb down and 342 lb up at 7-0-6, and 323 lb down and 342 lb up at 19-2-10 on top chord, and 114 lb down and 99 lb up at 7-0-0, and 114 lb down and 99 lb up at 19-2-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.  
 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S)**  
 Standard  
 1) Dead + Roof Live (balanced); Lumber Increase=1.33, Plate Increase=1.33  
 Uniform Loads (plf)  
 Vert: 1-3=-90, 3-6=-90, 6-8=-90, 16-19=-20  
 Concentrated Loads (lb)

Job	Truss	Truss Type	Qty	Ply	CHELLE CONST,HERSKOWITZ ADDITION A0039105
29875-1	HG1	HIP GIRDER TR	1	1	Job Reference (optional)

7.510 s, Jan 20 2014 Mitek Industries, Inc. Tue Aug 05 16:46:31 2014 Page 2  
 ID:UP4HGWE95MRp24CKsmTFLy67cU-JSbu058A\$JA7XKCjP\_ThZc2\_S8uBoPjBBwYyqs7M

**LOAD CASE(S)**

Standard

Concentrated Loads (lb)

Vert: 3=-125(F) 6=-125(F) 13=-15(F) 14=21(F)  
 12=-15(F) 4=-78(F) 10=21(F) 22=-78(F)  
 23=-78(F) 25=-78(F) 26=-78(F) 27=-15(F)  
 28=-15(F) 29=-15(F)

7.510 s, Jan 20 2014 Mitek Industries, Inc. Tue Aug 05 16:46:31 2014 Page 2  
 ID:UP4HGWE95MRp24CKsmTFLy67cU-JSbu058A\$JA7XKCjP\_ThZc2\_S8uBoPjBBwYyqs7M



Job 29875-1	Truss J3	Truss Type JACK TR @ 45	Qty 4	Ply 1	CHELLE CONST./HERSKOWITZ ADDITION A0039107
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Job Reference (optional)  
 7.510 s Jan 20 2014 MITek Industries, Inc. Tue Aug 05 16:46:32 2014 Page 1  
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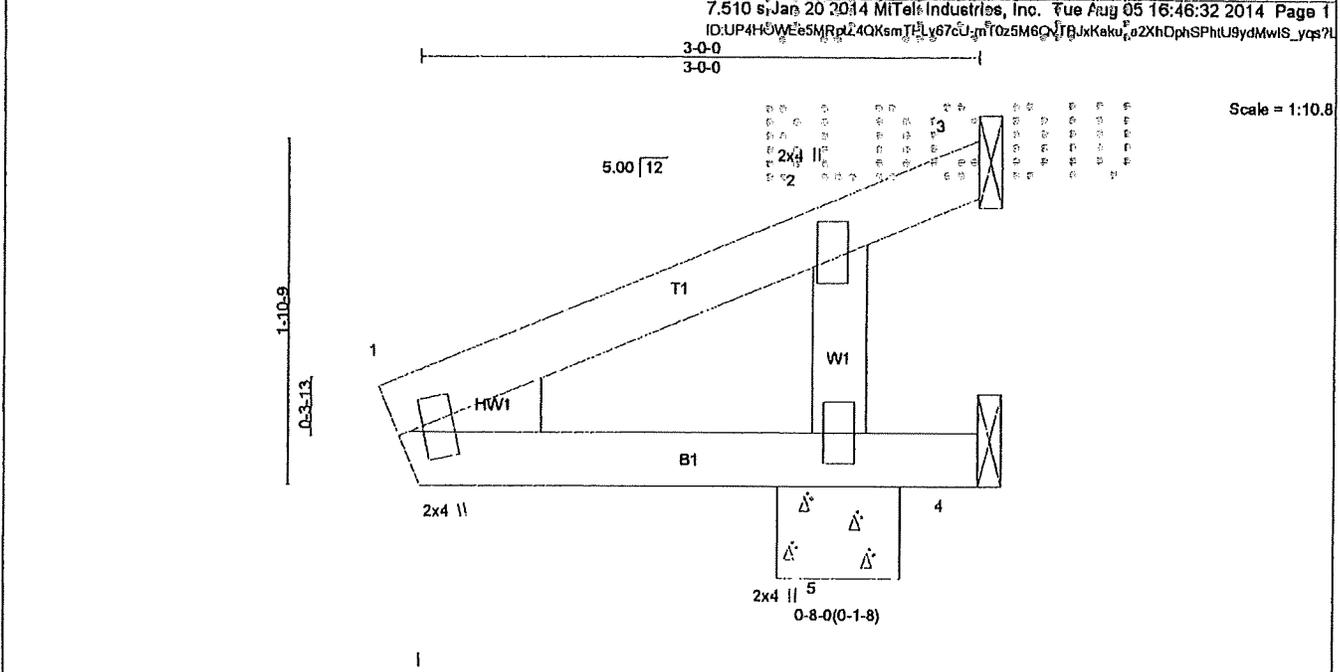


Plate Offsets (X,Y): [1:0-2-0,0-1-4], [2:0-3-0,0-0-4]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP	
TCLL 30.0	Plates Increase	1.33	TC 0.25	Vert(LL)	0.00	5	>999	240	MT20	244/190
TCDL 15.0	Lumber Increase	1.33	BC 0.31	Vert(TL)	0.00	5	>999	180		
BCLL 0.0	Rep Stress Incr	YES	WB 0.16	Horz(TL)	0.02	3	n/a	n/a		
BCDL 10.0	Code FRC2010/TPI2007		(Matrix-M)							
									Weight: 12 lb	FT = 0%

**LUMBER**  
 TOP CHORD 2x4 SP No.2  
 BOT CHORD 2x4 SP No.2  
 WEBS 2x4 SP No.3  
 WEDGE  
 Left: 2x4 SP No.3  
**BRACING**  
 TOP CHORD  
 Structural wood sheathing directly applied or 3-0-0 oc purlins.  
 BOT CHORD  
 Structural wood sheathing directly applied or 10-0-0 oc bracing.

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

**WEBS**  
 2-5=-356/534

**NOTES**  
 1) Wind: ASCE 7-10; Vult=175mph (3-second gust) Vasd=136mph; HVHZ; TC DL=9.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Encl., GCpl=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60  
 2) Plates checked for a plus or minus 0 degree rotation about its center.  
 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.  
 4) Refer to girder(s) for truss to truss connections.  
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 3=127, 4=139, 5=373.  
 6) "Semi-rigid pitchbreaks with fixed heels"  
 Member end fixity model was used in the analysis and design of this truss.

**REACTIONS (lb/size)**

3	=	-127/Mechanical
4	=	-139/Mechanical
5	=	612/0-8-0
Max Horz		
5	=	81(LC 10)
Max Uplift		
3	=	-127(LC 1)
4	=	-139(LC 1)
5	=	-373(LC 10)
Max Grav		
3	=	68(LC 10)
4	=	92(LC 10)
5	=	612(LC 1)

**LOAD CASE(S)**  
 Standard

**FORCES (lb)**  
 Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

Job 29875-1	Truss J5	Truss Type JACK TR @ 45	Qty 4	Ply 1	CHELLE CONST./HERSKOWITZ ADDITION A0039108
Job Reference (optional)					
7.510 s Jan 20 2014 MiTek Industries, Inc. Tue Aug 05 16:46:33 2014 Page 1					
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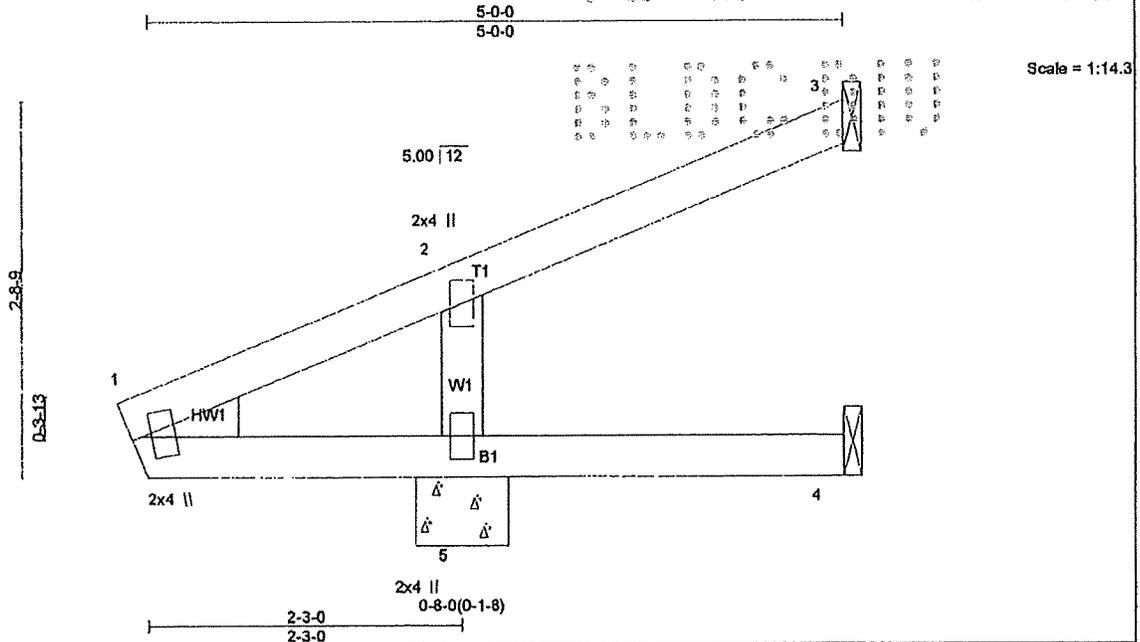


Plate Offsets (X,Y): [1:0-2-0,0-1-4]						
<b>LOADING (psf)</b>	<b>SPACING</b>	<b>CSI</b>	<b>DEFL</b>	<b>in (loc)</b>	<b>l/defl</b>	<b>L/d</b>
TCLL 30.0	Plates Increase 1.33	TC 0.25	Vert(LL) -0.01	4-5	>999	240
TCDL 15.0	Lumber Increase 1.33	BC 0.20	Vert(TL) 0.01	4-5	>999	180
BCLL 0.0	Rep Stress Incr YES	WB 0.15	Horz(TL) 0.02	3	n/a	n/a
BCDL 10.0	Code FRC2010/TPI2007	(Matrix-M)				
						<b>PLATES</b> MT20
						<b>GRIP</b> 244/190
						Weight: 19 lb FT = 0%

<b>LUMBER</b>	<b>TOP CHORD</b> 2x4 SP No.2	<b>TOP CHORD</b> 1-2=-268/107
<b>BOT CHORD</b> 2x4 SP No.2	<b>WEBS</b> 2x4 SP No.3	<b>BOT CHORD</b> 1-5=-97/266
<b>WEDGE</b>	Left: 2x4 SP No.3	<b>WEBS</b> 2-5=-344/496
<b>BRACING</b>	<b>TOP CHORD</b>	<b>NOTES</b>
	Structural wood sheathing directly applied or 5-0-0 oc purlins.	1) Wind: ASCE 7-10; Vult=175mph (3-second gust) Vasd=136mph; HVHZ; TCDL=9.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
	<b>BOT CHORD</b>	2) Plates checked for a plus or minus 0 degree rotation about its center.
	Structural wood sheathing directly applied or 10-0-0 oc bracing.	3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
	MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.	4) Refer to girder(s) for truss to truss connections.
<b>REACTIONS (lb/size)</b>		5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (l=lb) 5=253.
3 =	76/Mechanical	6) "Semi-rigid pitchbreaks with fixed heels"
4 =	-1/Mechanical	Member end fixity model was used in the analysis and design of this truss.
5 =	491/0-8-0	
Max Horz		
5 =	134(LC 10)	
Max Uplift		
3 =	-54(LC 10)	
4 =	-1(LC 1)	
5 =	-253(LC 10)	
Max Grav		
3 =	88(LC 15)	
4 =	28(LC 3)	
5 =	491(LC 1)	
<b>FORCES (lb)</b>		<b>LOAD CASE(S)</b>
Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.		Standard

Job 29875-1	Truss J7	Truss Type JACK TR	Qty 7	Ply 1	CHELLE CONST./HERSKOWITZ ADDITION A0039109
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Job Reference (optional)  
7.510 g Jan 20 2014 MiTek Industries, Inc. Tue Aug 05 16:46:33 2014 Page 1  
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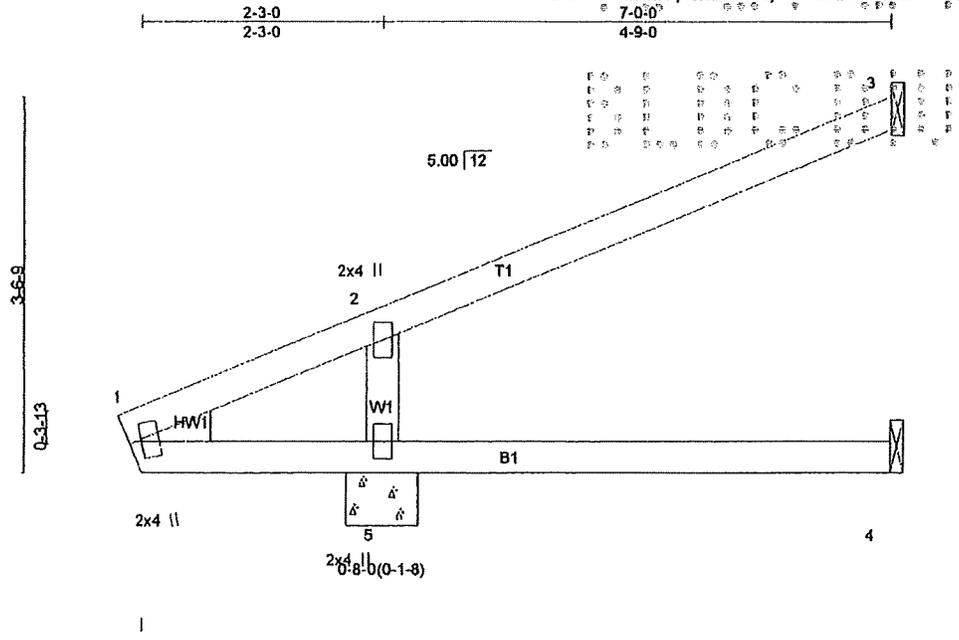


Plate Offsets (X,Y): [1:0-2-0,0-1-0]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 30.0	Plates Increase	1.33	TC 0.44	Vert(LL)	-0.02 4-5	>999	240	MT20	244/190
TCDL 15.0	Lumber Increase	1.33	BC 0.19	Vert(TL)	-0.04 4-5	>999	180		
BCLL 0.0	Rep Stress Incr	YES	WB 0.19	Horz(TL)	0.02 3	n/a	n/a		
BCDL 10.0	Code FRC2010/TPI2007		(Matrix-M)						
								Weight: 25 lb	FT = 0%

**LUMBER**  
 TOP CHORD 2x4 SP No.2  
 BOT CHORD 2x4 SP No.2  
 WEBS 2x4 SP No.3  
 WEDGE  
 Left: 2x4 SP No.3  
**BRACING**  
 TOP CHORD  
 Structural wood sheathing directly applied or 6-0-0 oc purlins.  
 BOT CHORD  
 Structural wood sheathing directly applied or 10-0-0 oc bracing.

TOP CHORD  
1-2=-382/149  
 BOT CHORD  
1-5=-127/352  
 WEBS  
2-5=-465/634

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

**NOTES**  
 1) Wind: ASCE 7-10; Vult=175mph (3-second gust) Vasd=136mph; HVHZ; TCDL=9.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) zone; cantilever left exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60  
 2) Plates checked for a plus or minus 0 degree rotation about its center.  
 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.  
 4) Refer to girder(s) for truss to truss connections.  
 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 3=110, 5=263.  
 6) "Semi-rigid pitchbreaks with fixed heels"  
 Member end fixity model was used in the analysis and design of this truss.

**REACTIONS (lb/size)**

3	=	168/Mechanical
4	=	35/Mechanical
5	=	583/0-8-0
Max Horz		
5	=	188(LC 10)
Max Uplift		
3	=	-110(LC 10)
5	=	-263(LC 10)
Max Grav		
3	=	176(LC 15)
4	=	74(LC 3)
5	=	583(LC 1)

**FORCES (lb)**  
 Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD  
 1-2=-382/149

**LOAD CASE(S)**  
 Standard

Job 29875-1	Truss T1	Truss Type COMMON TR	Qty 3	Ply 1	CHELLE CONST./HERSKOWITZ ADDITION A0039110
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Job Reference (optional)  
7.510 s Jan 20 2014 MiTek Industries, Inc. Tue Aug 05 16:46:35 2014 Page 1  
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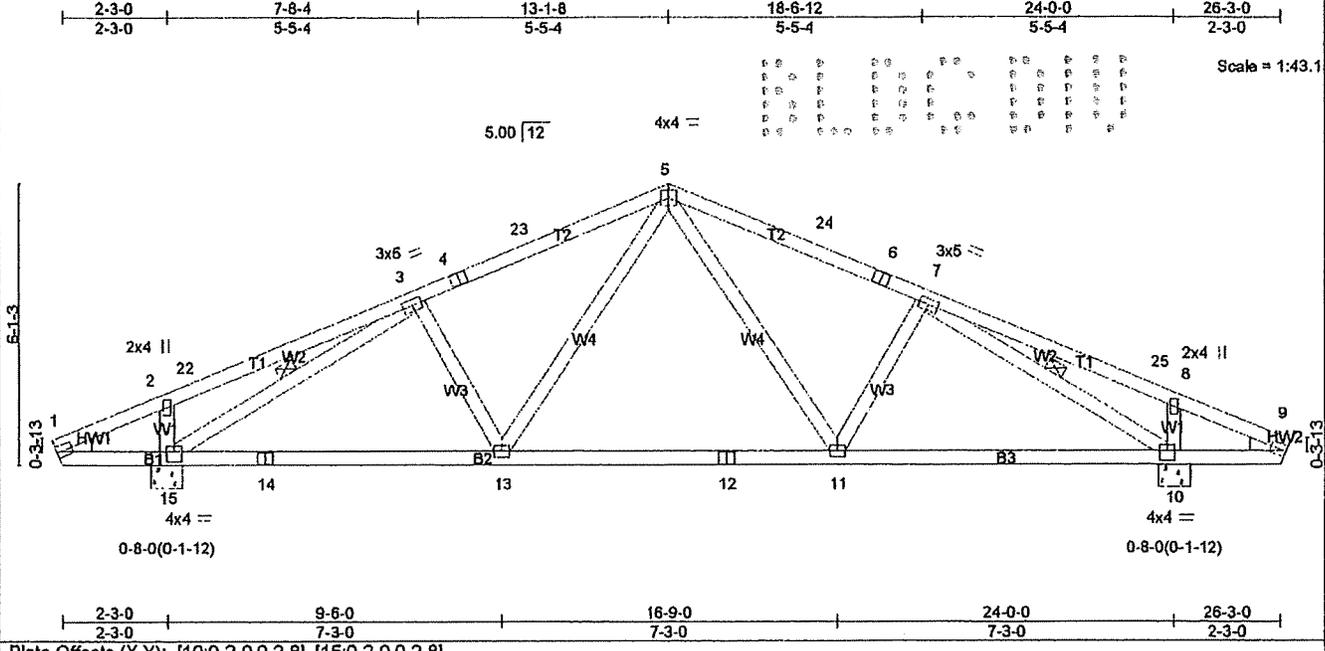


Plate Offsets (X,Y): [10:0-2-0,0-2-8], [15:0-2-0,0-2-8]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 30.0	Plates Increase	1.33	TC 0.47	Vert(LL)	0.09 11-13	>999	240	MT20	244/190
TCDL 15.0	Lumber Increase	1.33	BC 0.50	Vert(TL)	-0.18 11-13	>999	180		
BCLL 0.0	Rep Stress Incr	YES	WB 0.41	Horz(TL)	0.06 10	n/a	n/a		
BCDL 10.0	Code FRC2010/TPI2007		(Matrix-M)						
								Weight: 136 lb	FT = 0%

**LUMBER**  
TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.2  
WEBS 2x4 SP No.3  
WEDGE  
Left: 2x4 SP No.3,  
Right: 2x4 SP No.3  
**BRACING**  
TOP CHORD  
Structural wood sheathing directly applied or 4-4-15  
oc purlins.  
BOT CHORD  
Structural wood sheathing directly applied or 9-5-12  
oc bracing.  
WEBS  
1 Row at midpt  
7-10, 3-15

**TOP CHORD**  
1-2=-323/74, 3-4=-1487/659,  
4-23=-1400/672, 5-23=-1384/693,  
5-24=-1384/691, 6-24=-1400/670,  
6-7=-1487/656, 8-9=-344/74  
**BOT CHORD**  
1-15=0/308, 14-15=-374/1380,  
13-14=-374/1380, 12-13=-233/1087,  
11-12=-233/1087,  
10-11=-370/1365, 9-10=0/332  
**WEBS**  
5-11=-128/436, 7-11=-256/225,  
7-10=-1618/838, 8-10=-431/364,  
5-13=-106/436, 3-13=-255/208,  
3-15=-1618/820, 2-15=-431/362

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.  
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 10=449, 15=567.  
7) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

**LOAD CASE(S)**  
Standard

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

**REACTIONS (lb/size)**

10 =	1467/0-8-0
15 =	1467/0-8-0
Max Horz	
15 =	-234(LC 8)
Max Uplift	
10 =	-449(LC 10)
15 =	-567(LC 10)
Max Grav	
10 =	1467(LC 1)
15 =	1467(LC 1)

**NOTES**  
1) Unbalanced roof live loads have been considered for this design.  
2) Wind: ASCE 7-10; Vult=175mph (3-second gust) Vasd=136mph; HVHZ; TCDL=9.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=26ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) -0-1-5 to 2-10-11, Interior(1) 2-10-11 to 10-1-8, Exterior(2) 10-1-8 to 13-1-8, Interior(1) 16-1-8 to 23-4-5 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60  
3) All plates are 3x4 MT20 unless otherwise indicated.  
4) Plates checked for a plus or minus 0 degree rotation about its center.

**FORCES (lb)**  
Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

Job 29875-1	Truss T1A	Truss Type CB TR	Qty 1	Ply 1	CHELLE CONST,HERSKOWITZ ADDITION A0039111
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Job Reference (optional)  
7.510 s Jan 20 2014 MiTek Industries, Inc. Tue Aug 05 16:46:36 2014 Page 1  
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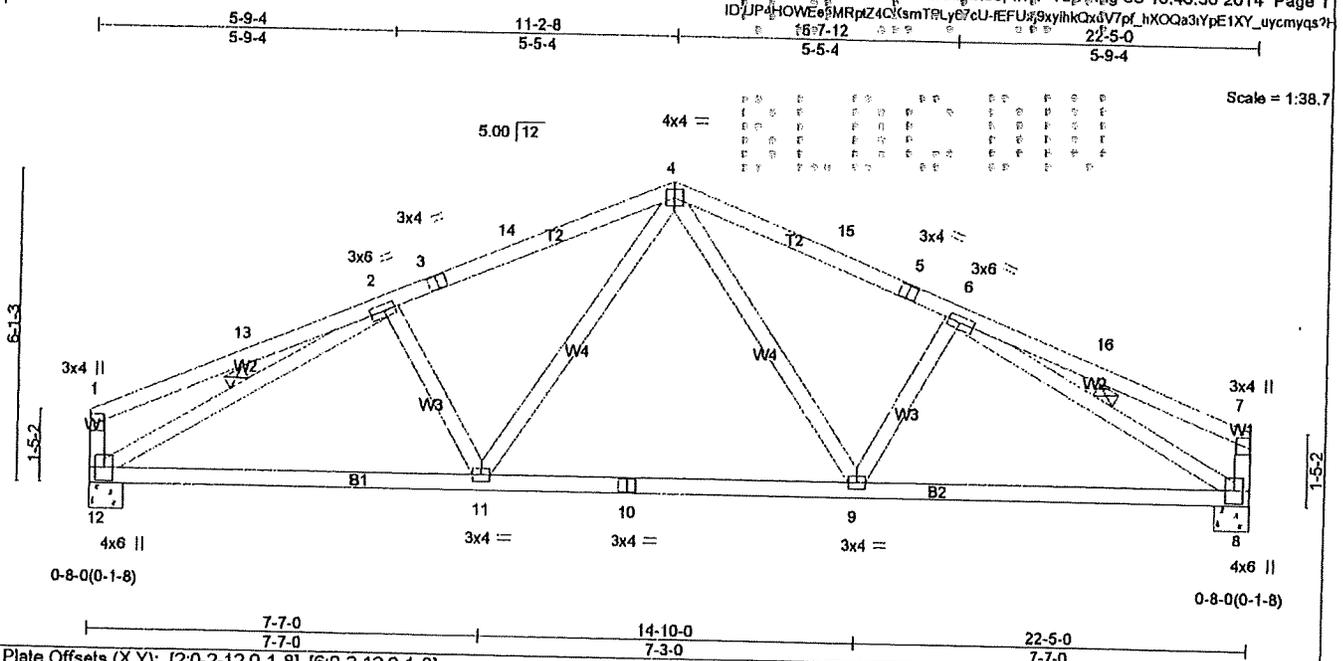


Plate Offsets (X,Y): [2:0-2-12,0-1-8], [6:0-2-12,0-1-8]					
<b>LOADING</b> (psf)	<b>SPACING</b> 2-0-0	<b>CSI</b>	<b>DEFL</b> in (loc) l/defl L/d	<b>PLATES</b>	<b>GRIP</b>
TCLL 30.0	Plates Increase 1.33	TC 0.52	Vert(LL) 0.09 9-11 >999 240	MT20	244/190
TCDL 15.0	Lumber Increase 1.33	BC 0.57	Vert(TL) -0.22 8-9 >999 180		
BCLL 0.0	Rep Stress Incr YES	WB 0.42	Horz(TL) 0.06 8 n/a n/a		
BCDL 10.0	Code FRC2010/TPI2007	(Matrix-M)			
				Weight: 122 lb	FT = 0%

**LUMBER**  
TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.2  
WEBS 2x4 SP No.3 \*Except\*  
W1: 2x4 SP No.2

**BRACING**  
TOP CHORD  
Structural wood sheathing directly applied or 4-2-6 oc purlins, except end verticals.  
BOT CHORD  
Structural wood sheathing directly applied or 7-8-3 oc bracing.  
WEBS  
1 Row at midpt 2-12, 6-8

**TOP CHORD**  
1-13=-278/216, 2-3=-1623/856,  
3-14=-1536/870, 4-14=-1517/890,  
4-15=-1517/890, 5-15=-1536/870,  
5-6=-1623/856, 7-16=-278/216,  
1-12=-298/258, 7-8=-298/258

**BOT CHORD**  
11-12=-575/1515, 10-11=-336/1174,  
9-10=-336/1174, 8-9=-575/1515

**WEBS**  
4-9=-190/491, 6-9=-304/290,  
4-11=-190/491, 2-11=-304/290,  
2-12=-1605/651, 6-8=-1605/651

6) "Semi-rigid pitchbreaks with fixed heels"  
Member end fixity model was used in the analysis and design of this truss.

**LOAD CASE(S)**  
Standard

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

**REACTIONS** (lb/size)

12 =	1217/0-8-0
8 =	1217/0-8-0
Max Horz	
12 =	-276(LC 8)
Max Uplift	
12 =	-378(LC 10)
8 =	-378(LC 10)
Max Grav	
12 =	1217(LC 1)
8 =	1217(LC 1)

**FORCES** (lb)  
Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
**TOP CHORD**  
1-13=-278/216, 2-3=-1623/856,

**NOTES**

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=175mph (3-second gust) Vasd=136mph; HVHZ; TCDL=9.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Encl., GCpl=0.18; MWFRS (directional) and C-C Exterior(2) 0-1-12 to 3-1-12, Interior(1) 3-1-12 to 8-2-8, Exterior(2) 8-2-8 to 11-2-8, Interior(1) 14-2-8 to 19-3-4 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Plates checked for a plus or minus 0 degree rotation about its center.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=378, 8=378.



Job 29875-1	Truss V6	Truss Type VALLEY TR	Qty 1	Ply 1	CHELLE CONST./HERSKOWITZ ADDITION A0039115
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7.510 s Jan 20 2014 MITek Industries, Inc. Tue Aug 05 16:46:37 2014 Page 1  
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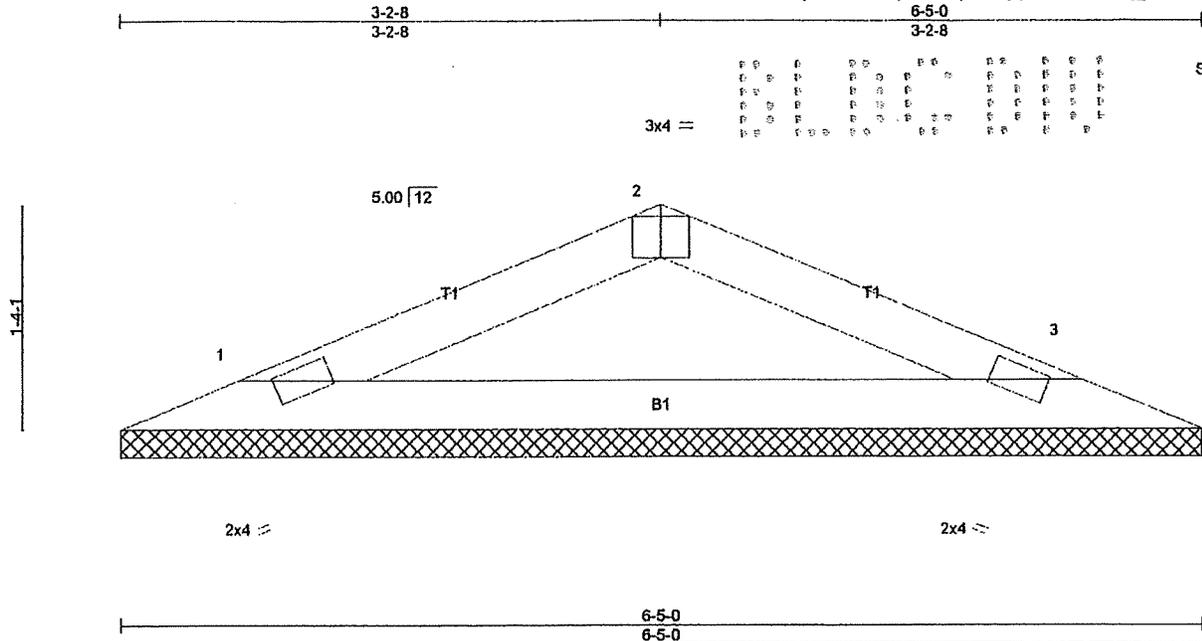


Plate Offsets (X,Y): [2:0-2:0,Edge]									
<b>LOADING (psf)</b>	<b>SPACING</b>	2-0-0	<b>CSI</b>	<b>DEFL</b>	in (loc)	l/defl	L/d	<b>PLATES</b>	<b>GRIP</b>
TCLL 30.0	Plates Increase	1.33	TC 0.22	Vert(LL)	n/a	-	n/a	999	MT20
TCDL 15.0	Lumber Increase	1.33	BC 0.31	Vert(TL)	n/a	-	n/a	999	
BCLL 0.0	Rep Stress Incr	YES	WB 0.00	Horz(TL)	0.00	3	n/a	n/a	
BCDL 10.0	Code FRC2010/TPI2007		(Matrix)						Weight: 18 lb FT = 0%

**LUMBER**  
 TOP CHORD 2x4 SP No.2  
 BOT CHORD 2x4 SP No.2

**BRACING**  
 TOP CHORD  
 Structural wood sheathing directly applied or 6-0-0 oc purlins.  
 BOT CHORD  
 Structural wood sheathing directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

**REACTIONS (lb/size)**

1 =	273/6-5-0
3 =	273/6-5-0
Max Horz	
1 =	-42(LC 8)
Max Uplift	
1 =	-85(LC 10)
3 =	-85(LC 10)
Max Grav	
1 =	273(LC 1)
3 =	273(LC 1)

**FORCES (lb)**  
 Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
 TOP CHORD  
 1-2=-324/345, 2-3=-324/345  
 BOT CHORD  
 1-3=-249/268

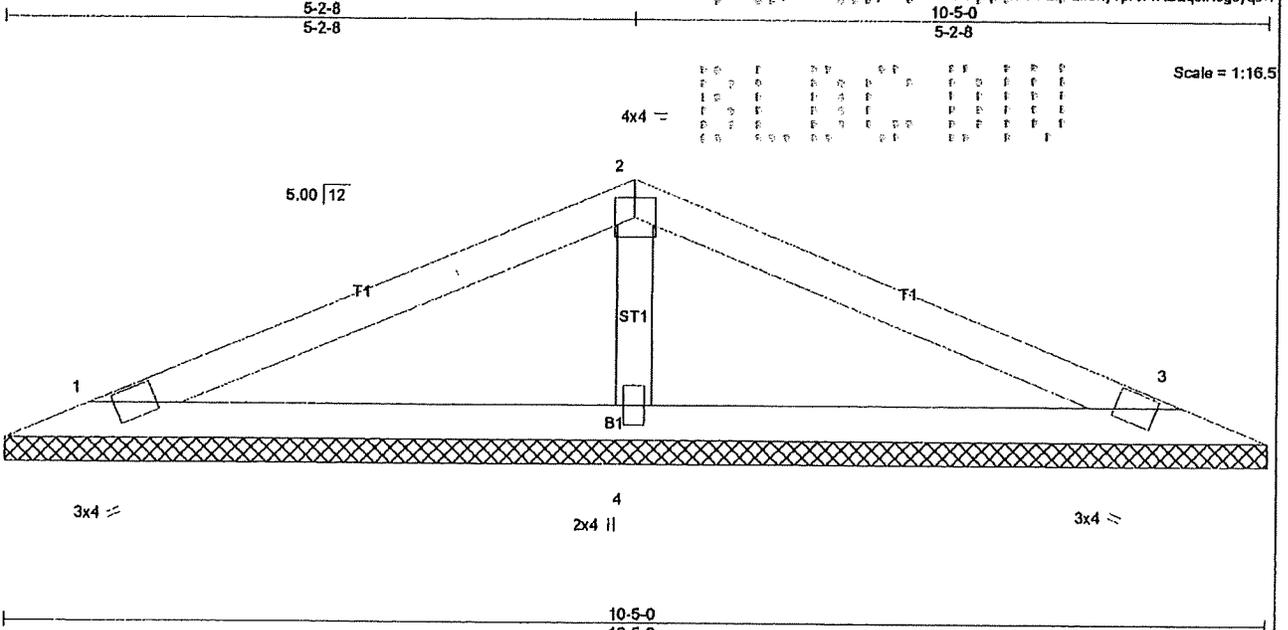
**NOTES**

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=175mph (3-second gust) Vasd=136mph; HVHZ; TCDL=9.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Encl., GCpl=0.18; MWFRS (directional) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Plates checked for a plus or minus 0 degree rotation about its center.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 7) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

**LOAD CASE(S)**  
 Standard

Job 29875-1	Truss V10	Truss Type VALLEY TR	Qty 1	Ply 1	CHELLE CONST./HERSKOWITZ ADDITION A0039112
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Job Reference (optional)  
7.510 Jan 20:2014 MITek Industries, Inc. Tue Aug 05 16:46:38 2014 Page 1  
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<b>LOADING (psf)</b>	<b>SPACING</b>	<b>CSI</b>	<b>DEFL</b>	<b>PLATES</b>	<b>GRIP</b>
TCLL 30.0	2-0-0 Plates Increase 1.33	TC 0.32	in (loc) l/defl L/d	MT20	244/190
TCDL 15.0	Lumber Increase 1.33	BC 0.25	Vert(LL) n/a - n/a 999		
BCLL 0.0	Rep Stress Incr YES	WB 0.10	Vert(TL) n/a - n/a 999		
BCDL 10.0	Code FRC2010/TPI2007	(Matrix)	Horz(TL) 0.00 3 n/a n/a		
				Weight: 32 lb	FT = 0%

**LUMBER**  
TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.2  
OTHERS 2x4 SP No.3

**BRACING**  
TOP CHORD  
Structural wood sheathing directly applied or 6-0-0 oc purlins.  
BOT CHORD  
Structural wood sheathing directly applied or 10-0-0 oc bracing.

MITek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

**REACTIONS (lb/size)**

1 =	216/10-5-0
3 =	216/10-5-0
4 =	554/10-5-0
Max Horz	
1 =	-76(LC 8)
Max Uplift	
1 =	-78(LC 10)
3 =	-78(LC 10)
4 =	-150(LC 10)
Max Grav	
1 =	223(LC 19)
3 =	223(LC 20)
4 =	554(LC 1)

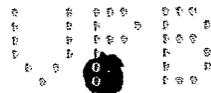
**NOTES**

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=175mph (3-second gust) Vasd=136mph; HVHZ; TCCL=9.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Encl., GCpl=0.18; MWFRS (directional) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Plates checked for a plus or minus 0 degree rotation about its center.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3 except (jt=lb) 4=150.
- 7) "Semi-rigid pitchbreaks with fixed heels"

Member end fixity model was used in the analysis and design of this truss.

**LOAD CASE(S)**  
Standard

**FORCES (lb)**  
Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
**WEBS**  
2-4=382/338



Job 29875-1	Truss V14	Truss Type VALLEY TR	Qty 1	Ply 1	CHELLE CONST./HERSKOWITZ ADDITION A0039113
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Job Reference (optional)  
7.510 6 Jan 20 2014 MiTek Industries, Inc. Tue Aug 05 16:46:38 2014 Page 1  
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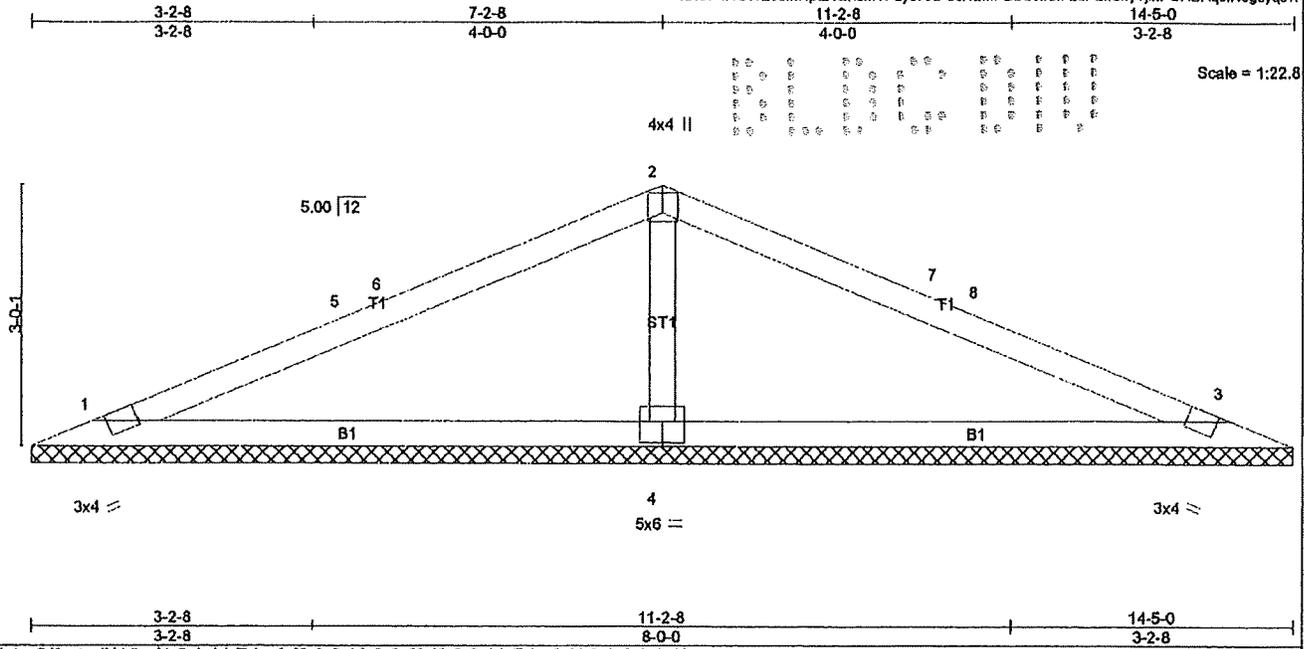


Plate Offsets (X,Y): [1:0-1-11,Edge], [2:0-2-12,0-2-0], [3:0-2-11,Edge], [4:0-3-0-0-3-0]

<b>LOADING (psf)</b>	<b>SPACING</b> 2-0-0	<b>CSI</b>	<b>DEFL</b> in (loc) l/defl L/d	<b>PLATES</b>	<b>GRIP</b>
TCLL 30.0	Plates Increase 1.33	TC 0.72	Vert(LL) n/a - n/a 999	MT20	244/190
TCDL 15.0	Lumber Increase 1.33	BC 0.49	Vert(TL) n/a - n/a 999		
BCLL 0.0	Rep Stress Inor YES	WB 0.13	Horz(TL) 0.00 3 n/a n/a		
BCDL 10.0	Code FRC2010/TPI2007	(Matrix)		Weight: 46 lb	FT = 0%

**LUMBER**

TOP CHORD 2x4 SP No.2  
BOT CHORD 2x4 SP No.2  
OTHERS 2x4 SP No.3

**BRACING**

TOP CHORD  
Structural wood sheathing directly applied or 6-0-0 oc purlins.  
BOT CHORD  
Structural wood sheathing directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

**REACTIONS (lb/size)**

1 =	312/14-5-0
3 =	312/14-5-0
4 =	802/14-5-0
Max Horz	
1 =	-110(LC 8)
Max Uplift	
1 =	-112(LC 10)
3 =	-112(LC 10)
4 =	-218(LC 10)
Max Grav	
1 =	322(LC 19)
3 =	322(LC 20)
4 =	802(LC 1)

**FORCES (lb)**

Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS  
2-4=553/423

**WEBS**

2-4=553/423

**NOTES**

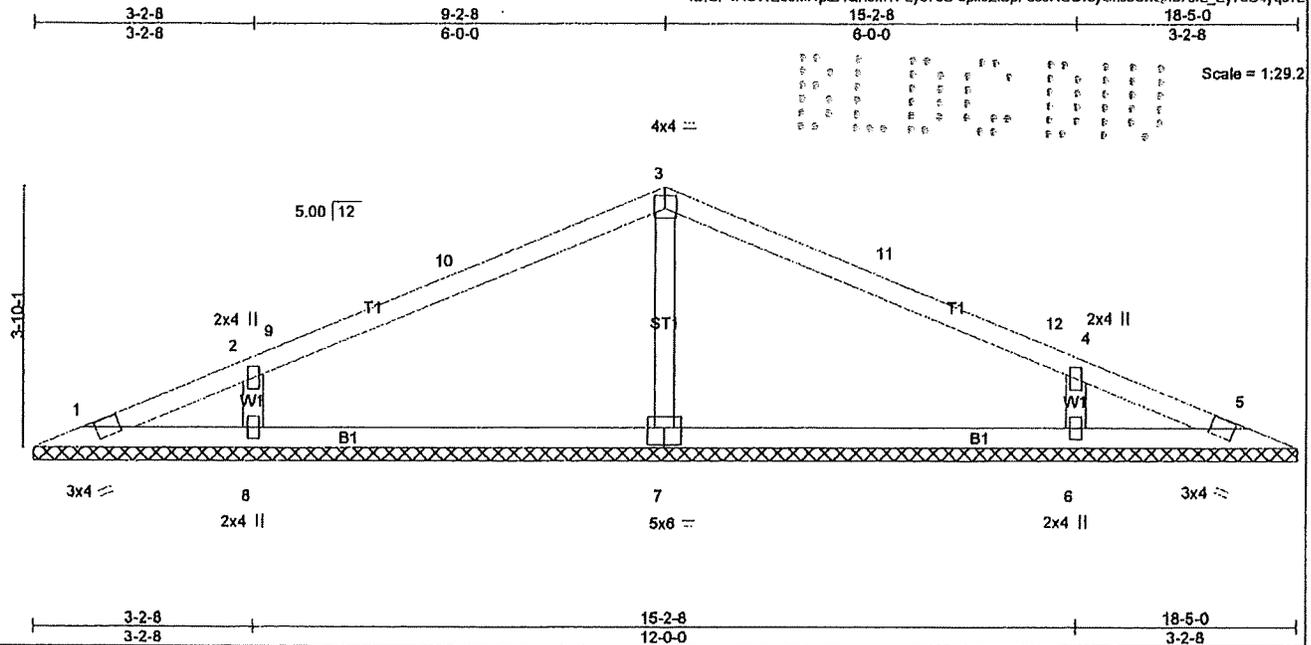
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=175mph (3-second gust) Vasd=136mph; HVHZ; TCDL=9.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) 0-8-12 to 3-8-12, Interior(1) 3-8-12 to 4-2-8, Exterior(2) 4-2-8 to 7-2-8, Interior(1) 10-2-8 to 10-8-4 zone; cantilever left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Plates checked for a plus or minus 0 degree rotation about its center.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=112, 3=112, 4=218.
- 7) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

**LOAD CASE(S)**

Standard

Job 29875-1	Truss V18	Truss Type VALLEY TR	Qty 1	Ply	CHELLE CONST./HERSKOWITZ ADDITION A0039114
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LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 30.0	2-0-0	TC 0.56	in (loc) l/defl L/d	MT20	244/190
TCDL 15.0	Plates Increase 1.33	BC 0.26	Vert(LL) n/a - n/a 999		
BCLL 0.0	Lumber Increase 1.33	WB 0.13	Vert(TL) n/a - n/a 999		
BCDL 10.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.00 5 n/a n/a	Weight: 62 lb	FT = 0%
	Code FRC2010/TPI2007				

**LUMBER**  
 TOP CHORD 2x4 SP No.2  
 BOT CHORD 2x4 SP No.2  
 WEBS 2x4 SP No.3  
 OTHERS 2x4 SP No.3

**BRACING**  
 TOP CHORD  
 Structural wood sheathing directly applied or 6-0-0 oc purlins.  
 BOT CHORD  
 Structural wood sheathing directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

**REACTIONS** All bearings 18-5-0.  
 (lb) - Max Horz  
 1=-144(LC 8)  
 Max Uplift  
 All uplift 100 lb or less at joint(s) 1, 5, 7 except  
 6=-257(LC 10), 8=-257(LC 10)  
 Max Grav  
 All reactions 250 lb or less at joint(s) 1, 5 except 7=602(LC 1), 6=615(LC 20), 8=615(LC 19)

**FORCES (lb)**  
 Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.  
**WEBS**  
 3-7=-472/278, 4-6=-533/439,  
 2-8=-533/439

**NOTES**

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=175mph (3-second gust) Vasd=136mph; HVHZ; TCDL=9.0psf; BCDL=6.0psf; h=12ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (directional) and C-C Exterior(2) 0-8-12 to 3-8-12, Interior(1) 3-8-12 to 6-2-8, Exterior(2) 6-2-8 to 9-2-8, Interior(1) 12-2-8 to 14-8-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Plates checked for a plus or minus 0 degree rotation about its center.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 7 except (t=lb) 6=257, 8=257.
- 7) "Semi-rigid pitchbreaks with fixed heels" Member end fixity model was used in the analysis and design of this truss.

**LOAD CASE(S)**  
 Standard

U.S.A.

 This safety alert symbol is used to attract your attention! **PERSONAL SAFETY IS INVOLVED!** When you see this symbol - **BECOME ALERT - HEED ITS MESSAGE.**

 **DANGER:** A **DANGER** designates a condition where failure to follow instructions or heed warning will most likely result in serious personal injury or death or damage to structures.

 **CAUTION:** A **CAUTION** identifies safe operating practices or indicates unsafe conditions that could result in personal injury or damage to structures.

 **WARNING:** A **WARNING** describes a condition where failure to follow instructions could result in severe personal injury or damage to structures.

# HIB-91 Summary Sheet

## COMMENTARY and RECOMMENDATIONS for HANDLING, INSTALLING & BRACING METAL PLATE CONNECTED WOOD TRUSSES®

  
**TRUSS PLATE INSTITUTE**  
583 D'Onofrio Dr., Suite 200  
Madison, Wisconsin 53719  
(608) 833-5900

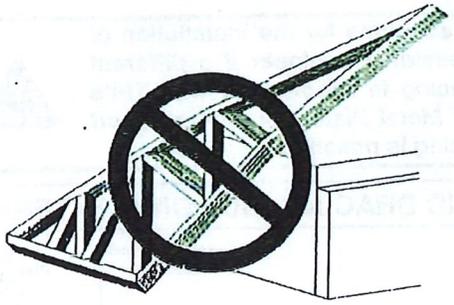
*It is the responsibility of the installer (builder, building contractor, licensed contractor, erector or erection contractor) to properly receive, unload, store, handle, install and brace metal plate connected wood trusses to protect life and property.* The installer must exercise the same high degree of safety awareness as with any other structural material. TPI does not intend these recommendations to be interpreted as superior to the project Architect's or Engineer's design specification for handling, installing and bracing wood trusses for a particular roof or floor. These recommendations are based upon the collective experience of leading technical

personnel in the wood truss industry, but must, due to the nature of responsibility involved, be presented as a guide for the use of a qualified building designer/installer. Thus, the Truss Plate Institute, Inc. expressly disclaims any responsibility for damages arising from the use, application or reliance on the recommendation and information contained herein by building designers, installers, and other Copyright © by Truss Plate Institute, Inc. All rights reserved. This document, any part thereof must not be reproduced in any form without written permission of the publisher. Printed in the United States of America.

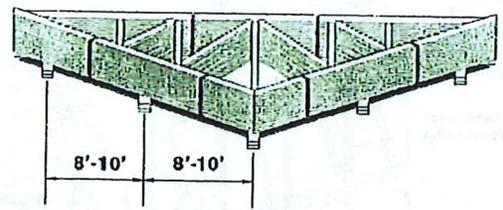
 **CAUTION:** The builder, building contractor, licensed contractor, erector or erection contractor is advised to obtain and read the entire booklet "Commentary and Recommendations for Handling, Installing & Bracing Metal Plate Connected Wood Trusses, HIB-91" from the Truss Plate Institute.

 **CAUTION:** All temporary bracing should be no less than 2x4 grade marked lumber. All connections should be made with minimum of 2-16d nails. All trusses assumed 2' on-center or less. All multi-ply trusses should be connected together in accordance with design drawings prior to installation.

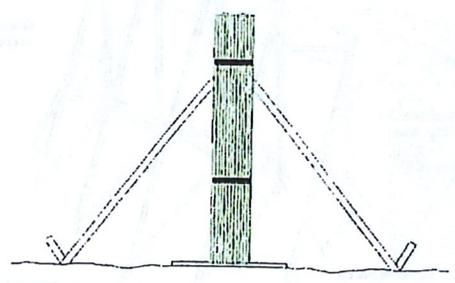
### TRUSS STORAGE



 **CAUTION:** Trusses should not be unloaded on rough terrain or uneven surfaces which could cause damage to the truss.



Trusses stored horizontally should be supported on blocking to prevent excessive lateral bending and lessen moisture gain.



Trusses stored vertically should be braced to prevent toppling or tipping.

 **WARNING:** Do not break banding until installation begins or lift bundled trusses by the bands.

 **DANGER:** Do not store bundles upright unless properly braced.

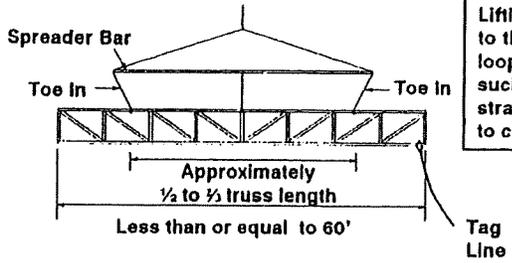
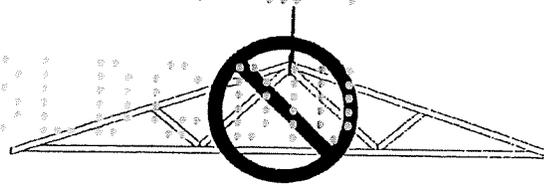
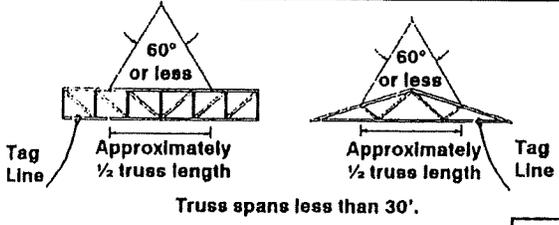
 **WARNING:** Do not use damaged trusses.

 **DANGER:** Walking on trusses which are lying flat is extremely dangerous and should be strictly prohibited.

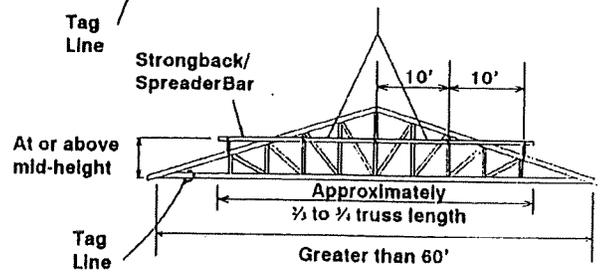
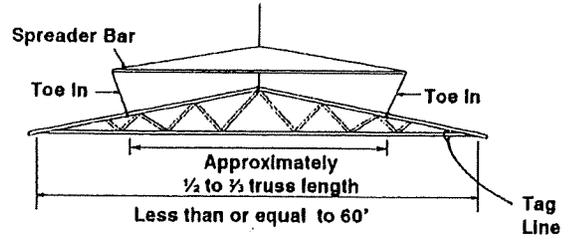
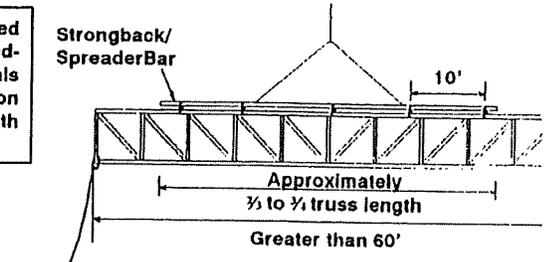
**WARNING:** Do not attach cables, chains, or hooks to the web members.

**WARNING:** Do not lift single trusses with spans greater than 30' by the peak.

**MECHANICAL INSTALLATION**



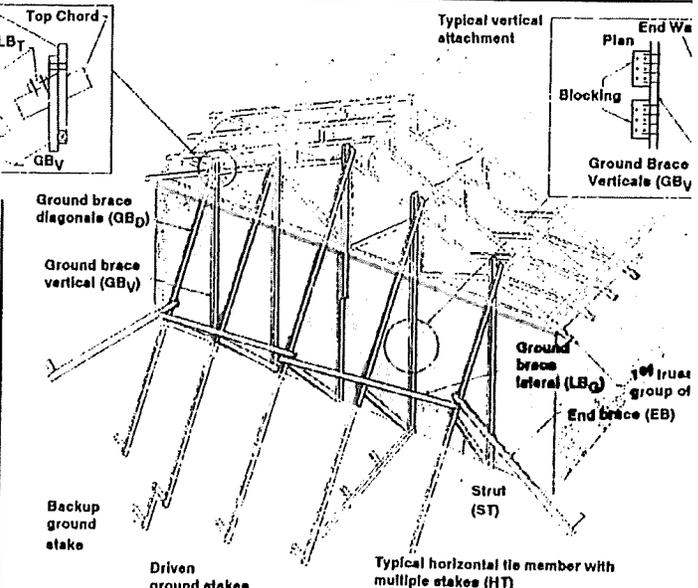
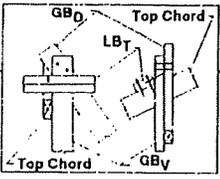
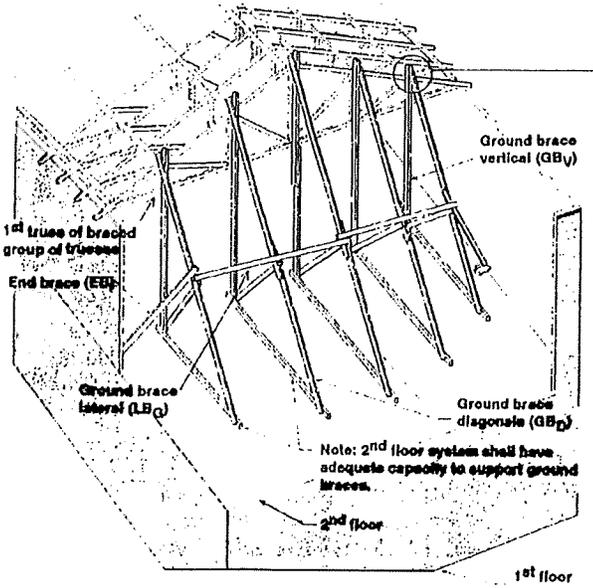
Lifting devices should be connected to the truss top chord with a closed-loop attachment utilizing materials such as slings, chains, cables, nylon strapping, etc. of sufficient strength to carry the weight of the truss.



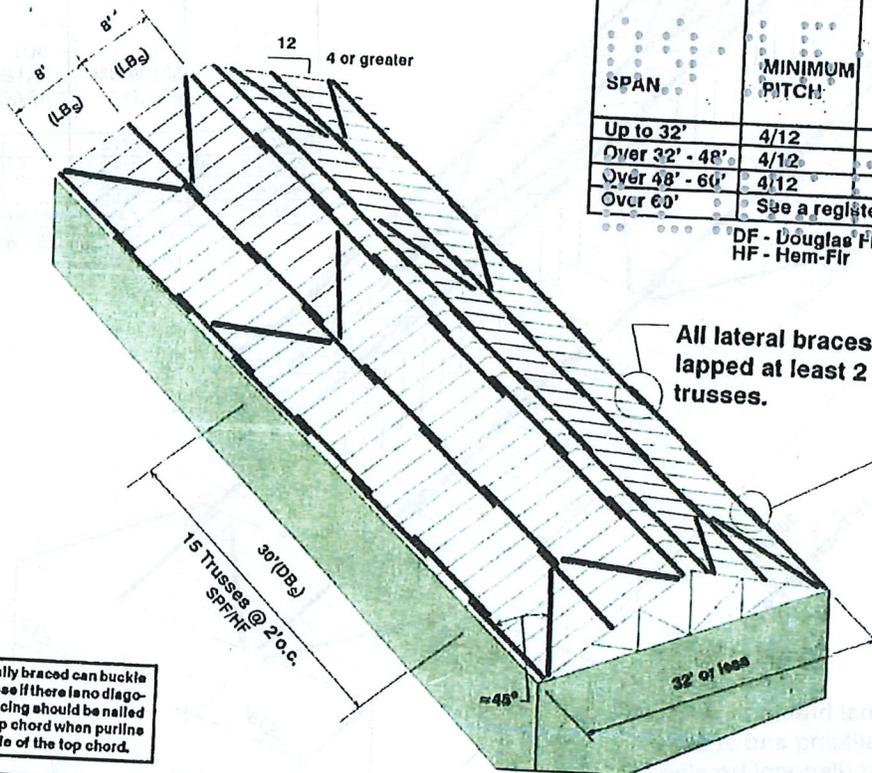
**CAUTION:** Temporary bracing shown in this summary sheet is adequate for the installation of trusses with similar configurations. Consult a registered professional engineer if a different bracing arrangement is desired. The engineer may design bracing in accordance with TPI's *Recommended Design Specification for Temporary Bracing of Metal Plate Connected Wood Trusses, DSB-89*, and in some cases determine that a wider spacing is possible.

**GROUND BRACING: BUILDING INTERIOR**

**GROUND BRACING: BUILDING EXTERIOR**

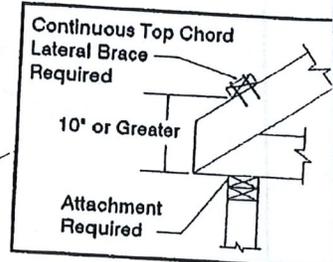


**CAUTION:** Ground bracing required for all installations.



SPAN	MINIMUM PITCH	TOP CHORD LATERAL BRACE SPACING (LBs)	TOP CHORD DIAGONAL BRACE SPACING (DBs) [# trusses]	
			SP/DF	SPF/HF
Up to 32'	4/12	8'	20	15
Over 32' - 48'	4/12	8'	10	7
Over 48' - 60'	4/12	8'	6	4
Over 60'	See a registered professional engineer			

DF - Douglas Fir-Larch  
 HF - Hem-Fir  
 SP - Southern Pine  
 SPF - Spruce-Pine-Fir

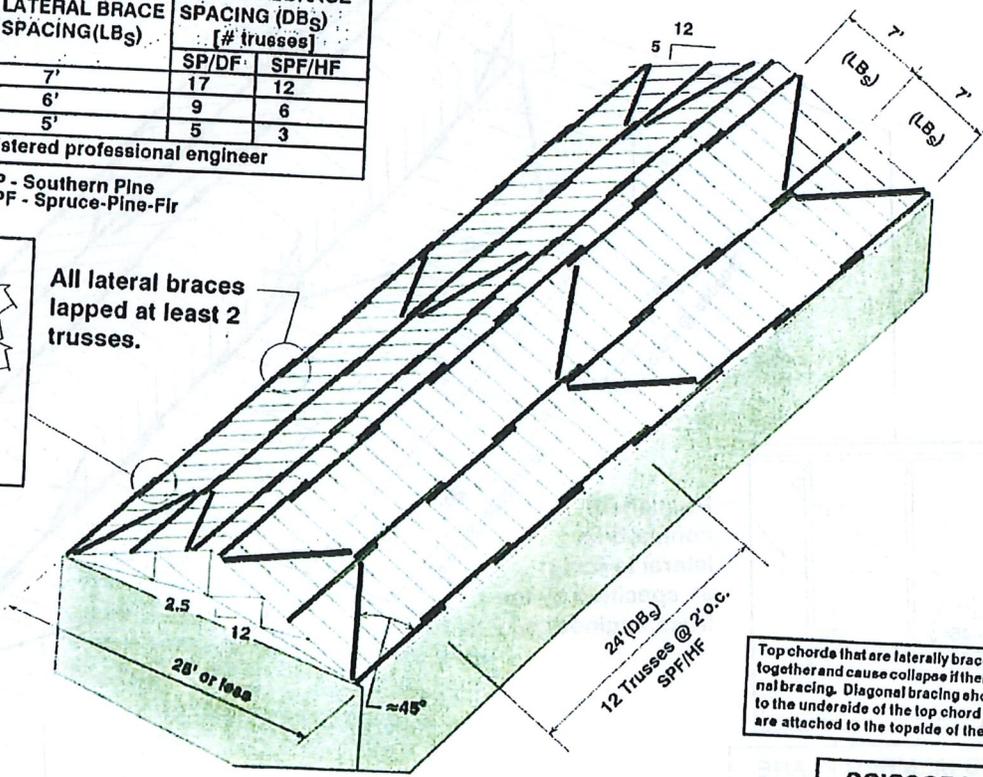
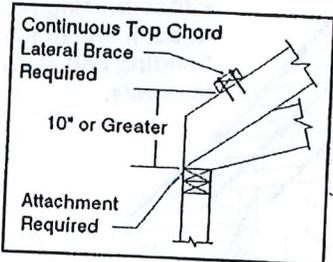


Top chords that are laterally braced can buckle together and cause collapse if there is no diagonal bracing. Diagonal bracing should be nailed to the underside of the top chord when purlins are attached to the topside of the top chord.

**WARNING:** Failure to follow these recommendations could result in severe personal injury or damage to trusses or buildings.

SPAN	MINIMUM PITCH DIFFERENCE	TOP CHORD LATERAL BRACE SPACING (LBs)	TOP CHORD DIAGONAL BRACE SPACING (DBs) [# trusses]	
			SP/DF	SPF/HF
Up to 28'	2.5	7'	17	12
Over 28' - 42'	3.0	6'	9	6
Over 42' - 60'	3.0	5'	5	3
Over 60'	See a registered professional engineer			

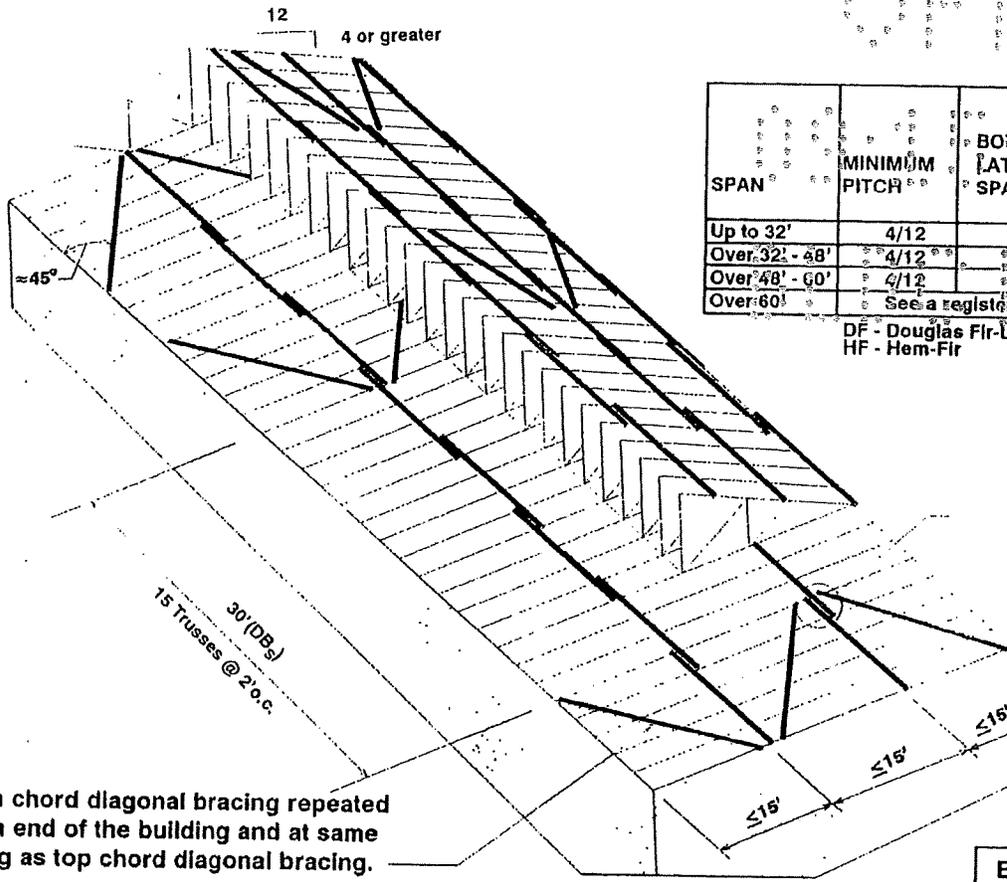
DF - Douglas Fir-Larch  
 HF - Hem-Fir  
 SP - Southern Pine  
 SPF - Spruce-Pine-Fir



Top chords that are laterally braced can buckle together and cause collapse if there is no diagonal bracing. Diagonal bracing should be nailed to the underside of the top chord when purlins are attached to the topside of the top chord.

Frame 3

**SCISSORS TRUSS**



SPAN	MINIMUM PITCH	BOTTOM CHORD LATERAL BRACE SPACING (LBs)	BOTTOM CHORD DIAGONAL BRACING (DBs) [# trusses]	
			SP/DF	SPF/HF
Up to 32'	4/12	15'	20	15
Over 32' - 48'	4/12	15'	10	7
Over 48' - 60'	4/12	15'	6	4
Over 60'	See a registered professional engineer			

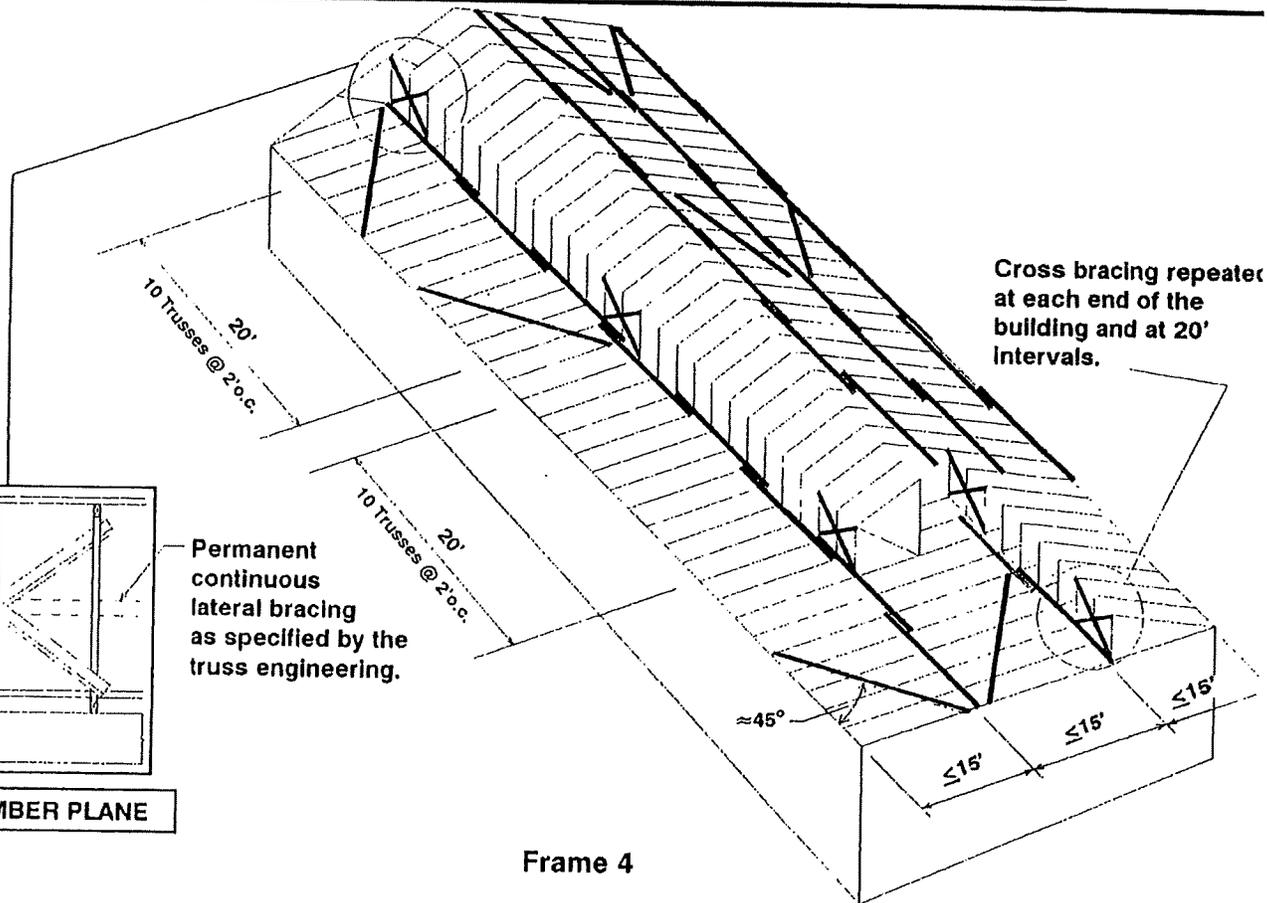
DF - Douglas Fir-Larch  
 HF - Hem-Fir  
 SP - Southern Pine  
 SPF - Spruce-Pine-Fir

Bottom chord diagonal bracing repeated at each end of the building and at same spacing as top chord diagonal bracing.

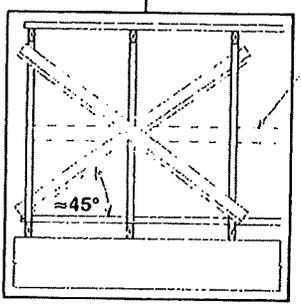
All lateral braces lapped at least 2 trusses.

**BOTTOM CHORD PLANE**

**WARNING:** Failure to follow these recommendations could result in severe personal injury or damage to trusses or buildings.



Cross bracing repeated at each end of the building and at 20' intervals.



**WEB MEMBER PLANE**

Permanent continuous lateral bracing as specified by the truss engineering.

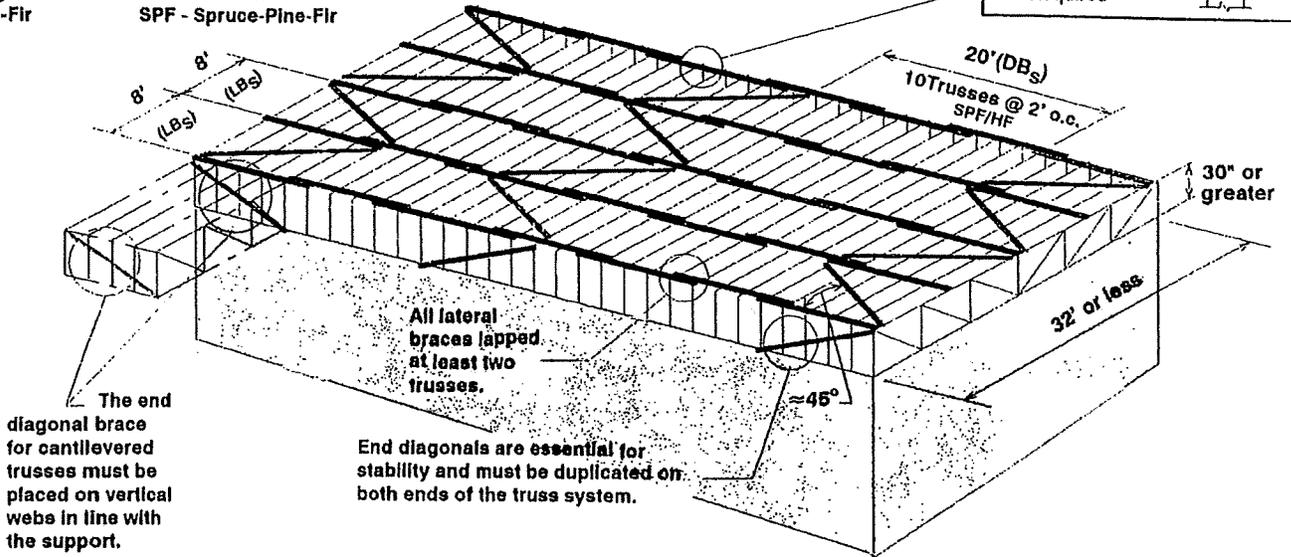
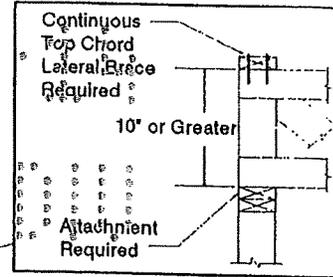
**Frame 4**

SPAN	MINIMUM DEPTH	TOP CHORD LATERAL BRACE SPACING (LB <sub>s</sub> )	TOP CHORD DIAGONAL BRACE SPACING (DB <sub>s</sub> )	
			[# trusses]	
			SP/DF	SPF/HF
Up to 32'	30"	8'	16	10
Over 32' - 48'	42"	6'	6	4
Over 48' - 60'	48"	5'	4	2
Over 60'	See a registered professional engineer			

DF - Douglas Fir-Larch SP - Southern Pine  
 HF - Hem-Fir SPF - Spruce-Pine-Fir

### 2x4/2x6 PARALLEL CHORD TRUSS

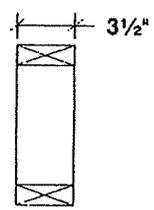
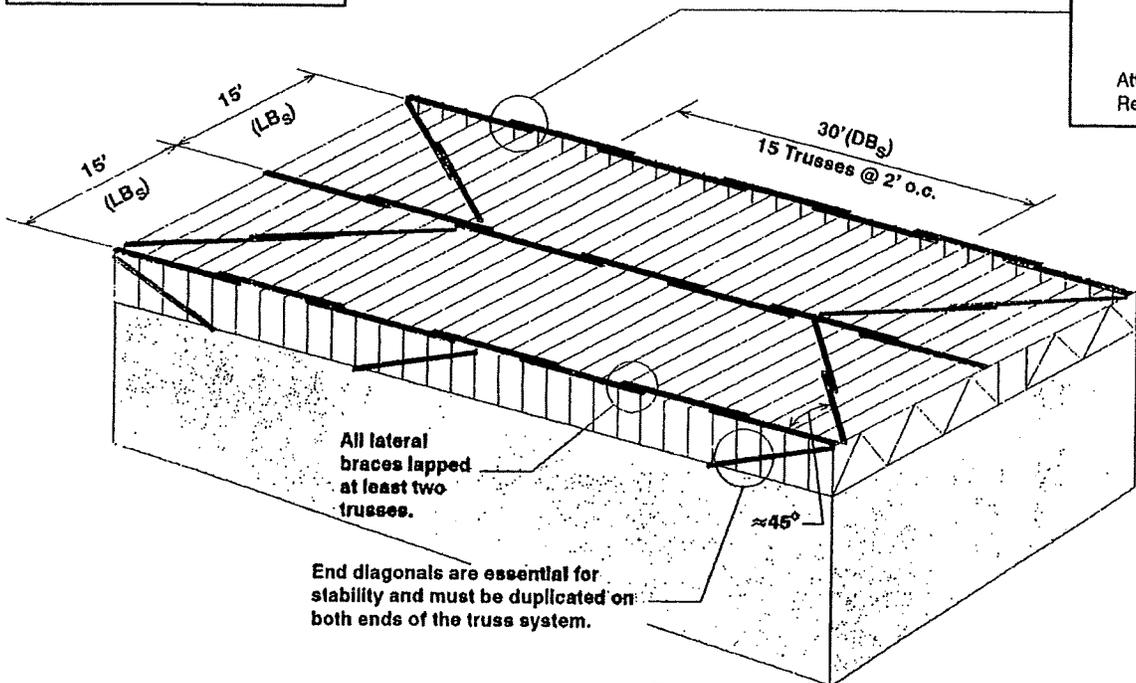
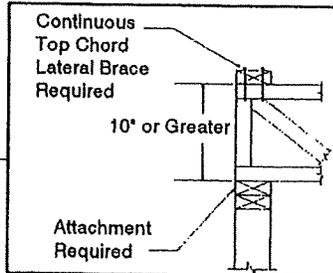
Top chords that are laterally braced can buckle together and cause collapse if there is no diagonal bracing. Diagonal bracing should be nailed to the underside of the top chord when purlins are attached to the topside of the top chord.



**WARNING:** Failure to follow these recommendations could result in severe personal injury or damage to trusses or buildings.

### 4x2 PARALLEL CHORD TRUSS: TOP CHORD

Top chords that are laterally braced can buckle together and cause collapse if there is no diagonal bracing. Diagonal bracing should be nailed to the underside of the top chord when purlins are attached to the topside of the top chord.

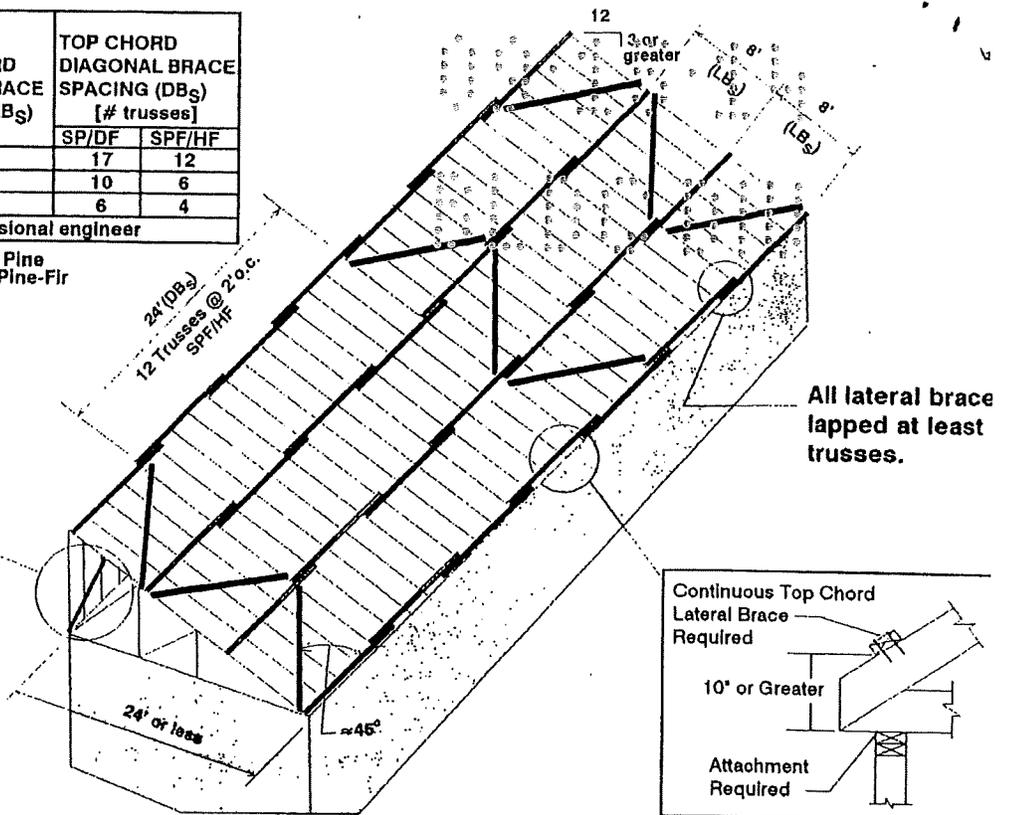


Trusses must have lumber oriented in the horizontal direction to use this brace spacing.

SPAN	MINIMUM PITCH	TOP CHORD LATERAL BRACE SPACING (LB <sub>S</sub> )	TOP CHORD DIAGONAL BRACE SPACING (DB <sub>S</sub> ) [# trusses]	
			SP/DF	SPF/HF
Up to 24'	3/12	8'	17	12
Over 24' - 42'	3/12	7'	10	6
Over 42' - 54'	3/12	6'	6	4
Over 54'	See a registered professional engineer			

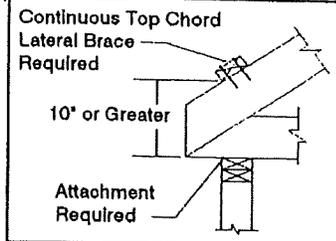
DF - Douglas Fir-Larch  
HF - Hem-Fir

SP - Southern Pine  
SPF - Spruce-Pine-Fir



Diagonal brace also required on end verticals.

Top chords that are laterally braced can buckle together and cause collapse if there is no diagonal bracing. Diagonal bracing should be nailed to the underside of the top chord when pullins are attached to the top side of the top chord.



**MONO TRUSS**

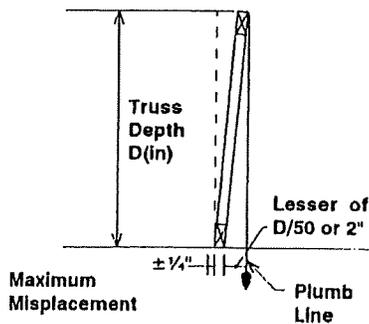
**WARNING:** Failure to follow these recommendations could result in severe personal injury or damage to trusses or buildings.

**INSTALLATION TOLERANCES**

**BOW**

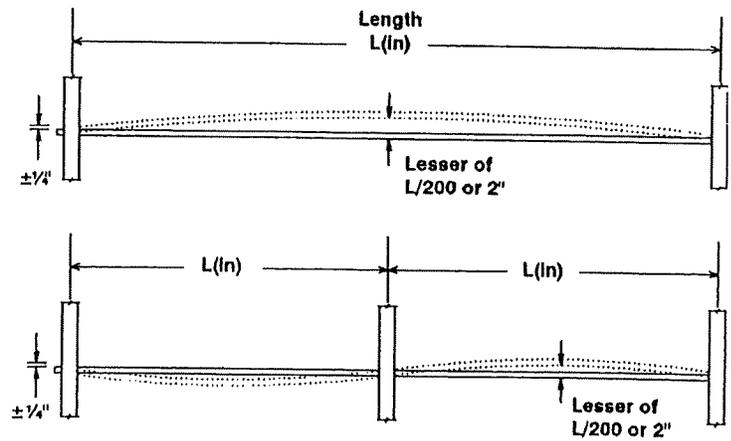
**PLUMB**

D (in)	D/50	D (ft)
12"	1/4"	1'
24"	1/2"	2'
36"	3/4"	3'
48"	1"	4'
60"	1-1/4"	5'
72"	1-1/2"	6'
84"	1-3/4"	7'
96"	2"	8'
108"	2"	9'



**OUT-OF-PLUMB INSTALLATION TOLERANCES.**

**WARNING:** Do not cut trusses.



L (in)	L/200	L (ft)
50"	1/4"	4.2'
100"	1/2"	8.3'
150"	3/4"	12.5'

L (in)	L/200	L (ft)
200"	1"	16.7'
250"	1-1/4"	20.8'
300"	1-1/2"	25.0'

**OUT-OF-PLANE INSTALLATION TOLERANCES.**

**DANGER:** Under no circumstances should construction loads of any description be placed on unbraced trusses.



**HANGER SCHEDULE**

MARK	MFG BY	TYPE	NAILS TO TRUSS	NAILS TO GIRDER	Girder/Heel	MAX. UPLIFT (POUNDS)	MAX. REACTIONS	MAX. REACTIONS	ENGR. SET PAGE #	CODE
							(POUNDS) 25% & 33%	(POUNDS) @ 0%		
1	USP	MSH213 45 L & R	4 - 10d	22 - 10d	2x4/2x4	730	1770	1770	6C	FL - 822
			4 - 10d	14 - 10d		730	1126	1126		
			4 - 10d	12 - 10d		730	965	965		
			4 - 10d	8 - 10d		730	643	643		
3	USP	MSH422	4 - 10d	22 - 10d	FLOOR MAX. 20" DEEP	1570	3470	3075	6C	FL - 822
			4 - 10d	20 - 10d		1570	3154	2796		
			4 - 10d	14 - 10d		1570	2208	1956		
			4 - 10d	10 - 10d		1570	1577	1397		
2	USP	HUS26	6 - 16d	14 - 16d	2X6/2X6	1925	3295	2635	6B	FL - 818
			6 - 16d	10 - 16d	2X6/2X4	1925	2354	1882		
			4 - 16d	6 - 16d	2X4/2X4	1283	1412	1129		
4	USP	HUS28	8 - 16d	22 - 16d	2X8/2X8	2570	4345	3970	6B	FL - 818
			6 - 16d	18 - 16d	2X8/2X6	1927	3555	3248		
			6 - 16d	12 - 16d	2X6/2X6	1927	2370	2165		
			4 - 16d	8 - 16d	2X6/2X4	1285	1580	1444		
11	USP	THD28-2	16 - 10d	28 - 16d	2X8/2X8	2595	4935	3950	6D	FL - 815
			12 - 10d	18 - 16d	2X8/2X6	1909	3172	2700		
			12 - 10d	12 - 16d	2X6/2X6	1273	2010	1607		
			4 - 10d	8 - 16d	2X6/2X4	849	1340	1071		
12	USP	THDH28-3	10 - 16d	36 - 16d	2X8/2X8	2665	7825	6260	6E	FL - 821
			8 - 16d	20 - 16d	2X6/2X6	2132	4347	3477		
9	USP	EHUH 28 - 3N	16-HN20A	40-HN20A	2X8/2X8	2850	5335	5335	8	
			14-HN20A	26-HN20A	2X6/2X6	2493	3467	3467		
NOA No:03-0611.05										
SPH1 by Bldr	See Eng. Detail	Special Hanger	2-5/8" Bolts	4-5/8" Bolts	MIN. 2X6 VERTICAL	4748	5675	4540	8C	N/A
SPH2 by Bldr	See Eng. Detail	Special Hanger	3-5/8" Bolts	6-5/8" Bolts	MIN. 2X6 VERTICAL	7800	7810	6248	8D	N/A
SPH3 by Bldr	See Eng. Detail	Special Hanger	4-5/8" Bolts	9-5/8" Bolts	MIN. 2X8 VERTICAL	10900	11700	9360	8E	N/A

**Note:**

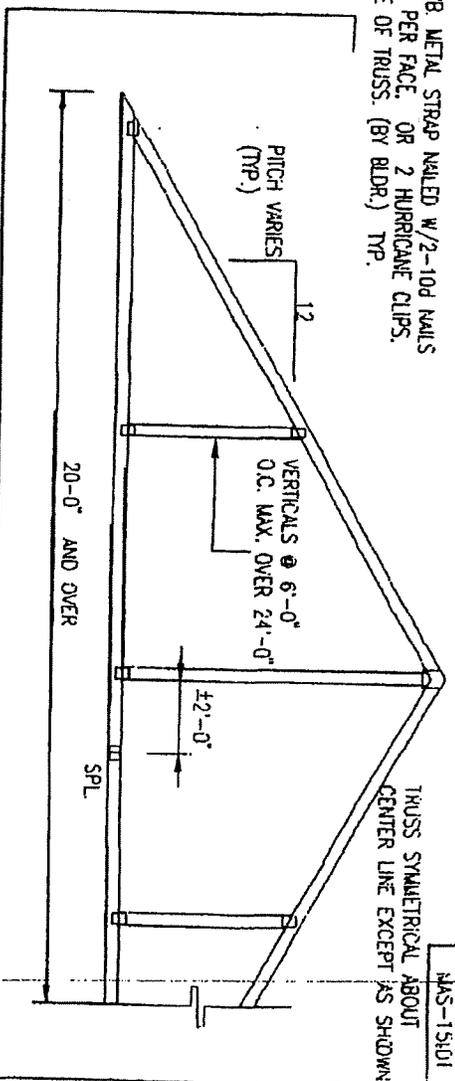
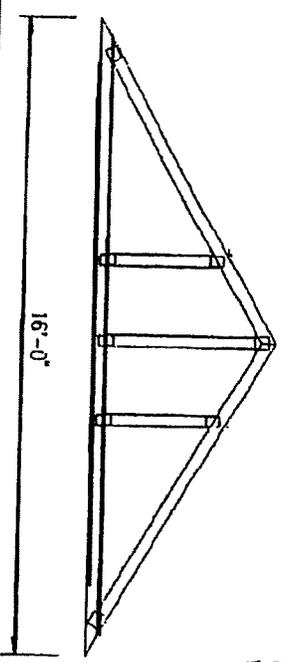
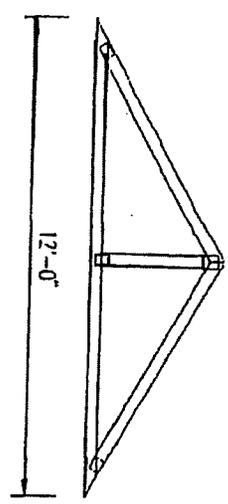
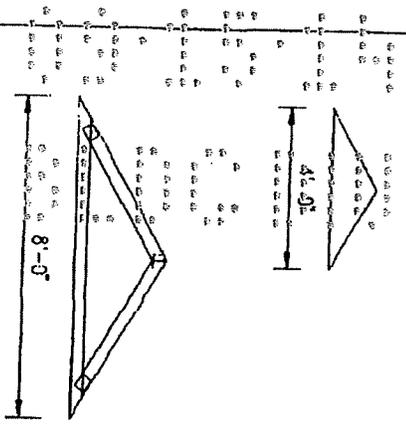
- \* These values and specifications have been taken from manufacturer literature approvals.
  - \* All hangers must be installed in accordance with the manufacturer specifications.
  - \* Cross reference hanger mark shown in truss layout with mark on this page for hanger type required.
- Example: (1) Use one .MSH213 from USP MFG.  
See page 6C for MFG Spec's included with Engr. Sheets).  
All Special Hangers to be provided by Builder.

Excel: 9: \Schedule \ Hanger Schedule - DT

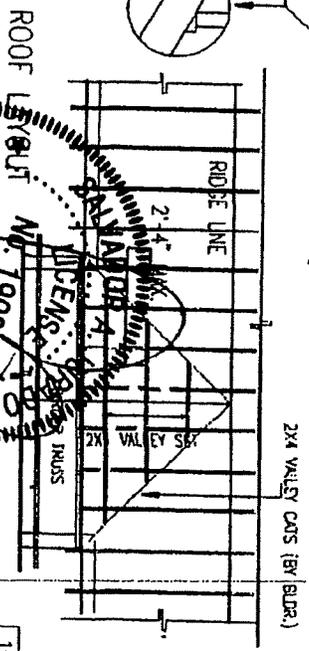
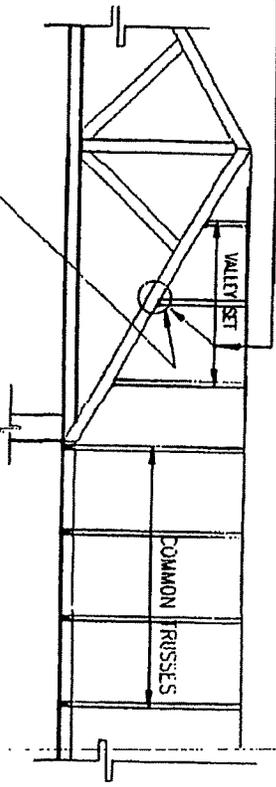
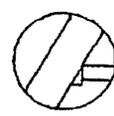
**Minimum Grade Of Lumber**  
**2X4 SYP. No. 2ND 19**

PLATES ARE METR. M20-249 20G MANUFACTURED FROM ASTM A 466 GRD. A GALVANIZED STEEL (EXCEPT AS SHOWN).  
 PLATE MUST BE INSTALLED ON EA. FACE OF JOINT SYMMETRICALLY (EXCEPT AS SHOWN) DESIGN CONFORMS W/ AISC SPECS, **FBC 2007**  
 THIS DESIGN IS FOR TRUSS FABRICATION ONLY. FOR PERMANENT BRACING (WHICH IS ALWAYS REQD.) CONSULT BLDG. ARCHITECT

USE 1" 30G GALV. METAL STRAP NAILED W/ 2-10d NAILS  
 ON EA. MEMBER, PER FACE, OR 2 HURRICANE CLIPS,  
 ONE ON EA. FACE OF TRUSS. (BY BLDG.) TYP.



6" WEDGE NAILED TO TRUSS  
 W/ 2-16d NAILS. (BY BLDG.)  
 TYP.



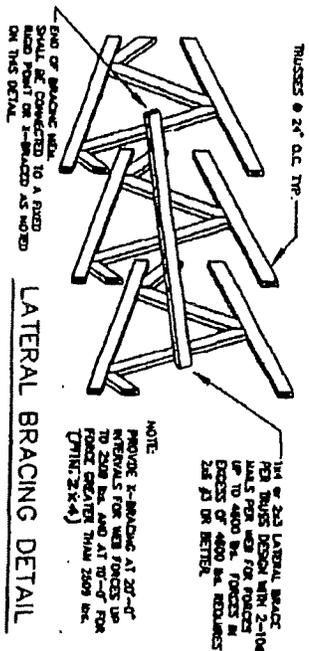
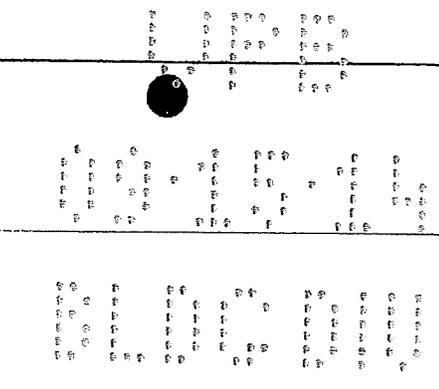
**VALLEY SET CONNECTION DETAIL**

**DADE TRUSSES**  
 Company Inc.  
 6401 N.W. 74 AVE. MIAMI, FLA. 33186  
 (305) 596-4045 (MIAMI) (305) 467-5988  
 (800) 1-500-584-7472 Fax: (305) 477-4022

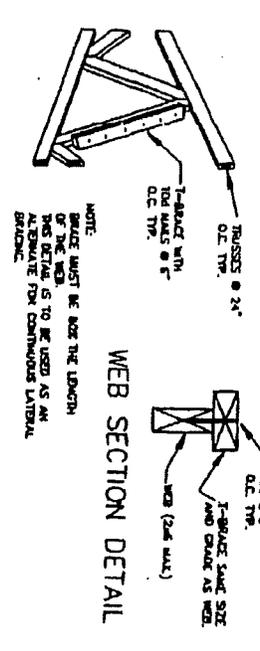
APPROVED FOR STATE OF FLORIDA  
 PROFESSIONAL ENGINEER  
 No. 19992  
 DATE: 04/07/09  
 DRAWING JOB No. 04/07/09

Minimum Grade Of Lumber  
2X4 Syp. No. 2ND 19

PLATES ARE NITEK K20-249,200 MANUFACTURED FROM ASTM A 466 GRD A GALVANIZED STEEL (EXCEPT AS SHOWN).  
PLATE MUST BE INSTALLED ON EA. FACE OF JOINT SYMMETRICALLY (EXCEPT AS SHOWN) DESIGN CONFORMS W/ AISC SPECS, FBG 2007  
THIS DESIGN IS FOR TRUSS FABRICATION ONLY. FOR PERMANENT BRACING (WHICH IS ALWAYS REQD.) CONSULT BLDG. ARCHITECT



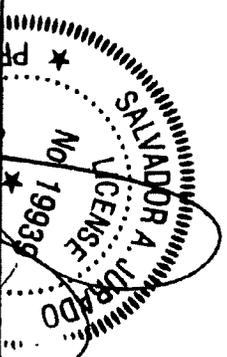
ALT. LATERAL BRACING DETAIL



LATERAL BRACING DETAIL



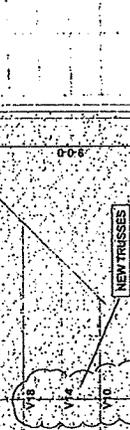
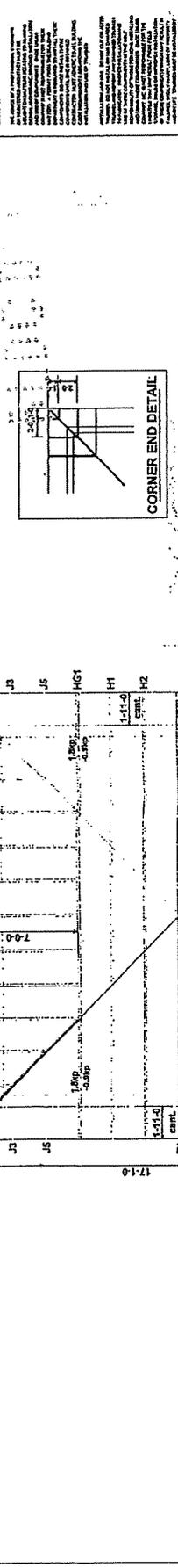
DAVID TRUSS  
Company Inc  
441 E.W. 7th Ave. Moul. FL 32114  
Phone (321) 322-4212 Fax (321) 482-3800  
Web 1-800-254-7042 Fax (321) 477-4002



APPROVED FOR THE TRUSS CO. INC.  
PROFESSIONAL ENGINEER  
SALVADOR A. JURADO

DES. BY : MN  
CHECKED BY : SJ  
DATE : 04/07/09  
DRAWING :  
JOB No. :

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Submittal For: **ROOF TRUSSES**  
 Date: **9.17.14**  
 Reviewed By: **GEORGE L. ESCOBAR**  
 DRAWN BY: **GEORGE L. ESCOBAR**  
 CHECKED BY: **GEORGE L. ESCOBAR**  
 APPROVED BY: **GEORGE L. ESCOBAR**  
 NO EXCEPTIONS TO MAIN  
 MORE COMMENTS

GEORGE L. ESCOBAR ARCHITECT, PALM BEACH, FLORIDA  
 1000 S. PALM BEACH BLVD., SUITE 100  
 PALM BEACH, FLORIDA 33480  
 TEL: 561-833-1111  
 FAX: 561-833-1112  
 WWW: GEORGELESCOBAR.COM

DATE: \_\_\_\_\_  
 CONTRACT: \_\_\_\_\_  
 PROJECT: \_\_\_\_\_  
 SHEET: \_\_\_\_\_ OF \_\_\_\_\_  
 DRAWN BY: \_\_\_\_\_  
 CHECKED BY: \_\_\_\_\_  
 APPROVED BY: \_\_\_\_\_

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MARK	TYPE	DATE	BY
1	ADD	09/17/14	GEORGE L. ESCOBAR
2	ADD	09/17/14	GEORGE L. ESCOBAR
3	ADD	09/17/14	GEORGE L. ESCOBAR
4	ADD	09/17/14	GEORGE L. ESCOBAR
5	ADD	09/17/14	GEORGE L. ESCOBAR
6	ADD	09/17/14	GEORGE L. ESCOBAR
7	ADD	09/17/14	GEORGE L. ESCOBAR
8	ADD	09/17/14	GEORGE L. ESCOBAR
9	ADD	09/17/14	GEORGE L. ESCOBAR
10	ADD	09/17/14	GEORGE L. ESCOBAR
11	ADD	09/17/14	GEORGE L. ESCOBAR
12	ADD	09/17/14	GEORGE L. ESCOBAR
13	ADD	09/17/14	GEORGE L. ESCOBAR
14	ADD	09/17/14	GEORGE L. ESCOBAR
15	ADD	09/17/14	GEORGE L. ESCOBAR
16	ADD	09/17/14	GEORGE L. ESCOBAR
17	ADD	09/17/14	GEORGE L. ESCOBAR
18	ADD	09/17/14	GEORGE L. ESCOBAR
19	ADD	09/17/14	GEORGE L. ESCOBAR
20	ADD	09/17/14	GEORGE L. ESCOBAR

SCALE: 3/8" = 1' - 0"  
 ROOF LAYOUT



CHELLE CONSTRUCTION  
 HERSKOWITZ ADDITION  
 PALMETTO BAY, FL

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*J. Wall*  
 # 2014-5499

*BTR 2014-0074*

**OFFICE COPY**

**SEVEN FIVE SEVEN  
 MARCH 19 2012**

VILLAGE OF PALMISTO BAY			
THIS COPY OF PLAN MUST BE AVAILABLE ON BUILDING SITE OR NO INSPECTION WILL BE GIVEN			
SECTION	BY	DATE	DISAPPROVED
ZONING			
P & Z			
LANDSCAPING			
FLOOR			
PUBLIC WORKS			
BUILDING	<i>Mc</i>	<i>9/17/14</i>	
HANDICAP			
STRUCTURAL	<i>Mc</i>	<i>9/17/14</i>	
ELECTRICAL			
MECHANICAL			
PLUMBING			
UTILITIES			
BUILDING OFFICIAL			

Subject to compliance with all Federal, State and County Law, rules and regulations. The Village of Palmisto Bay assumes no responsibility for accuracy of all results of these plans.

**NOTICE:** In addition to the requirements of these permits there may be additional restrictions applicable to this in the public record of this county and village.