D

33-5023-000-0710

Approved

Edward Silva, Village Manager

1/4/19

Date

VILLAGE OF PALMETTO BAY
9705 E. HIBISCUS STREET
PALMETTO BAY, FLORIDA 33157
(305) 259-1234 Fax: (305) 259-1290

REQUEST FOR PUBLIC RECORDS

Requests are filled in accordance with the provisions of Chapters 119 and 257, Florida Statutes.

DATE: 12-10-18

NAME: Joyce McGhee

COMPANY: Alexander Montessori

ADDRESS:

PHONE: 305-233-4540 FAX:

EMAIL: oc@alexandermontessori.com

REQUEST (Attach addit page, if necessary): Copies of the following documents:

Pls for permit number
BT-2017-9965.
Fay 6 Level 2

****

FOR USE BY VILLAGE ONLY

DATE FORWARDED: 12/7/18

TRACKING NO.: 2018-399

DATE REQUEST FILLED: 1/9

ASSIGNED DEPT: Building

ESTIMATED TIME (IF APPLE):

NUMBER OF COPIES: 117

HOW WAS REQUEST FILL

ESTIMATED COST:

IF NOT FILLED, REASON:

BY:

Village of Palmetto Bay Permit Application

Building and Capital Projects
9705 E. Hibiscus Street
Palmetto Bay, Florida 33157
Telephone: (305) 259-1250 Fax: (366) 927-5576
Inspection Line: (305) 259-1253
A division of the Department of Planning, Zoning, and Building Services

GENERAL INFORMATION: Please read these instructions carefully before submitting the work for review

This application must be completed and signed by both the property owner and qualifier. Both of these signatures must be notarized. Please print legibly or type in order not to delay your application. For roofing permits, in addition to this permit you must also fill out a roofing permit application. Express permits require an additional fee and will only be accepted between the hours of 8:00 A.M. and 10:00 A.M., Monday through Friday. All other permits/plans must be dropped off before 3:00 p.m. for regular processing. During the processing of your application you may be asked to submit additional information. There may be additional permits and reviews required from other governmental agencies not affiliated with Palmetto Bay.

APPLICATION:

Clerk's Initials	Plan Process Number	Master Permit Number	Subsidiary Permit Number(s)	Expiration Date
MBC	BLD-2017-9965			

Job Address: 14400 Old Cutler Rd Palmetto Bay FL 33158.
Address Unit number City State Zip Code

Folio Number: 33 5023 000 071D

Lot: _____ Block: _____

Subdivision: _____ PB: _____ PG: _____

Current Use of Property: School

Proposed Use of Property: _____

Description of Work: NEW MODULAR BATHROOM FACILITY TO BE ADA COMPLIANT

Zoning: _____ Square Feet: _____

Linear Feet: _____ Units: _____ Stories: _____

Value of Proposed Work: $20,000.00 Est. Bldg. Value: _____

Tax Assessed Value: _____

Flood Zone: _____ Base Floor Elev.: _____

Homeowner's Association: _____

Tenant Information: _____ Unit Number: _____

Check Permit Type		Check Permit Change		Check Type of Improvement			
Building	X	Change of Contractor		New Construction		Deck/Concrete flatwork	
Electrical		Permit Renewal		Exterior Alteration		Window Replacement	
Mechanical		Plan Revision		Interior Alteration		Shutters	
Plumbing		Permit Extension		Attached Addition		Garage Doors	
LPGX		Supplement		Detached Addition	X	Storage Shed	
Roofing		Re-inspection Fee		Repair		Railings	
Fence		PACE Program		Repair due to Fire		Stairs	
Sign				Demolish		Windows/Doors	
Public Works				Screen Enclosure		Roofing	
Other				Driveway		Re-Roof	
				Fence		Seal-cote	
				Pool		Other	

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Architect Information	Engineer Information
Name: BELLON ARCHITECTURE	Name:
License Number: AR0008737	License Number:
Address: 13131 SW 132 ST. #104	Address:
Telephone Number: 305-278-7776	Telephone Number:
Fax Number: 305-278-7473	Fax Number:
Other: 305-527-9983	Other:

Property Owner	Contractor
Name: James MCGhee II & Joyce McGhee	Company Name A+E DEVELOPMENT CORP.
Address: 14400 Old Cutler Rd Palmetto Bay	Qualifier AMY BELLON
Home Telephone:	License Number CGC 061671
Business Telephone: 305-233-4540	Address 13131 SW 132 ST. #104 MIAMI FL
Other Telephone:	Telephone Number 305-527-9983 - PICK UP
Fax Number:	Fax Number 305-278-7473
Does Property have Homestead Exemption no	Other 305-278-7776

Bond Company (if applicable)	Mortgage Lender
Name:	Name:
Address:	Address:
City: State: Zip	City: State: Zip

Classification Of Proposed Work

Residential ☐ Duplex ☐ Townhouse ☐ Multi Family ☐ Commercial ☐ Industrial ☐ Other ☐

Effective Code_____ Occupancy_____ Construction Type _____

Zoning: Variance Number: Remarks:

OWNER AFFIDAVIT

Application is hereby made to obtain a permit to do the work and installation as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work will be performed to meet the standards of all laws regulating construction in this jurisdiction. I understand that a separate permit must be secured for ELECTRICAL, MECHANICAL, PLUMBING, SIGNS, WELLS, POOLS, ROOFING, SHUTTERS, WINDOWS, FURNACES, BOILERS, HEATERS, TANKS, FENCE, DRIVEWAYS and AIR CONDITIONERS, ETC. In addition to the requirements of this permit, there may be additional restrictions found in the Public Records, and there may be additional permits required from other governmental entities.

I, the owner of the property, have disclosed all information related to any work that has been performed in the prior twelve months to the Building Division as part of this application. Further, I am fully aware that if the cumulative cost of work to my home or business under this and any other permit equals or exceeds fifty percent of the fair market value of the structure, the entire structure must meet the present federal flood criteria for finished floor elevation. I am also fully aware that if the cost of work to my home or business under this and any other permit equals or exceeds fifty per cent of the replacement cost of the structure, then the entire structure must conform to current code requirements of the Florida Building Code.

I, the owner of the property, understand that a permit application is subject to denial and a validated permit or permit card is subject to revocation or modification based upon applicable deeds, covenants, declarations, easements and any other legal restriction. By issuing a permit, the Village of Palmetto Bay makes no representation as to the existence or validity of any property restriction.

I, the owner of the property certify that all the foregoing information is accurate and that all work will be done in compliance with all applicable laws regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

Joyce McGhee
Signature of Owner

State of Florida, County of MIAMI-DADE

Sworn to (or affirmed) and subscribed before this 2nd day of March, 2017.
by (print name) JOYCE MCGHEE

Notary Name KELSY LASCANO

Personally known ☑ or I.D.
Type of identification produced

Notary Public State of Florida
Kelsy V Lascano
My Commission FF 227437
Expires 05/10/2019

Amy Bellon
Signature of Qualifier

State of Florida, County of MIAMI-DADE

Sworn to (or affirmed) and subscribed before this 2nd day of March, 2017.
by (print name)

Notary Name KELSY LASCANO

Personally known or I.D.
Type of identification produced

Notary Public State of Florida
Kelsy V Lascano
My Commission FF 227437
Expires 05/10/2019

IMPORTANT NOTICES

- Do not begin work without receiving you validated permit and permit card. Applying for a permit does not grant you the right to commence construction. Construction can only occur during the hours of 7:00 a.m. to 7:00 p.m., Monday thru Saturday and from the hours of 9:00 a.m. to 6:00 p.m. on Sundays and Federal holidays.
- All construction sites must be maintained in a clean and orderly condition free from construction debris. Failure to do so will result in a fine and a suspension of inspections until said property is cleaned.
- Streets and neighboring properties shall be kept free from dirt and debris.
- Swales must be protected from damage by equipment or vehicles and sidewalks cannot be blocked.
- Portable Toilets for construction jobs require a separate permit. If toilet is not available the inspection will not be performed.
- Water cannot be discharged into the right of way or storm drains without the approval of the Public Works Department.
- No equipment or materials can be stored on the right of way; they must only be stored on your property.
- Florida Department of Health approval is required for applications involving Septic Tanks. Department of Environmental Resources Management (DERM) and /or Miami-Dade Water and Sewer Department approval is required for applications involving sewers and water. The tree section of the Department of Environmental Resources Management (DERM) approval is required on all landscape plans and on all tree removal applications.

CHECKLIST (OFFICE USE ONLY)

☐ OWNER-BUILDER FORM (Attached)

☐ FIRE DEPARTMENT APPROVAL (Commercial/multi (family only)

☐ CONCURRENCY (New Construction)

☐ PROOF OF OWNERSHIP (Attached)

☐ CONDO ASSOCIATION APPROVAL

☐ HEALTH DEPARTMENT APPROVAL (Septic/Sewer)

☐ IMPACT FEE (New Construction)

☐ SCHOOL REVIEW (New Construction)

☐ DERM REVIEW (New Construction/Additions/Tree Removal)

☐ PUBLIC WORKS

☐ PERMIT CLERK REVIEW
By: _____

☐ Complete Application
☐ Current liability ins.
☐ Worker's Comp.
☐ Cont. Lic. Check

☐ OTHER (Specify and Attach)

☐ FLORIDA DEPARTMENT OF BUISNESS AND PROFESSIONAL REGULATION APPROVAL (RESTAURANTS)

☒ UPFRONT FEES AMOUNT: 1500 Paid 3/2/17

PERMIT FEES (OFFICE USE ONLY)

Scanning Fees (3.67 per sheet) Large Sheets		Art in Public Places	
Scanning Fees (1.85 per sheet) Small Sheets		Certificate of Use and Occupancy	
Village of Palmetto Bay Permit Fees	568.88	Concurrency Fee (7%)	
Miami-Dade County Fees (sq. ft. x $65/1000x0.60)	12.00	Technology Fee (6.3%)	
		Zoning Inspection Fee (157.50 per application)	48.00
Radon-Inspector State Educational Fund and DCA State Fee	8.56 8.56	Administration Fee	48.00
Code Enforcement Fine		Express Fee(25.00)	
Certificate of Completion		Public Works Fee	
Construction Sign Fee		Landscape Review Fee (175.00 per hour)	
Roll-off Waste Container Fee (105.00 per container site)		Special Review Fee 2 @ 85.89	171.78
Rework Fee		Other Trailer tie down Insp	143.00

$ 1008.78

PLAN REVIEWER APPROVAL AREA (OFFICE USE ONLY)

SECTION	REJECTED DATE NUMBER 1	REJECTED DATE NUMBER 2	REJECTED DATE NUMBER 3	APPROVAL DATE	REVIEWER APPROVAL'S NAME
ZONING				3/9/17	
ELECTRICAL					
MECHANICAL					
PLUMBING	3-10-17			8-8-17	
FIRE					
ROOFING					
PUBLIC WORKS					
PLANNING				3/9/17	
FT&I DISTRICT					
STRUCTURAL	MC 3/14/17			8/16/17	MC
BUILDING	DMG 3/24/17	DMG 5/18/17		8/17/17	DMG
BUILDING OFFICIAL					

REWORKS: A fee of **$100.00** may be charged for failure to make required corrections previously indicated. The fee shall be charged after the initial review plus one follow up review per trade. Please note that Florida Statutes 553.80 section 2(b) states "with respect to evaluation of design professionals documents, if a local government finds it necessary, in order to enforce compliance with the Florida Building Code and issue a permit, to reject design documents required by the code three or more times for failure to correct a code violation specifically noted in each rejection, including but not limited to egress, fire protection, structural stability, energy, accessibility, lighting, ventilation, electrical, mechanical, plumbing, and gas systems, or other requirements identified by rule of the Florida Building Commission adopted pursuant to Chapter 120, the local government shall impose, each time after third such review the plans are rejected for that code violation a fee of **four times the amount** of the proportion of the permit fee attributed to plans review".

ISSUING OFFICIAL

FINAL PLAN REVIEWED AND PREPARED FOR ISSUANCE BY: _____ DATE: 8/17/17

CONDITIONS OF APPROVAL

PLAN TRACKING

Plans Checked out	Date	Clerk	Plans Checked in	Date	Clerk
ERIK BELLON	03/29/17	MAR	Checked in	5-16-17	SB
ERIK BELLON	05/16/17		ERIK Bellon	5/18/17	SB
ERIK BELLON	05/16/17				
ERIK BELLON	05/17/17				
ERIK BELLON	08/10/17	MAR	ERIK BELLON		

VILLAGE OF PALMETTO BAY
COMMUNITY AND ECONOMIC DEVELOPMENT
9705 East Hibiscus Street Palmetto Bay, FL 33157
305-259-1250

NOTICE TO VILLAGE OF PALMETTO BAY BUILDING OFFICIAL OF EMPLOYMENT AS SPECIAL INSPECTOR

I (We) have been retained by ALEXANDER MONTESSORI to perform special inspector services at the PRE-FAB CONC. REST. project on the below listed structures as of 5.18.17 (date). I am a registered architect or professional engineer licensed in the State of Florida.

PROCESS / PERMIT NUMBER: BLD 2017-9965

SPECIAL INSPECTOR FOR PILING, CMDC sect. 8-22
SPECIAL INSPECTOR FOR TRUSSES >35' LONG OR 6' HIGH, CMDC sect. 8-22
SPECIAL INSPECTOR FOR REINFORCED MASONRY, FBC 2122.2.4 & CMDC sect. 8-22
SPECIAL INSPECTOR FOR STEEL CONNECTIONS, CMDC sect. 8-22
SPECIAL INSPECTOR FOR SOIL COMPACTION, CMDC sect. 8-22 ✓
SPECIAL INSPECTOR FOR PRECAST UNITS & ATTACHMENTS, CMDC sect. 8-22
(For tilt-up walls systems, inspect reinforcement as well)
SPECIAL INSPECTOR FOR CURTAIN WALL, CMDC sect. 8-22
SPECIAL INSPECTOR FOR STRUCTURAL GLAZING, CMDC sect. 8-22
SPECIAL INSPECTOR FOR LIGHTWEIGHT INSULATING CONCRETE, CMDC sect. 8-22
SPECIAL INSPECTOR FOR COMPOSITE FLOOR SYSTEM, CMDC sect. 8-22
SPECIAL INSPECTOR FOR _____

Note*: *Only the marked boxes apply.

The following individual(s) employed by this firm, or me are authorized representatives to perform inspection *

1. LEOPOLDO BELLON
2. _____
3. _____
4. _____

**Special Inspectors utilizing authorized representatives shall insure the authorized representative is qualified by education or licensure to perform the duties assigned by the Special Inspector. The qualifications shall include: licensure as a professional engineer or architect; graduation from an engineering education program in civil or structural engineering; graduation from an architectural education program; successful completion of the NCEES Fundamentals Examination; or registration as building inspector or general contractor.*

I (We) will notify the Village of Palmetto Bay Building Official of any changes regarding authorized personnel performing inspection services.

I (We) understand that a Special Inspector inspection log for each building must be displayed in a convenient location on the site for reference by the Village of Palmetto Bay Inspector. All mandatory inspections, as required by the Florida Building Code, must be performed by the Village. The Village building inspections must be called for on all mandatory inspections. *Inspections performed by the Special Inspector hired by the Owner are in addition to the mandatory inspections performed by the Department.* Further, upon completion of the work under each Building Permit, I will submit to the Building Inspector at the time of final inspection the completed inspection log form and a sealed statement indicating that to the best of my knowledge, belief and professional judgment those portions of the project outlined above meet the intent of the Florida Building Code and are in substantial accordance with the approved plans.

Signed and Sealed _____

Date 05.18.17

Engineer/Architect Name LEOPOLDO BELLON (PRINT)

Address 13131 SW 132 st. #INF
MIAMI FL 33186

Phone No. 305.790.2626

REGISTERED ARCHITECT LEOPOLDO BELLON AR 3 STATE OF FLORIDA

PRECAST CONCRETE RESTROOM FACILITY

Design Wind Speed: 175 mph V_{ult}

Prepared for:
MONTESSORI SCHOOL
17800 OLD CUTLER ROAD
PALMENTO BAY, FL 33157

By
Linden Parchment PE (54071)
8002 Glitter Court
Orlando FL, 32836

Revision 2
April 18, 2017

Revision 2 Narrative

General Note: All wind pressure data within the calculations have changed because the design wind velocity was changed from 170 mph Vult to 175 mph Vult (136 mph Working). Page numbers added.

- Page 4 – Components and Cladding wind pressures changed.
- Pages 5 through 15 were all completely modified because of the change to the design wind velocity.
- Page 16: Stability calculations were modified. The change in wind velocity did not alter the design.
- Pages 17 – 21. Wall panel designs for all the exterior walls and roof panels were modified. The change in wind velocity did not alter the structural detailing.
- Pages 22. Wind pressures on the Lexan window panels were modified. The change in wind velocity did not alter the design of the panel.

LINDEN G. PARCHMENT
LICENSE
No. 54071
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

Linden Parchment PE
PE# 54071

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CODES AND REFERENCES

Building Codes
Florida Building Code - 2014

ASCE7-10 Minimum Design Loads for Buildings and Other Structures

Material Codes
Concrete: Building Code Requirements for Structural Concrete, ACI 318 - 14

References
Precast Post Tension: PCI Design Handbook - 7th Edition

BUILDING INFORMATION

A. Exterior Building Dimensions and Weight Calculation
Building Length 18 ft 1 in
Building Width 10 ft 8 in

Building Eave Height 8 ft 3 inch

Panel Weights based on concrete weight = 150 pcf

P1	4,920 lb
P2	4,500 lb
P3	3,520 lb
P4	3,520 lb
P5	2,444 lb
P6	2,444 lb
R1	4,500 lb
R2	10,600 lb
F1	12,000 lb

Building Total = 48,428 lb

DESIGN LOADS

A. Florida Building Code -2014
1. Live Loads
a. Roof 20 psf
b. Floor 100 psf

2. Dead Loads
a. Roof 63 psf
b. Floor 63 psf
c. 3" Wall panels 38 psf

3. Wind Loads - ASCE 7-10, Chapters 28 and 30.
This structure is designed combining nominal loads using Allowable Stress Design. The load combinations and load factors for Strength Design are used only on the materials authorized by the applicable design standard.

4. Wind loading calculation (ASCE 7-10)
- Vult = 175 mph
- Risk Category II
- Roof Angle, 14 degrees
- Exposure Category C
- Enclosed Building

Wind Loads (psf) Based on the ASCE 7-10 Chapter 28
Wind pressures on Overhangs (Roof Panel R1) include $0.7C_p$

Components and Cladding Working Pressures ASCE 7-10 Chapter 30 (note, earlier versions of the calculations showed Ultimate Pressures).

ROOF ZONES
1. +28.3 PSF, -45 PSF
2. +28.3 PSF, -78 PSF
3. +28.3 PSF, -115.5 PSF

WALL ZONE
Lexan Window +49 PSF, -53 PSF
Door +47 PSF, -61 PSF

MATERIALS
Concrete Design Strength = 5,000 psi
Concrete Strength for Post Tensioning = 4,500 psi
Initial Concrete Strength = 3,500 psi
Post Tensioned Cable Specified Tension = 270 ksi, 41k Polystrand CP50, ½" diameter, 7 wire strand (ASTM A416)
Reinforcing Steel Yield Strength = 60 ksi
Welded Wire Fabric Strength = 65 ksi

MecaWind Pro v2.2.7.4 per ASCE 7-10

Developed by MECA Enterprises, Inc. Copyright www.mecaenterprises.com

Date	: 3/20/2017	Project No.	: 16-8-01
Company Name	: LCP Solutions LLC	Designed By	: Linden Parchment PE
Address	: 8002 Glitter Court	Description	: Montessouri Concrete Dbl RR
City	: Orlando	Customer Name	: Leesburg Concrete
State	: Florida	Proj Location	: Palmetto Bay FL

File Location: C:\WORK\Montessouri.wnd

Directional Procedure Simplified Diaphragm Building (Ch 27 Part 2)

Basic Wind Speed(V)	= 175.00 mph		
Structural Category	= II	Exposure Category	= C
Natural Frequency	= N/A	Flexible Structure	= No
Importance Factor	= 1.00	Kd Directional Factor	= 0.85
Alpha	= 9.50	Zg	= 900.00 ft
At	= 0.11	Bt	= 1.00
Am	= 0.15	Bm	= 0.65
Cc	= 0.20	l	= 500.00 ft
Epsilon	= 0.20	Zmin	= 15.00 ft
Pitch of Roof	= 3.437393 : 12	Slope of Roof(Theta)	= 15.98 Deg
h: Mean Roof Ht	= 9.17 ft	Type of Roof	= GABLED
RHt: Ridge Ht	= 10.00 ft	Eht: Eave Height	= 8.33 ft
OH: Roof Overhang at Eave	= 0.50 ft	Overhead Type	= OH w/ soffit
Bldg Length Along Ridge	= 18.00 ft	Bldg Width Across Ridge	= 10.66 ft

Gust Factor Calculations

Gust Factor Category I Rigid Structures - Simplified Method

Gust1: For Rigid Structures (Nat. Freq.>1 Hz) use 0.85 = 0.85

Gust Factor Category II Rigid Structures - Complete Analysis

Zm:	0.6*Ht	= 15.00 ft
lzm:	Cc*(33/Zm)^0.167	= 0.23
Lzm:	l*(Zm/33)^Epsilon	= 427.06 ft
Q:	(1/(1+0.63*((B+Ht)/Lzm)^0.63))^0.5	= 0.96
Gust2:	0.925*((1+1.7*lzm*3.4*Q)/(1+1.7*3.4*lzm))	= 0.90

Gust Factor Summary

Not a Flexible Structure use the Lessor of Gust1 or Gust2 = 0.85

Table 26.11-1 Internal Pressure Coefficients for Buildings, GCpi

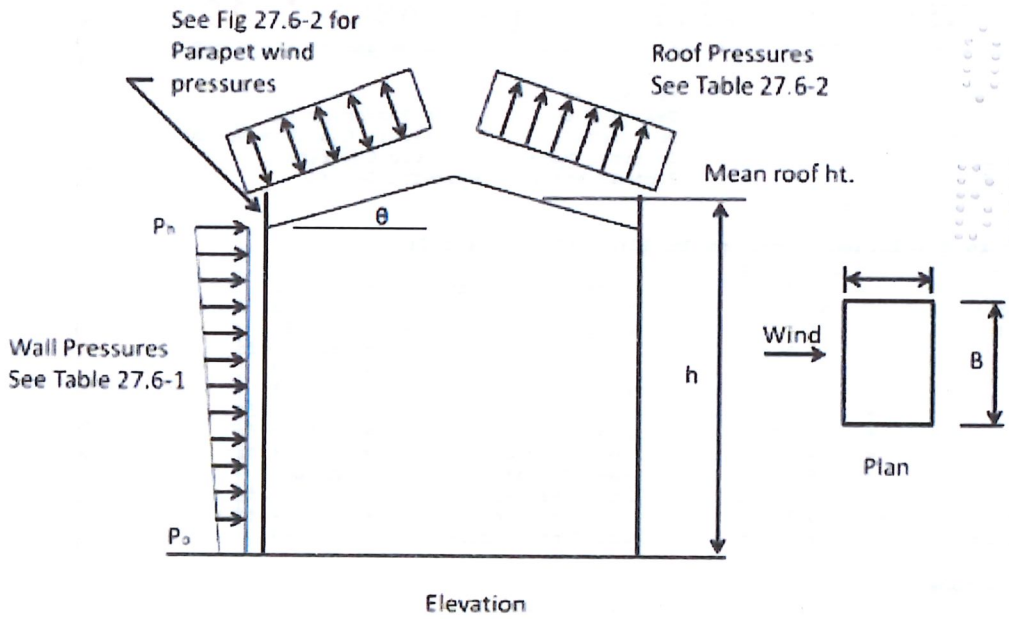
GCPi : Internal Pressure Coefficient = +/-0.18

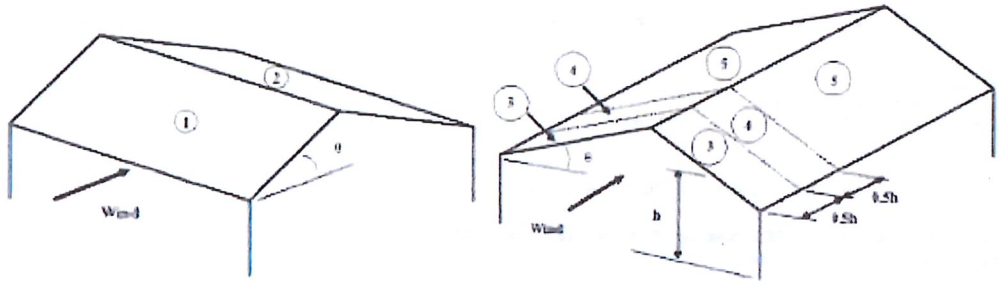
Topographic Adjustment

0.33*z	= 1.00
Kzt (0.33*z): Topographic factor at elevation 0.33*z	= 1.00
Vtopo: Adjust V per Para 27.5.2: V * [Kzt(0.33*z)]^0.5	= 175.00 mph

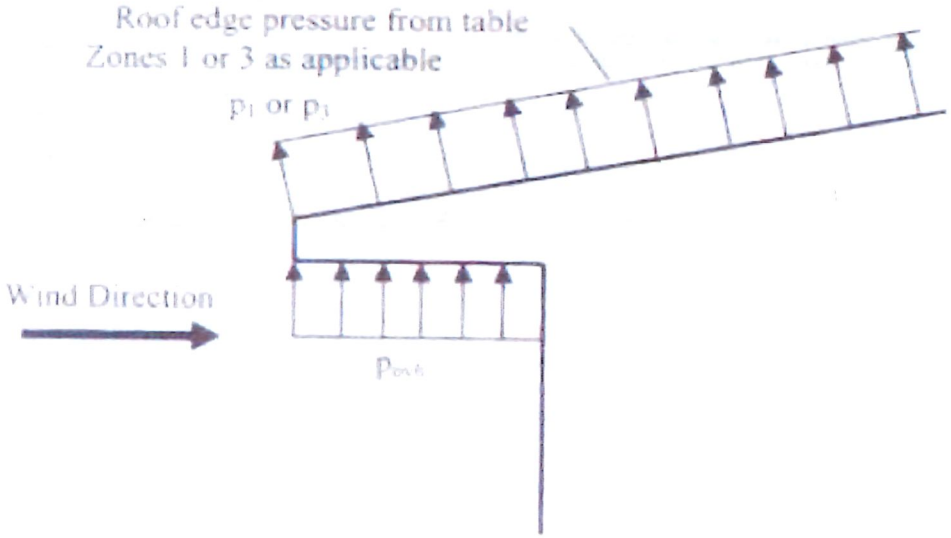
MWFRS Diaphragm Building Wind Pressures per Ch 27 Pt 2

All pressures shown are based upon STRENGTH Design, with a Load Factor of 1





Gabled Roof



MWFRS Pressures for Wind Normal to 18 ft wall (Normal to Ridge)

WALL PRESSURES PER TABLE 27.6-1

L/B:	Bldg Dim in Wind Dir / Bldg Dim Normal to Wind Dir	=	0.59
h:	Height to top of Windward Wall	=	8.33 ft
ph:	Net Pressure at top of wall (windward + leeward)	=	64.50 psf
p0:	Net Pressure at bottom of wall (windward + leeward)	=	64.50 psf
ps:	Side wall pressure acting away from wall = .54 * ph	=	-34.83 psf
pl:	Leeward wall pressure acting away from wall = .38 * ph	=	-24.51 psf
pwh:	Windward wall press @ top acting toward wall = ph-pl	=	39.99 psf
pw0:	Windward wall press @ bot acting toward wall = p0-pl	=	39.99 psf

ROOF PRESSURES PER TABLE 27.6-2

h:	Mean Roof Height	=	9.165 ft
Lambda:	Exposure Adjustment Factor	=	1.000
Slope:	Roof Slope	=	15.98 Deg

Zone	Load Case1 psf	Load Case2 psf
1	-54.24	12.25
2	-39.92	-14.30
3	-60.08	.00
4	-53.63	.00
5	-43.95	.00

Note: A value of '0' indicates that the zone/load case is not applicable.

ROOF OVERHANG LOADS (FIGURE 27.6-3):

LOAD CASE 1:

Povh1:	Overhang pressure for zone 1	=	-40.68 psf
Povh3:	Overhang pressure for zone 3	=	-45.06 psf

LOAD CASE 2:

Povh1:	Overhang pressure for zone 1	=	9.18 psf
Povh3:	Overhang pressure for zone 3	=	.00 psf

Normal to Ridge - Base Reactions - Walls+Roof +GCpi

Description	Press psf	Area ft^2	Fx Kip	Fy Kip	Fz Kip	Mx K-ft	My K-ft	Mz K-ft
Roof Windward	-54.24	100	.00	-1.49	5.20	0.1	.0	.0
Roof Leeward	-39.92	100	.00	1.10	3.83	-0.1	.0	.0
Windward Wall	39.99	18	.00	0.72	.00	5.6	.0	.0
Windward Wall	39.99	18	.00	0.72	.00	4.9	.0	.0
Windward Wall	39.99	18	.00	0.72	.00	4.2	.0	.0
Windward Wall	39.99	18	.00	0.72	.00	3.5	.0	.0
Windward Wall	39.99	18	.00	0.72	.00	2.8	.0	.0
Windward Wall	39.99	18	.00	0.72	.00	2.0	.0	.0
Windward Wall	39.99	18	.00	0.72	.00	1.3	.0	.0
Windward Wall	39.99	18	.00	0.72	.00	0.6	.0	.0
Windward Wall	39.99	6	.00	0.24	.00	0.0	.0	.0
Leeward Wall	-24.51	150	.00	3.68	.00	15.3	.0	.0
Side Wall	-34.83	89	-3.09	.00	.00	.0	12.9	.0
Side Wall	-34.83	89	3.09	.00	.00	.0	-12.9	.0
OH Bot Windward	-40.68	9	.00	.00	-0.37	-2.0	.0	.0
Side Wall	-34.83	10	-0.34	.00	.00	.0	3.0	.0
Side Wall	-34.83	10	0.34	.00	.00	.0	-3.0	.0
Total	.00	706	.00	9.28	8.67	38.3	.0	.0

Normal to Ridge - Base Reactions - Walls Only +GCpi

Description	Press psf	Area ft^2	Fx Kip	Fy Kip	Fz Kip	Mx K-ft	My K-ft	Mz K-ft
Windward Wall	39.99	18	.00	0.72	.00	5.6	.0	.0
Windward Wall	39.99	18	.00	0.72	.00	4.9	.0	.0
Windward Wall	39.99	18	.00	0.72	.00	4.2	.0	.0
Windward Wall	39.99	18	.00	0.72	.00	3.5	.0	.0
Windward Wall	39.99	18	.00	0.72	.00	2.8	.0	.0
Windward Wall	39.99	18	.00	0.72	.00	2.0	.0	.0
Windward Wall	39.99	18	.00	0.72	.00	1.3	.0	.0
Windward Wall	39.99	18	.00	0.72	.00	0.6	.0	.0

Windward Wall	39.99	6	.00	0.24	.00	0.0	.0	.0
Leeward Wall	-24.51	150	.00	3.68	.00	15.3	.0	.0
Side Wall	-34.83	89	-3.09	.00	.00	.0	12.9	.0
Side Wall	-34.83	89	3.09	.00	.00	.0	-12.9	.0
Side Wall	-34.83	10	-0.34	.00	.00	.0	3.0	.0
Side Wall	-34.83	10	0.34	.00	.00	.0	-3.0	.0
Total	.00	497	.00	9.67	.00	40.3	.0	.0

Normal to Ridge - Base Reactions - Walls+Roof -GCpi

Description	Press psf	Area ft^2	Fx Kip	Fy Kip	Fz Kip	Mx K-ft	My K-ft	Mz K-ft
Roof Windward	12.25	100	.00	0.34	-1.17	-0.0	.0	.0
Roof Leeward	-14.30	100	.00	0.39	1.37	-0.0	.0	.0
Windward Wall	39.99	18	.00	0.72	.00	5.6	.0	.0
Windward Wall	39.99	18	.00	0.72	.00	4.9	.0	.0
Windward Wall	39.99	18	.00	0.72	.00	4.2	.0	.0
Windward Wall	39.99	18	.00	0.72	.00	3.5	.0	.0
Windward Wall	39.99	18	.00	0.72	.00	2.8	.0	.0
Windward Wall	39.99	18	.00	0.72	.00	2.0	.0	.0
Windward Wall	39.99	18	.00	0.72	.00	1.3	.0	.0
Windward Wall	39.99	18	.00	0.72	.00	0.6	.0	.0
Windward Wall	39.99	6	.00	0.24	.00	0.0	.0	.0
Leeward Wall	-24.51	150	.00	3.68	.00	15.3	.0	.0
Side Wall	-34.83	89	-3.09	.00	.00	.0	12.9	.0
Side Wall	-34.83	89	3.09	.00	.00	.0	-12.9	.0
OH Bot Windward	9.18	9	.00	.00	0.08	0.5	.0	.0
Side Wall	-34.83	10	-0.34	.00	.00	.0	3.0	.0
Side Wall	-34.83	10	0.34	.00	.00	.0	-3.0	.0
Total	.00	706	.00	10.40	0.28	40.7	.0	.0

Normal to Ridge - Base Reactions - Walls Only -GCpi

Description	Press psf	Area ft^2	Fx Kip	Fy Kip	Fz Kip	Mx K-ft	My K-ft	Mz K-ft
Windward Wall	39.99	18	.00	0.72	.00	5.6	.0	.0
Windward Wall	39.99	18	.00	0.72	.00	4.9	.0	.0
Windward Wall	39.99	18	.00	0.72	.00	4.2	.0	.0
Windward Wall	39.99	18	.00	0.72	.00	3.5	.0	.0
Windward Wall	39.99	18	.00	0.72	.00	2.8	.0	.0
Windward Wall	39.99	18	.00	0.72	.00	2.0	.0	.0
Windward Wall	39.99	18	.00	0.72	.00	1.3	.0	.0
Windward Wall	39.99	18	.00	0.72	.00	0.6	.0	.0
Windward Wall	39.99	6	.00	0.24	.00	0.0	.0	.0
Leeward Wall	-24.51	150	.00	3.68	.00	15.3	.0	.0
Side Wall	-34.83	89	-3.09	.00	.00	.0	12.9	.0
Side Wall	-34.83	89	3.09	.00	.00	.0	-12.9	.0
Side Wall	-34.83	10	-0.34	.00	.00	.0	3.0	.0
Side Wall	-34.83	10	0.34	.00	.00	.0	-3.0	.0
Total	.00	497	.00	9.67	.00	40.3	.0	.0

Normal to Ridge - Base Reactions - Walls+Roof MIN

Description	Press psf	Area* ft^2	Fx Kip	Fy Kip	Fz Kip	Mx K-ft	My K-ft	Mz K-ft
Total	.00	0	.00	.00	.00	.0	.0	.0

Notes - Normal to Ridge

Note (1) X= Along Building ridge, Y = Normal to Building Ridge, Z = Vertical
Note (2) MIN = Minimum pressures on Walls = 16 psf and Roof = 8 psf
Note (3) Area* = Area of the surface projected onto a vertical plane normal to wind.

MWFRS Pressures for Wind Normal to 10.66 ft wall (Along Ridge)

WALL PRESSURES PER TABLE 27.6-1

L/B:	Bldg Dim in Wind Dir / Bldg Dim Normal to Wind Dir	=	1.69
h:	Height to top of Windward Wall	=	10.00 ft
ph:	Net Pressure at top of wall (windward + leeward)	=	58.73 psf
p0:	Net Pressure at bottom of wall (windward + leeward)	=	58.73 psf

```
ps:  Side wall pressure acting away from wall = .61 * ph      =   -35.76 psf
pl:  Leeward wall pressure acting away from wall = .3 * ph    =   -17.87 psf
pwh: Windward wall press @ top acting toward wall = ph-pl     =    40.86 psf
pw0: Windward wall press @ bot acting toward wall = p0-pl     =    40.86 psf
```

ROOF PRESSURES PER TABLE 27.6-2

```
h:       Mean Roof Height                                     =     9.165 ft
Lambda:  Exposure Adjustment Factor                           =     1.000
Slope:   Roof Slope                                           =    15.98 Deg
```

Zone	Load Case1 psf	Load Case2 psf
1	-54.24	12.25
2	-39.92	-14.30
3	-60.08	.00
4	-53.63	.00
5	-43.95	.00

Note: A value of '0' indicates that the zone/load case is not applicable.

ROOF OVERHANG LOADS (FIGURE 27.6-3):

```
LOAD CASE 1:
  Povh1: Overhang pressure for zone 1                         =   -40.68 psf
  Povh3: Overhang pressure for zone 3                         =   -45.06 psf

LOAD CASE 2:
  Povh1: Overhang pressure for zone 1                         =     9.18 psf
  Povh3: Overhang pressure for zone 3                         =      .00 psf
```

Along Ridge - Base Reactions - Walls+Roof +GCpi

Description	Press psf	Area ft^2	Fx Kip	Fy Kip	Fz Kip	Mx K-ft	My K-ft	Mz K-ft
Roof (0 to h/2)	-60.08	25	.00	-0.42	1.47	0.0	-9.8	-2.8
Roof (0 to h/2)	-60.08	25	.00	0.42	1.47	-0.0	-9.8	2.8
Roof (0 to h/2)	-60.08	2	.00	-0.04	0.14	0.4	-0.9	-0.3
Roof (0 to h/2)	-60.08	2	.00	0.04	0.14	-0.4	-0.9	0.3
Roof (h/2 to h)	-53.63	25	.00	-0.38	1.31	0.0	-2.8	-0.8
Roof (h/2 to h)	-53.63	25	.00	0.38	1.31	-0.0	-2.8	0.8
Roof (h/2 to h)	-53.63	2	.00	-0.04	0.12	0.4	-0.3	-0.1
Roof (h/2 to h)	-53.63	2	.00	0.04	0.12	-0.4	-0.3	0.1
Roof (h to 2h)	-43.95	49	.00	-0.59	2.07	0.0	9.5	2.7
Roof (h to 2h)	-43.95	49	.00	0.59	2.07	-0.0	9.5	-2.7
Roof (h to 2h)	-43.95	5	.00	-0.06	0.19	0.6	0.9	0.3
Roof (h to 2h)	-43.95	5	.00	0.06	0.19	-0.6	0.9	-0.3
Windward Wall	40.86	11	0.44	.00	.00	.0	-3.4	.0
Windward Wall	40.86	11	0.44	.00	.00	.0	-3.0	.0
Windward Wall	40.86	11	0.44	.00	.00	.0	-2.5	.0
Windward Wall	40.86	11	0.44	.00	.00	.0	-2.1	.0
Windward Wall	40.86	11	0.44	.00	.00	.0	-1.7	.0
Windward Wall	40.86	11	0.44	.00	.00	.0	-1.2	.0
Windward Wall	40.86	11	0.44	.00	.00	.0	-0.8	.0
Windward Wall	40.86	11	0.44	.00	.00	.0	-0.4	.0
Windward Wall	40.86	4	0.14	.00	.00	.0	-0.0	.0
Leeward Wall	-17.87	89	1.59	.00	.00	.0	-6.6	.0
Side Wall	-35.76	150	.00	5.36	.00	22.3	.0	.0
Side Wall	-35.76	150	.00	-5.36	.00	-22.3	.0	.0
Windward Wall	40.86	10	0.40	.00	.00	.0	-3.5	.0
Leeward Wall	-17.87	10	0.17	.00	.00	.0	-1.5	.0
Total	.00	715	5.79	.00	10.60	.0	-33.7	.0

Along Ridge - Base Reactions - Walls Only +GCpi

Description	Press psf	Area ft^2	Fx Kip	Fy Kip	Fz Kip	Mx K-ft	My K-ft	Mz K-ft
Windward Wall	40.86	11	0.44	.00	.00	.0	-3.4	.0
Windward Wall	40.86	11	0.44	.00	.00	.0	-3.0	.0
Windward Wall	40.86	11	0.44	.00	.00	.0	-2.5	.0
Windward Wall	40.86	11	0.44	.00	.00	.0	-2.1	.0
Windward Wall	40.86	11	0.44	.00	.00	.0	-1.7	.0
Windward Wall	40.86	11	0.44	.00	.00	.0	-1.2	.0

Windward Wall	40.86	11	0.44	.00	.00	.0	-0.8	.0
Windward Wall	40.86	11	0.44	.00	.00	.0	-0.4	.0
Windward Wall	40.86	4	0.14	.00	.00	.0	-0.0	.0
Leeward Wall	-17.87	89	1.59	.00	.00	.0	-6.6	.0
Side Wall	-35.76	150	.00	5.36	.00	22.3	.0	.0
Side Wall	-35.76	150	.00	-5.36	.00	-22.3	.0	.0
Windward Wall	40.86	10	0.40	.00	.00	.0	-3.5	.0
Leeward Wall	-17.87	10	0.17	.00	.00	.0	-1.5	.0
Total	.00	497	5.79	.00	.00	.0	-26.3	.0

Along Ridge - Base Reactions - Walls+Roof -GCpi

Description	Press psf	Area ft^2	Fx Kip	Fy Kip	Fz Kip	Mx K-ft	My K-ft	Mz K-ft
Windward Wall	40.86	11	0.44	.00	.00	.0	-3.4	.0
Windward Wall	40.86	11	0.44	.00	.00	.0	-3.0	.0
Windward Wall	40.86	11	0.44	.00	.00	.0	-2.5	.0
Windward Wall	40.86	11	0.44	.00	.00	.0	-2.1	.0
Windward Wall	40.86	11	0.44	.00	.00	.0	-1.7	.0
Windward Wall	40.86	11	0.44	.00	.00	.0	-1.2	.0
Windward Wall	40.86	11	0.44	.00	.00	.0	-0.8	.0
Windward Wall	40.86	11	0.44	.00	.00	.0	-0.4	.0
Windward Wall	40.86	4	0.14	.00	.00	.0	-0.0	.0
Leeward Wall	-17.87	89	1.59	.00	.00	.0	-6.6	.0
Side Wall	-35.76	150	.00	5.36	.00	22.3	.0	.0
Side Wall	-35.76	150	.00	-5.36	.00	-22.3	.0	.0
Windward Wall	40.86	10	0.40	.00	.00	.0	-3.5	.0
Leeward Wall	-17.87	10	0.17	.00	.00	.0	-1.5	.0
Total	.00	497	5.79	.00	.00	.0	-26.8	.0

Along Ridge - Base Reactions - Walls Only -GCpi

Description	Press psf	Area ft^2	Fx Kip	Fy Kip	Fz Kip	Mx K-ft	My K-ft	Mz K-ft
Windward Wall	40.86	11	0.44	.00	.00	.0	-3.4	.0
Windward Wall	40.86	11	0.44	.00	.00	.0	-3.0	.0
Windward Wall	40.86	11	0.44	.00	.00	.0	-2.5	.0
Windward Wall	40.86	11	0.44	.00	.00	.0	-2.1	.0
Windward Wall	40.86	11	0.44	.00	.00	.0	-1.7	.0
Windward Wall	40.86	11	0.44	.00	.00	.0	-1.2	.0
Windward Wall	40.86	11	0.44	.00	.00	.0	-0.8	.0
Windward Wall	40.86	11	0.44	.00	.00	.0	-0.4	.0
Windward Wall	40.86	4	0.14	.00	.00	.0	-0.0	.0
Leeward Wall	-17.87	89	1.59	.00	.00	.0	-6.6	.0
Side Wall	-35.76	150	.00	5.36	.00	22.3	.0	.0
Side Wall	-35.76	150	.00	-5.36	.00	-22.3	.0	.0
Windward Wall	40.86	10	0.40	.00	.00	.0	-3.5	.0
Leeward Wall	-17.87	10	0.17	.00	.00	.0	-1.5	.0
Total	.00	497	5.79	.00	.00	.0	-26.8	.0

Along Ridge - Base Reactions - Walls+Roof MIN

Description	Press psf	Area* ft^2	Fx Kip	Fy Kip	Fz Kip	Mx K-ft	My K-ft	Mz K-ft
Total	.00	0	.00	.00	.00	.0	.0	.0

Notes - Along Ridge

Note (1) X= Along Building ridge, Y = Normal to Building Ridge, Z = Vertical
Note (2) MIN = Minimum pressures on Walls = 16 psf and Roof = 8 psf
Note (3) Area* = Area of the surface projected onto a vertical plane normal to wind.

Total Base Reaction Summary

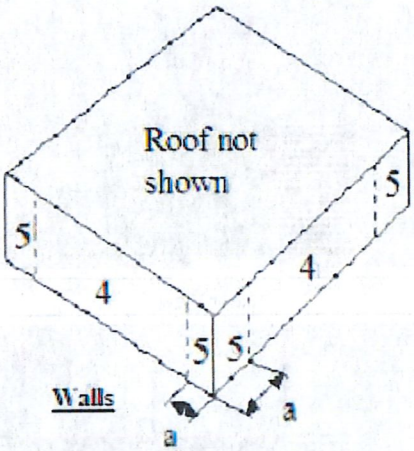
Description	Fx Kip	Fy Kip	Fz Kip	Mx K-ft	My K-ft	Mz K-ft
Normal to Ridge Walls+Roof +GCpi	.0	9.3	8.7	38.3	.0	.0
Normal to Ridge Walls Only +GCpi	.0	9.7	.0	40.3	.0	.0
Normal to Ridge Walls+Roof -GCpi	.0	10.4	0.3	40.7	.0	.0

Normal to Ridge Walls Only -GCpi	.0	9.7	.0	40.3	.0	.0
Normal to Ridge Walls+Roof MIN	.0	.0	.0	.0	.0	.0
Along Ridge Walls+Roof +GCpi	5.8	.0	10.6	.0	-33.7	.0
Along Ridge Walls Only +GCpi	5.8	.0	.0	.0	-26.8	.0
Along Ridge Walls+Roof -GCpi	5.8	.0	.0	.0	-26.8	.0
Along Ridge Walls Only -GCpi	5.8	.0	.0	.0	-26.9	.0
Along Ridge Walls+Roof MIN	.0	.0	.0	.0	.0	.0

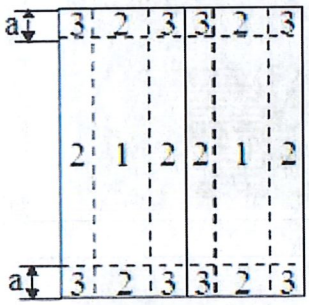
Notes Applying to MWFRS Reactions:

Note (1) Per Fig 27.4-1, Note 9, Use greater of Shear calculated with or without roof.
Note (2) X= Along Building ridge, Y = Normal to Building Ridge, Z = Vertical
Note (3) MIN = Minimum pressures on Walls = 16 psf and Roof = 8 psf
Note (4) MIN area is the area of the surface onto a vertical plane normal to wind.
Note (5) Total Roof Area (incl OH Top) = 199.60 sq. ft
Note (6) LC = Load Case (Some pressures can be zero, ref ASCE 7-10 Ch 27 Pt 2)

Wind Pressure on Components and Cladding (Ch 30 Part 1)



Walls



Gable Roof 7 < θ <= 45

All pressures shown are based upon STRENGTH Design, with a Load Factor of 1

Width of Pressure Coefficient Zone "a" = = 3.00 ft

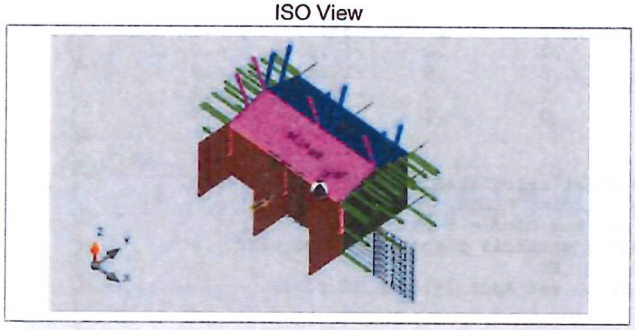
Description	Width ft	Span ft	Area ft^2	Zone	Max GCp	Min GCp	Max P psf	Min P psf
Lexan Window	1.00	3.00	3.0	4	1.00	-1.10	81.70	-88.62
Door	3.00	6.75	20.3	5	0.95	-1.29	77.95	-101.90
Roofing1	1.00	1.00	1.0	1	0.50	-0.90	47.08	-74.77
Roofing2	1.00	1.00	1.0	2	0.50	-1.70	47.08	-130.16
Roofing3	1.00	1.00	1.0	3	0.50	-2.60	47.08	-192.48

Khcc:Comp. & Clad. Table 6-3 Case 1 = 1.04
Qhcc:.00256*V^2*Khcc*Kht*Kd = 69.24 psf

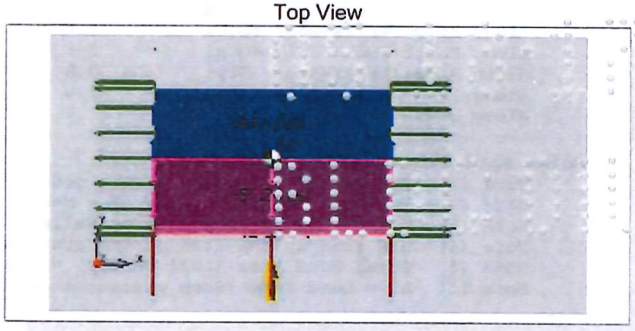
Wind Pressures - Main Wind Force Resisting System (MWFRS)

Acting on Length (Perpendicular to Ridge) AND Load Case 1

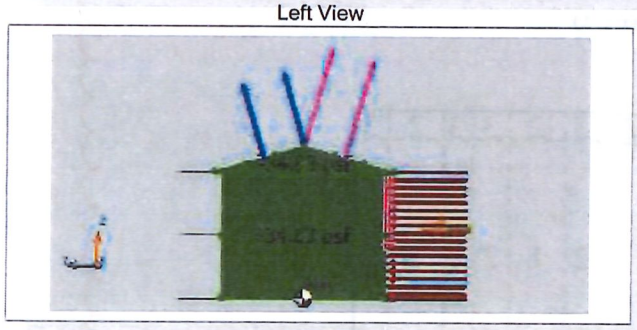
ISO View



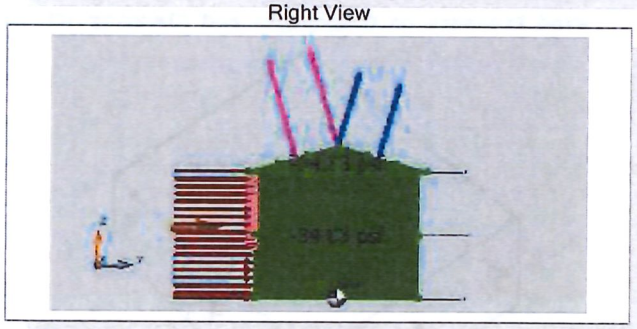
Top View



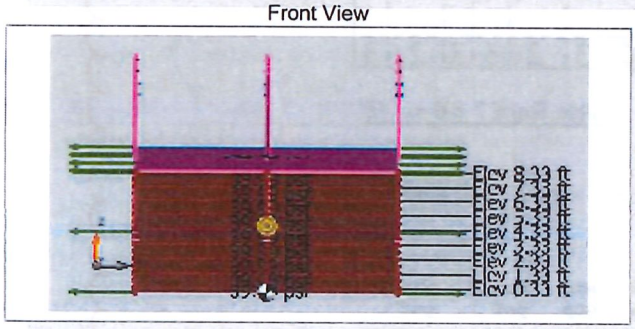
Left View



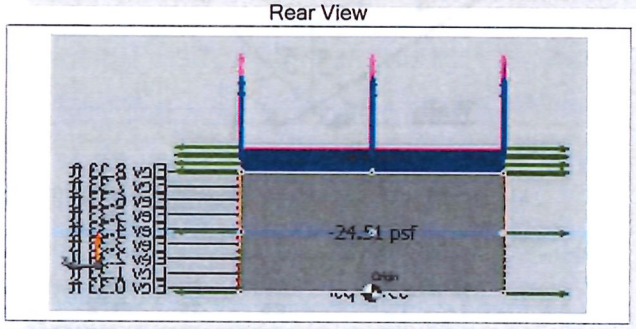
Right View



Front View



Rear View



Reactions at Building Base

Description	Fx (Kips)	Fy (Kips)	Fz (Kips)	Mx (Kip-ft)	My (Kip-ft)	Mz (Kip-ft)
Walls and Roof	.00	9.28	8.67	38.26	.00	.00
Walls Only	.00	9.67	.00	40.28	.00	.00

Pressures:

Roof Windward -54.24 psf

Roof Leeward -39.92 psf

Side Wall -34.83 psf

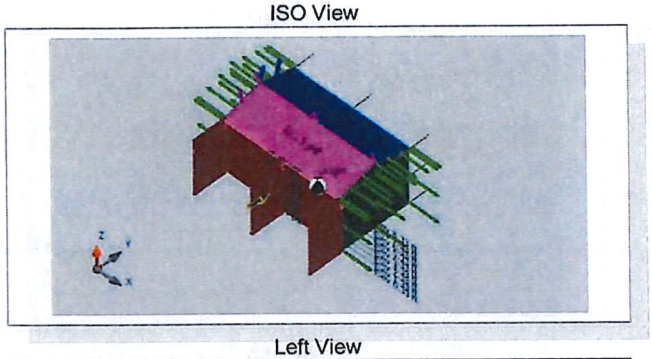
Windward Wall 39.99 psf

OH Bot Windward -40.68 psf

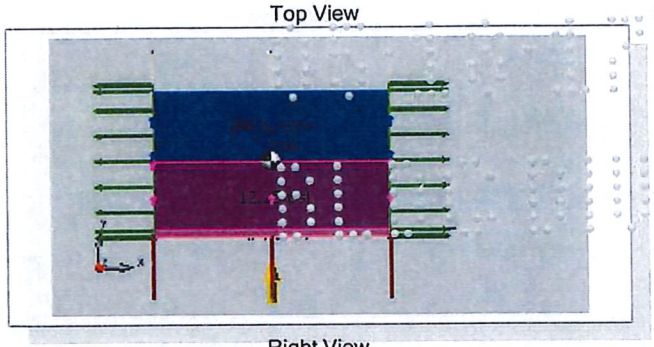
Wind Pressures - Main Wind Force Resisting System (MWFRS)

Acting on Length (Perpendicular to Ridge) AND Load Case 2

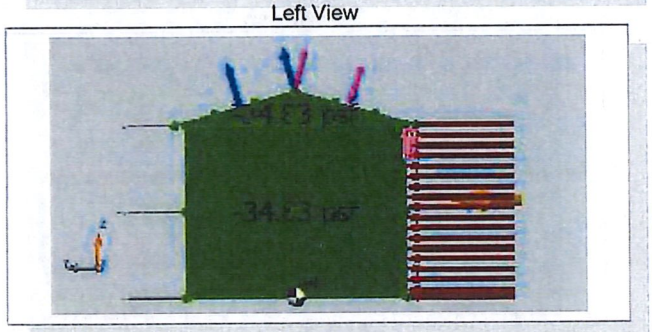
ISO View



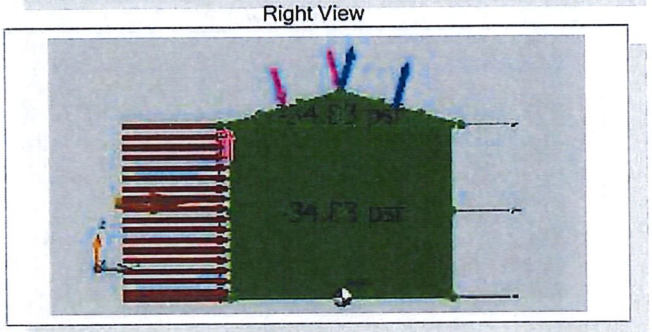
Top View



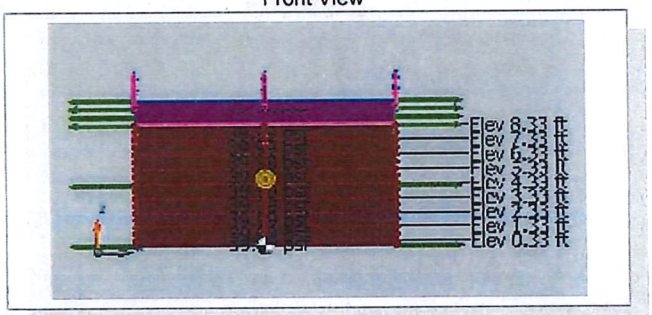
Left View



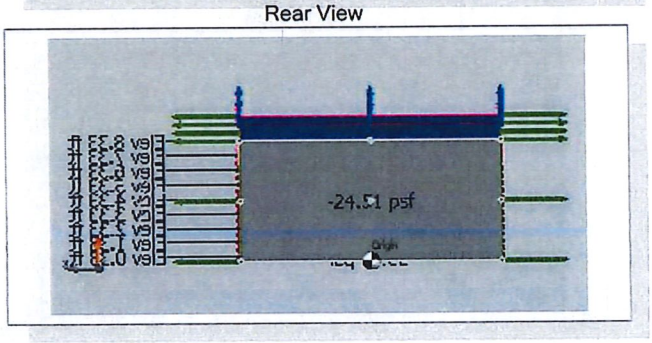
Right View



Front View



Rear View



Reactions at Building Base

Description	Fx (Kips)	Fy (Kips)	Fz (Kips)	Mx (Kip-ft)	My (Kip-ft)	Mz (Kip-ft)
Walls and Roof	.00	10.40	0.28	40.69	.00	.00
Walls Only	.00	9.67	.00	40.28	.00	.00

Pressures:

Roof Windward 12.25 psf

Leeward Wall -24.51 psf

Roof Leeward -14.3 psf

Side Wall -34.83 psf

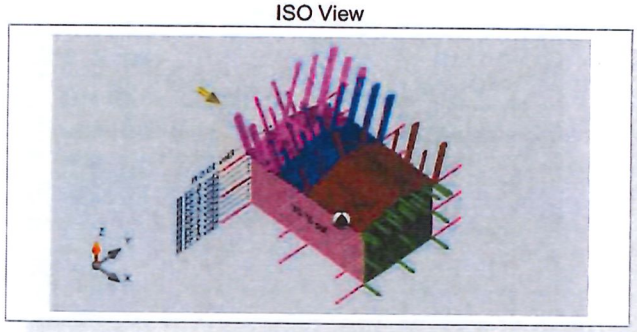
Windward Wall 39.99 psf

OH Bot Windward 9.18 psf

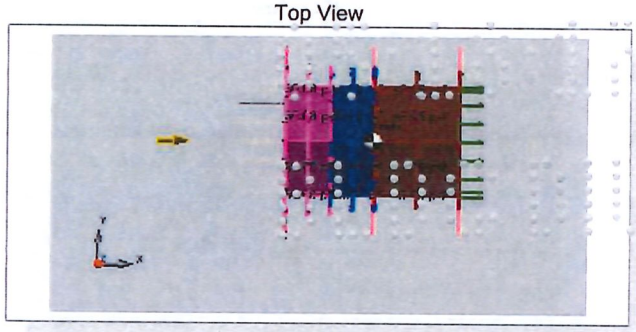
Wind Pressures - Main Wind Force Resisting System (MWFRS)

Acting on Width (Parallel to Ridge) AND Load Case 1

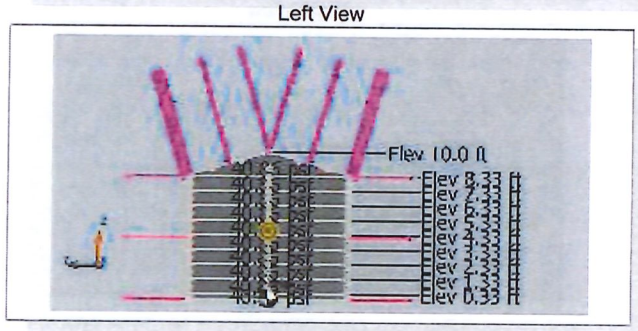
ISO View



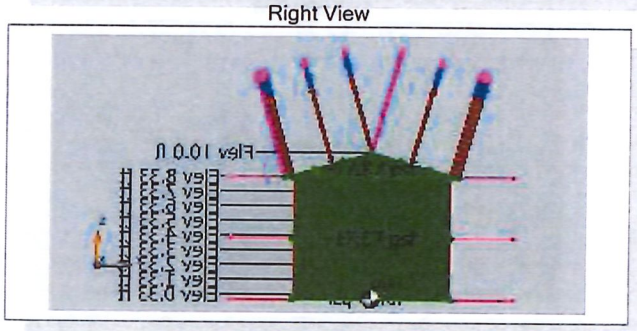
Top View



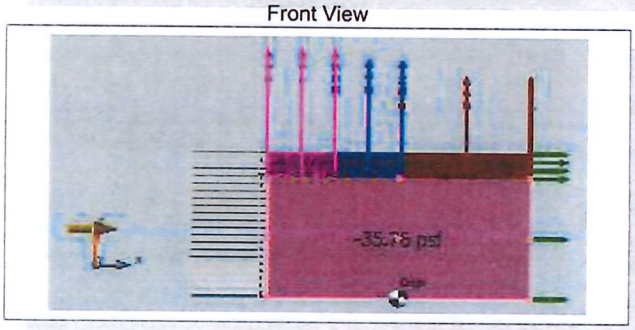
Left View



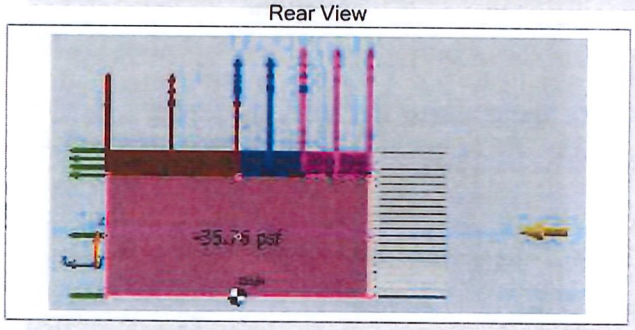
Right View



Front View



Rear View



Reactions at Building Base

Description	Fx (Kips)	Fy (Kips)	Fz (Kips)	Mx (Kip-ft)	My (Kip-ft)	Mz (Kip-ft)
Walls and Roof	5.79	.00	10.60	.00	-33.68	.00
Walls Only	5.79	.00	.00	.00	-26.80	.00

Pressures:

Roof (0 to h/2) -60.08 psf

Roof (h/2 to h) -53.63 psf

Roof (h to 2h) -43.95 psf

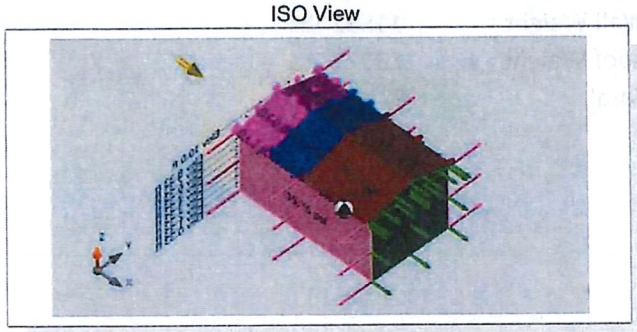
Leeward Wall -17.87 psf

Side Wall -35.76 psf

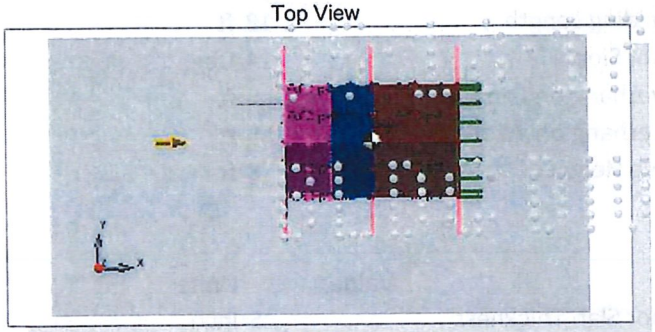
Wind Pressures - Main Wind Force Resisting System (MWFRS)

Acting on Width (Parallel to Ridge) AND Load Case 2

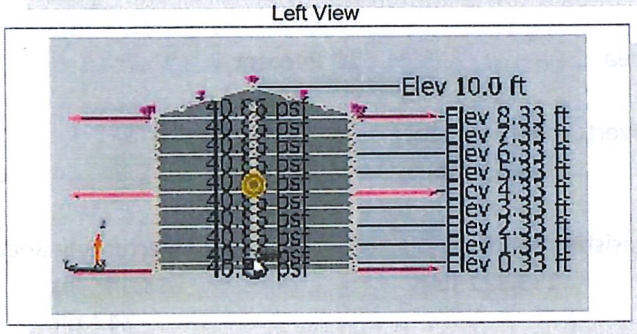
ISO View



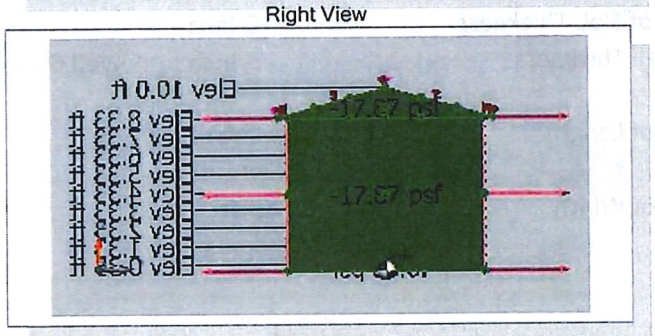
Top View



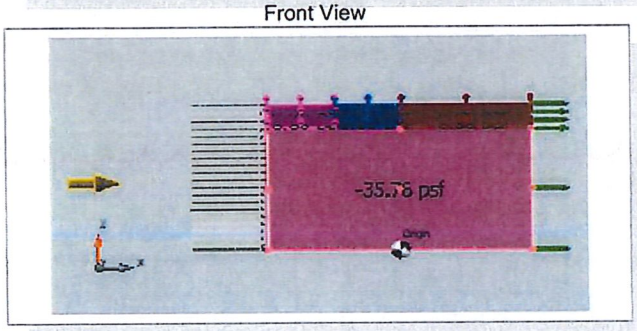
Left View



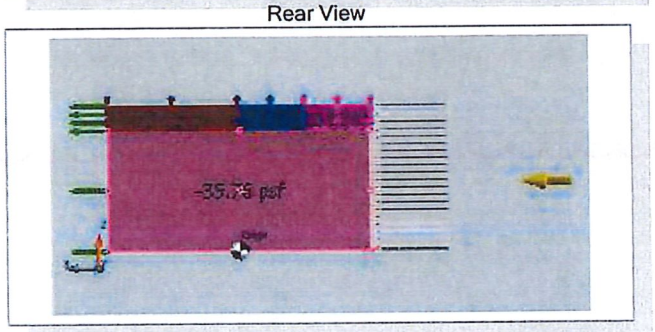
Right View



Front View



Rear View



Reactions at Building Base

Description	Fx (Kips)	Fy (Kips)	Fz (Kips)	Mx (Kip-ft)	My (Kip-ft)	Mz (Kip-ft)
Walls and Roof	5.79	.00	.00	.00	-26.80	.00
Walls Only	5.79	.00	.00	.00	-26.80	.00

Pressures:

Roof (0 to h/2) 0.0 psf
Windward Wall 40.86 psf

Roof (h/2 to h) 0.0 psf
Leeward Wall -17.87 psf

Roof (h to 2h) 0.0 psf
Side Wall -35.76 psf

Concrete Structure - Stability Calculation

Input Values

	Value	Units
Building Width	10.6	ft
Building Length	18	ft
Roof Slope	14	Deg
Overhang on Length	0.5	ft
Overhang on Width	0.5	ft
Eave Height	8	ft

Dead Loads	Factored Dead Load	Units
Floor Weight	10017	lbs
Wall Weight	11532	lbs
Roof Weight	13021	lbs
Total	34570	lbs

Input Values

	Value	Units
Floor Slab Thickness	5	Inch
Roof Slab Thickness	5	Inch
Wall Thickness	3	Inch
Roof Length	19.1	ft
Roof Width	12.6	ft

Wall Open Area 20 Percent

Overturning Moment
40,690 lb-ft

OTSF
5 OK

Resisting Moment
183,221 lb-ft

Overturning Reaction
13,446 lbs
747 plf

Design of Precast and Prestressed Concrete Flexure Panel - Roof Panel per ACI 318-14

Job: **Outback Sierra Restroom Building Roof Panel R1**

Combination of Prestressed and Non-Prestressed Reinforcement

		Value	Unit
Compressive Strength of Concrete	f'_c	5000	psi
Yield Strength of Reinforcing	f_y	60000	psi
Applied Dead Load	DL	2	psf
Applied Live Load	LL	20	psf
Applied Wind Load	WL	-60	psf
Span of Roof Panel	L	17.5	feet
Design Width	b	20	inch
Slab Depth	h	5	inch
Depth to Reinforcing	d	3	inch
Depth to Prestress	d'	2.5	inch
Calculated Dead Load of Slab		62.5	plf
Reduction Factor	ϕ	0.9	
Main Reinforcing	2	#5 Rebar	
Spacing	s	3	inch
Temperature Reinforcing		#4 Rebar	
Spacing		12	inch
Area of Prestressing Steel	A_{ps}	0.15	sq. inch
Prestress Factor (ACI 318 Chap 18)	γ_p	0.28	
	β_1	0.85	
Ultimate Prestress	f_{pu}	270	ksi
	As =	0.62	sq. inch
Prestress Losses		25	%

Load Combinations

U = 1.2D + 1.6L + 0.5W (16-3)

U = 1.2D + 0.5Lr + 1.0W (16-4)

U = 0.9D + 1.0W (16-6)

Concrete Density = 150 pcf

Ec = 4286.83 ksi

β_1 = 0.8 (.8 for f'c = 5 ksi)

n= 6.8

Unfactored Dead Load = 65.8333 plf

Unfactored Live Load = 33.3333 plf

Unfactored Wind Load = -100 plf

Applied Moment(s) $\boxed{\frac{wl^2}{8}}$

Jacking Stress 0.8 f_{pu}

#3 Bar = 0.11, #4 Bar = 0.2, #5 Bar = 0.31

WWF: 4x4 W4 x W4 = 0.12 at Fy = 65 ksi

a =	0.437647059	inch
a_{ps} =	0.000427689	
c =	0.547058824	inch
ε_1 =	0.013451613	> 0.005 for tension control
ω =	0.124	
ρ_p	0.003	
f_{ps}	242.3570824	ksi
P_{se}	24.3	kips
f_r	530.3300859	psi

M_{sus} 3796.22 lb-ft

Δ_i 0.9

Nominal Strength, M_n =	8,629	lb-ft
ϕM_n=	7,766	lb-ft
I_{cr}=	26.3	in^4
I_g=	208.3	in^4
I_e=	40.4	in^4
M_{cr} =	3,683	lb-ft

Mcr < ϕMn **OK**

Load Combinations for Strength Design

	(16-3)	(16-4)	(16-6)	
Factored Loads	82.3333	-4.333	-40.75	plf
Deflection	0.9076	-0.048	-0.4492	inch
Deflection Check (l/240)	**No Good**	**OK**	**OK**	
M_u =	5,066	-2,463	-3,857	lb-ft
ϕMn > Mu ?	**OK**	**OK**	**OK**	

Check Total Reinforcing **OK**

1605.2 Load combinations using strength design or load and resistance factor design.
1605.2.1 Basic load combinations.
Where strength design or load and resistance factor design is used, structures and portions thereof shall resist the most critical effects from the following combinations of factored loads:
1.2 D + 1.6 L + 0.5 Lr (Equation 16-2)
1.2 D + 1.6 Lr + 0.5 W (Equation 16-3)
1.2 D + 1.0 W + 0.5 Lr (Equation 16-4)
0.9 D + 1.0 W (Equation 16-6)

Design of Precast and Prestressed Concrete Flexure Panel - Roof Panel per ACI 318-14

Job: **Montessouri Restroom Building Roof Panel R2**

Combination of Prestressed and Non-Prestressed Reinforcement

		Value	Unit
Compressive Strength of Concrete	f'_c	5000	psi
Yield Strength of Reinforcing	f_y	60000	psi
Applied Dead Load	DL	2	psf
Applied Live Load	LL	20	psf
Applied Wind Load	WL	-60	psf
Span of Roof Panel	L	7.6	feet
Design Width	b	12	inch
Slab Depth	h	5	inch
Depth to Reinforcing	d	3	inch
Depth to Prestress	d'	2.5	inch
Calculated Dead Load of Slab		62.5	plf
Reduction Factor	ϕ	0.9	
Main Reinforcing	1	#4 Rebar	
Spacing	s	12	inch
Temperature Reinforcing		#4 Rebar	
Spacing		12	inch
Area of Prestressing Steel	A_{ps}	0.15	sq. inch
Prestress Factor (ACI 318 Chap 18)	γ_p	0.28	
	β_1	0.85	
Ultimate Prestress	f_{pu}	270	ksi
	As =	0.2	sq. inch
Prestress Losses		25	%
	a =	0.235294118	inch
	a_{ps} =	0.000702561	
	c =	0.294117647	inch
	ε_1 =	0.0276 > 0.005 for tension control	
	ω =	0.066666667	
	ρ_p	0.005	
	f_{ps}	238.8705882	ksi
	P_{se}	24.3	kips
	f_r	530.3300859	psi

Load Combinations

U = 1.2D + 1.6L + 0.5W (16-3)

U = 1.2D + 0.5Lr + 1.0W (16-4)

U = 0.9D + 1.0W (16-6)

Concrete Density = 150 pcf

Ec = 4266.83 ksi

β_1 = 0.8 (.8 for f'c = 5 ksi)

n= 6.8

Unfactored Dead Load = 64.5 plf

Unfactored Live Load = 20 plf

Unfactored Wind Load = -60 plf

Applied Moment(s) $\frac{wl^2}{8}$

Jacking Stress 0.8 f_{pu}

#3 Bar = 0.11, #4 Bar = 0.2, #5 Bar = 0.31

WWF: 4x4 W4 x W4 = 0.12 at Fy = 65 ksi

M_{sus} 610.09 lb-ft

Δ_i 47.5

Nominal Strength, M_n =	2,890	lb-ft
ϕM_n=	2,601	lb-ft
I_{cr}=	10.0	in^4
I_g=	125.0	in^4
I_e=	1226.4	in^4
M_{cr}=	2,210	lb-ft

Mcr < ϕMn **OK**

Load Combinations for Strength Design

	(16-3)	(16-4)	(16-6)	
Factored Loads	79.4	27.4	-1.95	plf
Deflection	0.00091	0.0003	-2E-05	inch
Deflection Check (l/240)	OK	OK	OK	
M_u =	790	-62	-274	lb-ft
ϕMn > Mu ?	OK	OK	OK	

Check Total Reinforcing **OK**

1605.2 Load combinations using strength design or load and resistance factor design.
1605.2.1 Basic load combinations.
Where strength design or load and resistance factor design is used, structures and portions thereof shall resist the most critical effects from the following combinations of factored loads:
1.2 D + 1.6 L + 0.5 Lr (Equation 16-2)
1.2 D + 1.6 Lr + 0.5 W (Equation 16-3)
1.2 D + 1.0 W + 0.5 Lr (Equation 16-4)
0.9 D + 1.0 W (Equation 16-6)

Design of Precast and Prestressed Concrete Flexure Panel - Wall Panel per ACI 318-14

Job: Montessouri Restroom - Panel P1

Non-Prestressed Panel

		Value	Unit
Compressive Strength of Concrete	f'_c	5000	psi
Yield Strength of Reinforcing	f_y	60000	psi
Applied Dead Load (Cladding)	DL	2	psf
Applied Roof Live Load	LL	153	plf
Applied Wind Load	WL	41	psf
Vertical Span of Wall Panel	L	9.83	feet
Design Width	b	12	inch
Wall Panel Depth	h	3	inch
Depth to Reinforcing	d	1.5	inch
Superimposed Dead Load on Wall	P_{u1}	483	plf
Calculated Dead Load of Wall	P_{u2}	37.5	psf
Reduction Factor	Φ	0.876816	
Main Reinforcing	1	#4 Bar	
Spacing		12	inch
Temperature Reinforcing		W4 WWF	
Spacing		12	inch
Eccentricity	e=	0.5	inch

FBC 1605.2.1 Basic load combinations

1.2 D + 1.6 Lr + 0.5 W (Equation 16-3)

1.2 D + 1.0 W + 0.5 Lr (Equation 16-4)

0.9 D + 1.0 W (Equation 16-6)

Concrete Density =	150	pcf
Ec =	4286.83	ksi
β_1 =	0.8	(.8 for f'c = 5000 psi)
n=	6.8	
Applied Moment(s)	$\frac{wl^2}{8} + Pdelta$	
Unfactored Dead Load =	522.5	plf
Unfactored Roof Live Load =	153	plf
Unfactored Wind Load =	41	plf
0.9 Dead Load =	470.25	plf
1.2 Dead Load =	627	plf
0.5 Live Load =	76.5	plf
1.6 Live Load =	244.8	plf
0.5 Wind Load =	20.5	plf
1.0 Wind Load =	41	plf

A_s =	0.12	sq. inch/ft	#3 Bar = 0.11, #4 Bar = 0.2, #5 Bar = 0.31
$A_{s(eff)}$ =	0.12805	sq. inch/ft	WWF: 4x4 W4 x W4 = 0.12 at Fy = 65 ksi
a=	0.160117647	inch	
c=	0.200147059	inch	
ε_1 =	0.019483468 > 0.005 for tension control		

Nominal Strength, M_n =	909	lb-ft
ΦM_n=	797	lb-ft
I_{cr} =	6.8	in^4
I_g =	27.0	in^4
M_{cr} =	795	lb-ft

Mcr < ϕMn **OK**

Load Combinations for Strength Design

	(16-3)	(16-4)	(16-6)	
Factored Loads	916.8	814.1	569.6	plf
Deflection Δ_u	0.01205	0.023469	0.023636	inch
Deflection Δ_{cr}	0.03713			inch
Deflection Δ_s	0.01205	0.023469	0.023636	inch
Deflection Check (l/150)	OK	OK	OK	
M_n =	909			lb-ft
ΦM_n=	797			lb-ft
M_u =	258.116	502.7636	506.3378	lb-ft
ϕMn > Mu ?	OK	OK	OK	
Mcr < ϕMn	OK			
Check Total Reinforcing	OK			

1605.2 Load combinations using strength design or load and resistance factor design.
1605.2.1 Basic load combinations.
Where strength design or load and resistance factor design is used, structures and portions thereof shall resist the most critical effects from the following combinations of factored loads:
1.2 D + 1.6 L + 0.5 Lr (Equation 16-2)
1.2 D + 1.6 Lr + 0.5 W (Equation 16-3)
1.2 D + 1.0 W + 0.5 Lr (Equation 16-4)
0.9 D + 1.0 W (Equation 16-6)

Design of Precast and Prestressed Concrete Flexure Panel - Wall Panel per ACI 318-14

Job: Montessouri Restroom - Panel P2

Non-Prestressed Panel

		Value	Unit
Compressive Strength of Concrete	f'_c	5000	psi
Yield Strength of Reinforcing	f_y	65000	psi
Applied Dead Load (Cladding)	DL	2	psf
Applied Roof Live Load	LL	90	plf
Applied Wind Load	WL	41	psf
Vertical Span of Wall Panel	L	8	feet
Design Width	b	12	inch
Wall Panel Depth	h	3	inch
Depth to Reinforcing	d	1.5	inch
Superimposed Dead Load on Wall	P_{u1}	315	plf
Calculated Dead Load of Wall	P_{u2}	37.5	psf
Reduction Factor	Φ	0.88488	
Main Reinforcing	1	W4 WWF	
Spacing		12	inch
Temperature Reinforcing		W4 WWF	
Spacing		12	inch
Eccentricity	e=	0.5	inch

FBC 1605.2.1 Basic load combinations

1.2 D + 1.6 Lr + 0.5 W (Equation 16-3)

1.2 D + 1.0 W + 0.5 Lr (Equation 16-4)

0.9 D + 1.0 W (Equation 16-6)

Concrete Density =	150	pcf
Ec =	4286.83	ksi
β_1 =	0.8	(.8 for f'c = 5000 psi)
n=	6.8	
Applied Moment(s)	$\frac{wl^2}{8} + Pdelta$	
Unfactored Dead Load =	354.5	plf
Unfactored Roof Live Load =	90	plf
Unfactored Wind Load =	41	plf
0.9 Dead Load =	319.05	plf
1.2 Dead Load =	425.4	plf
0.5 Live Load =	45	plf
1.6 Live Load =	144	plf
0.5 Wind Load =	20.5	plf
1.0 Wind Load =	41	plf

A_s =	0.12	sq. inch/ft	#3 Bar = 0.11, #4 Bar = 0.2, #5 Bar = 0.31
$A_{s(eff)}$ =	0.124846154	sq. inch/ft	WWF: 4x4 W4 x W4 = 0.12 at Fy = 65 ksi
a=	0.165294118	inch	
c=	0.206617647	inch	
ε_1 =	0.018779359	> 0.005 for tension control	

Nominal Strength, M_n =	958	lb-ft
$ΦM_n$=	848	lb-ft
I_{cr} =	6.6	in^4
I_g =	27.0	in^4
M_{cr} =	795	lb-ft
Mcr < ϕMn	OK	

Load Combinations for Strength Design

	(16-3)	(16-4)	(16-6)	
Factored Loads	614.4	581	418.4	plf
Deflection Δ_u	0.00656	0.012927	0.012994	inch
Deflection Δ_{cr}	0.03112			inch
Deflection Δ_s	0.00656	0.012927	0.012994	inch
Deflection Check (l/150)	OK	OK	OK	
M_n =	958			lb-ft
$ΦM_n$=	848			lb-ft
M_u =	167.736	330.4432	332.1456	lb-ft
ϕMn > Mu ?	OK	OK	OK	
Mcr < ϕMn	OK			
Check Total Reinforcing	OK			

1605.2 Load combinations using strength design or load and resistance factor design.
1605.2.1 Basic load combinations.
Where strength design or load and resistance factor design is used, structures and portions thereof shall resist the most critical effects from the following combinations of factored loads:
1.2 D + 1.6 L + 0.5 Lr (Equation 16-2)
1.2 D + 1.6 Lr + 0.5 W (Equation 16-3)
1.2 D + 1.0 W + 0.5 Lr (Equation 16-4)
0.9 D + 1.0 W (Equation 16-6)

Design of Precast and Prestressed Concrete Flexure Panel - Wall Panel per ACI 318-14

Job: Montessouri Restroom - Panel P3 / P4

Non-Prestressed Panel

		Value	Unit
Compressive Strength of Concrete	f'_c	5000	psi
Yield Strength of Reinforcing	f_y	60000	psi
Applied Dead Load (Cladding)	DL	2	psf
Applied Roof Live Load	LL	20	plf
Applied Wind Load	WL	41	psf
Vertical Span of Wall Panel	L	9.5	feet
Design Width	b	44	inch
Wall Panel Depth	h	3	inch
Depth to Reinforcing	d	1.5	inch
Superimposed Dead Load on Wall	P_{u1}	126	plf
Calculated Dead Load of Wall	P_{u2}	37.5	psf
Reduction Factor	Φ	0.868752	
Main Reinforcing	2	#5 Bar	
Spacing		3	inch
Temperature Reinforcing		W4 WWF	
Spacing		12	inch
Eccentricity	e=	1	inch

FBC 1605.2.1 Basic load combinations

1.2 D + 1.6 Lr + 0.5 W (Equation 16-3)

1.2 D + 1.0 W + 0.5 Lr (Equation 16-4)

0.9 D + 1.0 W (Equation 16-6)

Concrete Density =	150	pcf
Ec =	4286.83	ksi
β_1 =	0.8	(.8 for f'c = 5000 psi)
n=	6.8	
Applied Moment(s)	$\frac{wl^2}{8} + Pdelta$	
Unfactored Dead Load =	270.833	plf
Unfactored Roof Live Load =	73.3333	plf
Unfactored Wind Load =	150.333	plf
0.9 Dead Load =	243.75	plf
1.2 Dead Load =	325	plf
0.5 Live Load =	36.6667	plf
1.6 Live Load =	117.333	plf
0.5 Wind Load =	75.1667	plf
1.0 Wind Load =	150.333	plf

A_s =	0.62	sq. inch/ft	#3 Bar = 0.11, #4 Bar = 0.2, #5 Bar = 0.31
$A_{s(eff)}$ =	0.6221	sq. inch/ft	WWF: 4x4 W4 x W4 = 0.12 at Fy = 65 ksi
a=	0.200278075	inch	
c=	0.250347594	inch	
ε_1 =	0.014975008 > 0.005 for tension control		

Nominal Strength, M_n =	4354	lb-ft
ΦM_n=	3783	lb-ft
I_{cr}=	32.0	in^4
I_g=	99.0	in^4
M_{cr} =	2917	lb-ft
Mcr < ϕMn	OK	

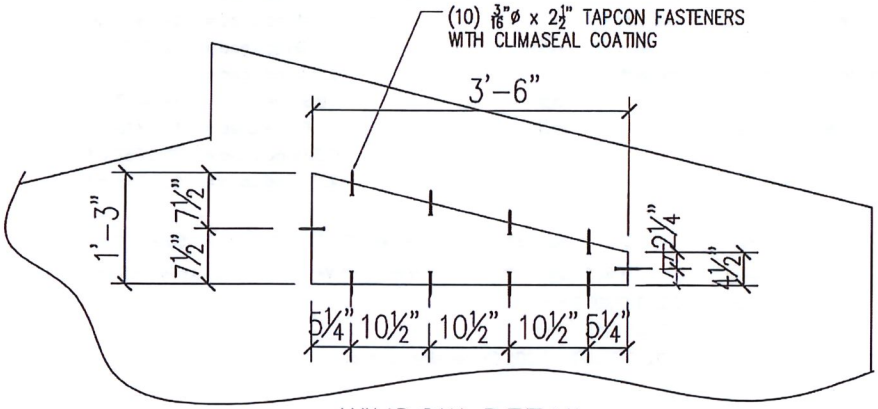
Load Combinations for Strength Design

	(16-3)	(16-4)	(16-6)	
Factored Loads	487.333	647.2	518.0333	plf
Deflection Δ_u	0.00818	0.016312	0.016326	inch
Deflection Δ_{cr}	0.02801			inch
Deflection Δ_s	0.00818	0.016312	0.016326	inch
Deflection Check (l/150)	OK	OK	OK	
M_n =	4354			lb-ft
ΦM_n=	3783			lb-ft
M_u =	851.592	1698.46	1699.927	lb-ft
ϕMn > Mu ?	OK	OK	OK	
Mcr < ϕMn	OK			
Check Total Reinforcing	OK			

1605.2 Load combinations using strength design or load and resistance factor design.
1605.2.1 Basic load combinations.
Where strength design or load and resistance factor design is used, structures and portions thereof shall resist the most critical effects from the following combinations of factored loads:
1.2 D + 1.6 L + 0.5 Lr (Equation 16-2)
1.2 D + 1.6 Lr + 0.5 W (Equation 16-3)
1.2 D + 1.0 W + 0.5 Lr (Equation 16-4)
0.9 D + 1.0 W (Equation 16-6)

Design Objective:
To design the $\frac{1}{4}$" thick Lexan Windows and the anchors.

LOADS:
WIND
Components and Cladding Wind Pressure = −88.6 psf



WINDOW DETAIL

Anchor Design:
Maximum tributary area for any fastener = 1 square foot. Maximum shear = 88.6 lbs
Data for $\frac{3}{16}$" Diameter Tapcon fastener with Climaseal coating:
Ultimate Shear Capacity = 860 lbs
Factor of Safety to ultimate = 5
Edge correction factor = 0.70

Allowable shear in concrete per fastener = (860 lb)/5 x 0.70 = 120 lbs (> 88.6 lbs OK)
Allowable shear in wood per fastener = 90 lb (> 88.6 lb OK)

Polycarbonate (Lexan) flexural strength = 12,000 psi @ 73° F
Factor of Safety = 5
so
Allowable flexural strength = 2400 psi
Tributary width = 10.5 inches
Section Modulus = (bd^2)/6 = (12*0.25^2)/6 = 0.125 in^3
Max span = 12 inch
Maximum moment due to wind = (WL^2)/8 = [(88.6/12)(12x12)]/8
= 133 in−lb
Maximum bending stress = Moment/ Section Modulus
= 133/0.125
= 1064 psi < 2400 psi (OK)

Design of Precast and Prestressed Concrete Flexure Panel - Floor Panel per ACI 318-14

Job: **Montessouri Restroom Building (During Lift)**

Combination of Prestressed and Non-Prestressed Reinforcement

		Value	Unit
Compressive Strength of Concrete	f'_c	5000	psi
Yield Strength of Reinforcing	f_y	60000	psi
Applied Dead Load	DL	2	psf
Applied Live Load	LL	0	psf
Applied Wind Load	WL	0	psf
Span of Floor Panel	L	18	feet
Design Width	b	12	inch
Slab Depth	h	5	inch
Depth to Reinforcing	d	3.75	inch
Depth to Prestress	d'	3	inch
Calculated Dead Load of Slab		62.5	plf
Reduction Factor	ϕ	0.9	
Main Reinforcing	1	#4 Rebar	
Spacing	s	12	inch
Temperature Reinforcing		#4 Rebar	
Spacing		12	inch
Area of Prestressing Steel	A_{ps}	0.15	sq. inch
Prestress Factor (ACI 318 Chap 18)	γ_p	0.28	
	β_1	0.85	
Ultimate Prestress	f_{pu}	270	ksi
	As =	0.2	sq. inch
Prestress Losses		20	%

Load Combinations

U = 1.2D + 1.6L + 0.5W (16-3)

U = 1.2D + 0.5Lr + 1.0W (16-4)

U = 0.9D + 1.0W (16-6)

Concrete Density =	150	pcf
Ec =	4286.83	ksi
β_1 =	0.8	(.8 for f'c = 5 ksi)
n=	6.8	
Unfactored Dead Load =	64.5	plf
Unfactored Live Load =	0	plf
Unfactored Wind Load =	0	plf
Applied Moment(s)	$\frac{wl^2}{8}$	
Jacking Stress	0.8	f_{pu}

#3 Bar = 0.11, #4 Bar = 0.2, #5 Bar = 0.31

WWF: 4x4 W4 x W4 = 0.12 at Fy = 65 ksi

a =	0.235294118	inch
a_{ps} =	0.00071782	
c =	0.294117647	inch
ε_1 =	0.03525 > 0.005 for tension control	
ω =	0.053333333	
ρ_p	0.004166667	
f_{ps}	244.0588235	ksi
P_{se}	25.92	kips
f_r	530.3300859	psi

M_{sus}	2612.25	lb-ft
Δ_i	0.6	

Nominal Strength, M_n =	3,642	lb-ft
ϕM_n=	3,277	lb-ft
I_{cr}=	16.3	in^4
I_g=	125.0	in^4
I_e=	31.8	in^4
M_{cr} =	2,210	lb-ft

Mcr < ϕMn **OK**

Load Combinations for Strength Design

	(16-3)	(16-4)	(16-6)	
Factored Loads	77.4	77.4	58.05	plf
Deflection	0.8396	0.8396	0.6297	inch
Deflection Check (l/240)	OK	OK	OK	
M_u =	3,135	3,135	2,351	lb-ft
ϕMn > Mu ?	OK	OK	OK	

Check Total Reinforcing **OK**

1605.2 Load combinations using strength design or load and resistance factor design.
1605.2.1 Basic load combinations.
Where strength design or load and resistance factor design is used, structures and portions thereof shall resist the most critical effects from the following combinations of factored loads:
1.2 D + 1.6 L + 0.5 Lr (Equation 16-2)
1.2 D + 1.6 Lr + 0.5 W (Equation 16-3)
1.2 D + 1.0 W + 0.5 Lr (Equation 16-4)
0.9 D + 1.0 W (Equation 16-6)

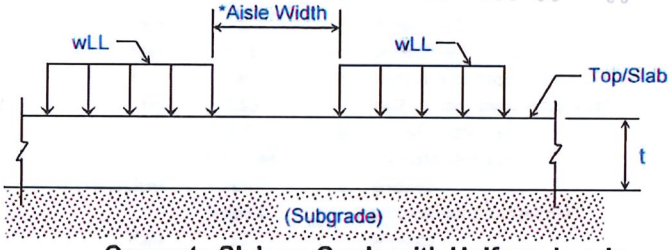
CONCRETE SLAB ON GRADE ANALYSIS

For Slab Subjected to Stationary Uniformly Distributed Live Loads

Job Name:	Sierra Outback 170	Subject:	Floor Slab Uniform Load Check		
Job Number:		Originator:		Checker:	

Input Data:

Slab Thickness, t =	5.000	in.
Concrete Strength, f 'c =	5000	psi
Subgrade Modulus, k =	70	pci
Factor of Safety, FS =	2.000	
Uniform Live Load, wLL =	100.00	psf



Concrete Slab on Grade with Uniform Loads

*Note: in an unjointed aisleway between uniformly distributed load areas, negative bending moment in slab may be up to twice as great as positive moment in slab beneath loaded area. Allowable uniform load determined below is based on critical aisle width and as a result, there are no restrictions on load layout configuration or uniformity of loading.

Results:

Design Parameters:

Modulus of Rupture, MR =	636.40	psi	MR = 9*SQRT(f 'c)
Allow. Bending Stress, Fb =	318.20	psi	Fb = MR/FS
Modulus of Elasticity, Ec =	4030509		Ec = 57000*SQRT(f 'c)
Poisson's Ratio, μ =	0.15		μ = 0.15 (assumed for concrete)
Radius of Stiffness, Lr =	27.99	in.	Lr = (Ec*t^3/(12*(1-μ^2)*k))^0.25
Critical Aisle Width, Wcr =	5.15	ft.	Wcr = (2.209*Lr)/12

Stationary Uniformly Distributed Live Loads:

wLL(allow) = 764.65 psf

$wLL_{(allow)}$ = 257.876*Fb*SQRT(k*t/Ec)

wLL(allow) >= wLL, O.K.

Reference:

1. "Concrete Floor Slabs on Grade Subjected to Heavy Loads"
   Army Technical Manual TM 5-809-12, Air Force Manual AFM 88-3, Chapter 15 (1987)
2. "Slab Thickness Design for Industrial Concrete Floors on Grade" (IS195.01D)
   by Robert G. Packard (Portland Cement Association, 1976)

Comments:

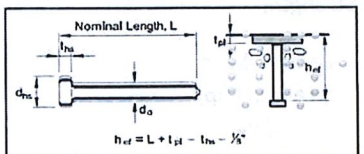
PL-1 Headed Concrete Anchor Capacity (PCI Chapter 6)

(Based on Steel Strength)

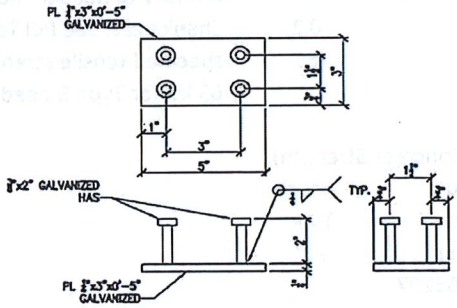
Design Tensile Strength (kip) ϕNs	Design Shear Strength (kip) ϕVs
39	39

Variables	Value	Description	Unit
ϕ	0.75	Strength Reduction Factor 0.65 (shear) 0.75 (tension)	None
n	4	number of headed studs in the assembly	None
Ase	0.2	Shank area. See PCI Table 6.5.1.2	sq. inch
fut	65	specified tensile strength 65 ksi for Type B headed studs	ksi

Table 6.5.1.2 Dimensions of headed studs



Shank Diameter, d_o (in.)	Shank Area, A_{se} (in.^{2})	Head Diameter, d_{hs} (in.)	Head Thickness, t_{hs} (in.)	Bearing Area A_{brg} (in.^{2})
1/4	0.05	1/2	3/16	0.15
3/8	0.11	3/4	3/16	0.33
1/2	0.20	1	5/16	0.59
5/8	0.31	1 1/4	5/16	0.92
3/4	0.44	1 1/4	3/8	0.79
7/8	0.60	1 3/8	3/8	0.88



(Based on Concrete Strength)

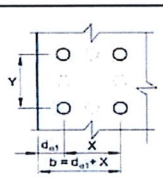
Breakout Strength Ncb (lbs)	Connector Type
6919.766207	PL1

ϕ	0.7	Confinement present? = 0.75 confined, 0.7 not

ϕNcb (lbs)
4844

Variables	Value	Description	Unit
Cbs	174.9001686	Breakout strength coeff. = 3.33λ(f'c/hef)^.5	None
An	50.54443359	This is for 4 stud config. See fig. 6.5.4.1 (d)	sq. inch
Ccrb	1	Cracking factor (1 is most commonly used)	None
ψed,N	0.782758621	Eq. 6.5.4.3	None
λ	1	Coeff. For lightweight concrete	None
f'c	5000	Concrete compressive strength	psi
hef	1.8125	Effective embedment depth	Inch
L	2	Stud Length (nominal)	Inch
ths	0.3125	Thickness of head on stud	Inch
tpl	0.25	Plate Thickness	Inch
de,min	0.75	Least edge distance	Inch
de1	4.75	Edge dist. in fig. 6.5.4.1 (d)	Inch
de2	0.75	Edge dist. in fig. 6.5.4.1 (d)	Inch
X	3	Stud spacing in fig. 6.5.4.1 (d)	Inch
Y	1.5	Stud spacing in fig. 6.5.4.1 (d)	Inch

Figure 6.16.15 Concrete tension strength of welded headed stud assemblies (Cont.)



CASE 2: Near one free edge

Applies to rectangular stud patterns with outside dimensions X and Y. See Figure 6.5.5.1. If stud spacing is ≥ $3h_{ef}$, single stud capacity multiplied by number of studs may govern design.

$\phi N_{cb} = \phi N_{cb}(\psi_{ed,N})$ [ϕN_{cb} from Table A ; $\psi_{ed,N}$ from Table B]

Assumptions for Table A:

- ϕ = 0.7 (no confinement reinforcement – multiply Table A values by 0.75/0.7 when confinement reinforcement is used)
- $C_{cr\sigma}$ = 1.0 (uncracked concrete – multiply Table A values by 0.8 when concrete may be cracked)
- C_{bs} = $3.33\sqrt{(f'_c / h_{ef})}$ (normal weight concrete – multiply Table A values by λ when lightweight concrete is used)
- f'_c = 5000 psi (multiply Table A values by $\sqrt{f'_c / 5000}$ for other concrete strengths)
- h_{ef} = $L_s + t_p - t_{hs}$
- t_p (plate thickness) = 1/4 in. t_{hs} (stud head thickness) = 3/8 in.

Headed Concrete Anchor Capacity (PCI Chapter 6) - PL1

(Based on Steel Strength)

Design Tensile Strength (kip)	Design Shear Strength (kip)
ϕNs	**ϕVs**
39	39

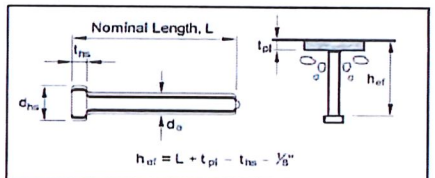
Variables	Value	Description	Unit
ϕ	0.75	Strength Reduction Factor 0.65 (shear) 0.75 (tension)	None
n	4	number of headed studs in the assembly	None
Ase	0.2	Shank area. See PCI Table 6.5.1.2	sq. inch
fut	65	specified tensile strength 65 ksi for Type B headed studs	ksi

(Based on Concrete Strength)

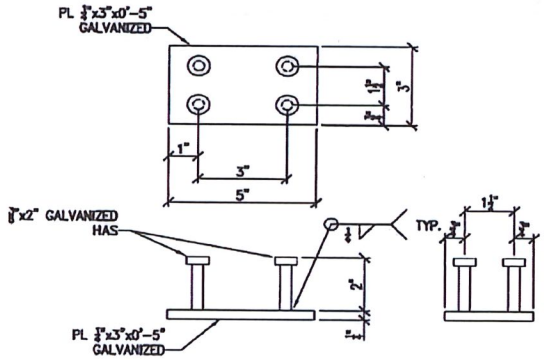
Breakout Strength Ncb (lbs)	Connector Type PL1	
6919.766207		
ϕ	0.7	Confinement present? = 0.75 confined, 0.7 not
ϕNcb (lbs)		
4844		

Variables	Value	Description	Unit
Cbs	174.9001686	Breakout strength coeff. = 3.33λ(f'c/hef)^.5	None
An	50.54443359	This is for 4 stud config. See fig. 6.5.4.1 (d)	sq. inch
Ccrb	1	Cracking factor (1 is most commonly used)	None
ψed,N	0.782758621	Eq. 6.5.4.3	None
λ	1	Coeff. For lightweight concrete	None
f'c	5000	Concrete compressive strength	psi
hef	1.8125	Effective embedment depth	Inch
L	2	Stud Length (nominal)	Inch
ths	0.3125	Thickness of head on stud	Inch
tpl	0.25	Plate Thickness	Inch
de,min	0.75	Least edge distance	Inch
de1	4.75	Edge dist. in fig. 6.5.4.1 (d)	Inch
de2	0.75	Edge dist. in fig. 6.5.4.1 (d)	Inch
X	3	Stud spacing in fig. 6.5.4.1 (d)	Inch
Y	1.5	Stud spacing in fig. 6.5.4.1 (d)	Inch

Table 6.5.1.2 Dimensions of headed studs



Shank Diameter, d_o (in.)	Shank Area, A_{se} (in.²)	Head Diameter, d_{hs} (in.)	Head Thickness, t_{hs} (in.)	Bearing Area, A_{brg} (in.²)
1/4	0.05	1/2	3/16	0.15
3/8	0.11	3/4	9/32	0.33
1/2	0.20	1	3/8	0.59
5/8	0.31	1 1/4	5/16	0.92
3/4	0.44	1 1/2	3/8	0.79
7/8	0.60	1 3/4	3/8	0.88



Headed Concrete Anchor Capacity (PCI Chapter 6) - PL2

(Based on Steel Strength)

Design Tensile Strength (kip)	Design Shear Strength (kip)
ϕNs	**ϕVs**
19.5	19.5

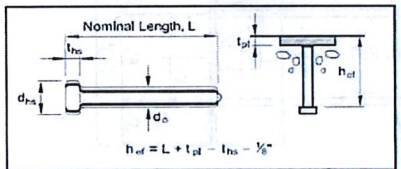
Variables	Value	Description	Unit
ϕ	0.75	Strength Reduction Factor 0.65 (shear) 0.75 (tension)	None
n	2	number of headed studs in the assembly	None
Ase	0.2	Shank area. See PCI Table 6.5.1.2	sq. inch
fut	65	specified tensile strength 65 ksi for Type B headed studs	ksi

(Based on Concrete Strength)

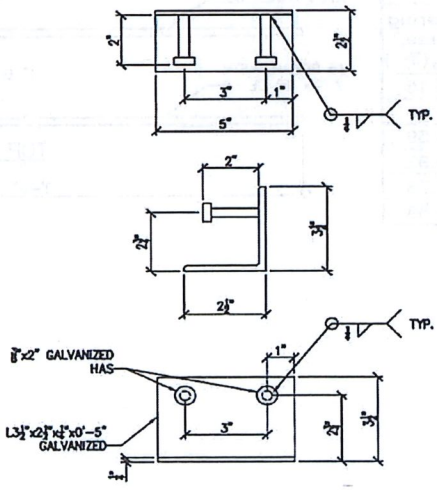
Breakout Strength Ncb (lbs)	Connector Type PL2	
7793.134757		
ϕ	0.7	Confinement present? = 0.75 confined, 0.7 not
ϕNcb (lbs)		
5455		

Variables	Value	Description	Unit
Cbs	174.9001686	Breakout strength coeff. = 3.33λ(f'c/hef)^.5	None
An	56.92382813	This is for 2 stud config. See fig. 6.5.4.1 (c)	sq. inch
Ccrb	1	Cracking factor (1 is most commonly used)	None
ψed,N	0.782758621	Eq. 6.5.4.3	None
λ	1	Coeff. For lightweight concrete	None
f'c	5000	Concrete compressive strength	psi
hef	1.8125	Effective embedment depth	Inch
L	2	Stud Length (nominal)	Inch
ths	0.3125	Thickness of head on stud	Inch
tpl	0.25	Plate Thickness	Inch
de,min	0.75	Least edge distance	Inch
de1	4.75	Edge dist. in fig. 6.5.4.1 (d)	Inch
de2	0.75	Edge dist. in fig. 6.5.4.1 (d)	Inch
X	3	Stud spacing in fig. 6.5.4.1 (d)	Inch
Y	1.5	Stud spacing in fig. 6.5.4.1 (d)	Inch

Table 6.5.1.2 Dimensions of headed studs



Shank Diameter, d_o (in.)	Shank Area, A_{se} (in.^{2})	Head Diameter, d_{hs} (in.)	Head Thickness, t_{hs} (in.)	Bearing Area, A_{brg} (in.^{2})
1/4	0.05	1/2	3/16	0.15
3/8	0.11	3/4	3/32	0.33
1/2	0.20	1	3/16	0.59
5/8	0.31	1 1/4	3/16	0.92
3/4	0.44	1 1/2	3/8	0.79
7/8	0.60	1 3/8	3/8	0.88



Lifting Anchor Design (PCI Chapter 6) LA-1

This spreadsheet calculates the capacity of lifting anchor LA-1

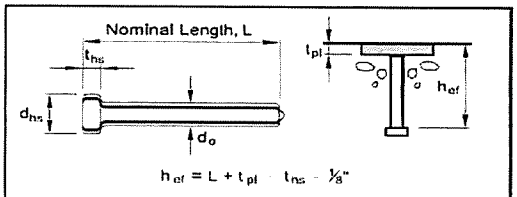
Type of lift Four point

Weight of Building	**51**	kip
Number of Anchors	**8**	Note: An impact factor of 1.2 shall be applied to the service forces
Shear per Anchor	**6.4**	kip

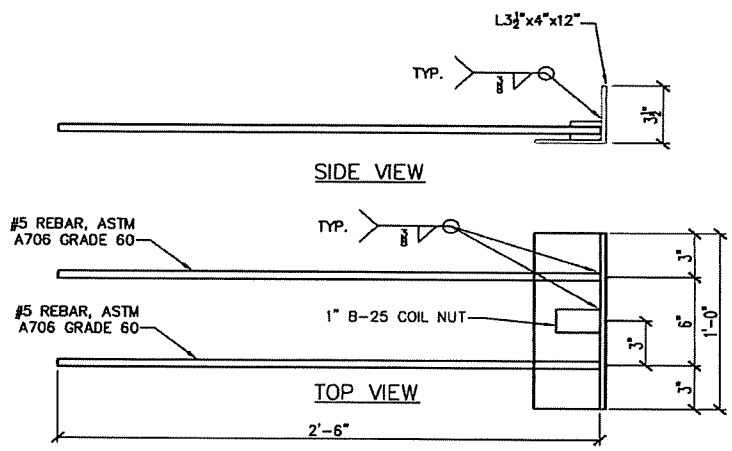
Variables	**Value**	**Description**	Unit	Ref.
ϕ	0.65	Strength Reduction factor	None	6.5.2.1
As	0.31	Tributary width	Inch	
Reinforcing	0.5	Rebar diameter	Inch	
n	2	Number of Studs		
A_{se}	0.2	Area of Each Stud	Sq. Inch	
F_{ut}	65	HSA Yield	ksi	
f_y	60	Yield strength	ksi	
fs	36	Allowable steel stress	ksi	
tp	0.375	Plate Thickness (>= 0.5*HAS dia.)	Inch	6.5.1.1
Concrete Density	145	Unit weight of cured concrete	pcf	

Equation	**Value**	**Failure Mode**			Check
PCI 6.5.2.1	16.9	Shear on Stud	kip	$\phi n A_{se} f_{ut}$	OK
PCI 6.5.2.1	16.9	Tension on Stud	kip	$\phi n A_{se} f_{ut}$	OK
PCI Fig 6.16.4	24.2	Weld on Rebar	kip	$\phi n A_s f_y$	OK
PCI 6.5.2.1	24.2	Tension on Rebar	kip	$\phi n A_s f_y$	OK
$0.6F_{ut}$	78.0	Shear Rupture of Rebar	kip	$0.6F_{ut}$	OK

Table 6.5.1.2 Dimensions of headed studs

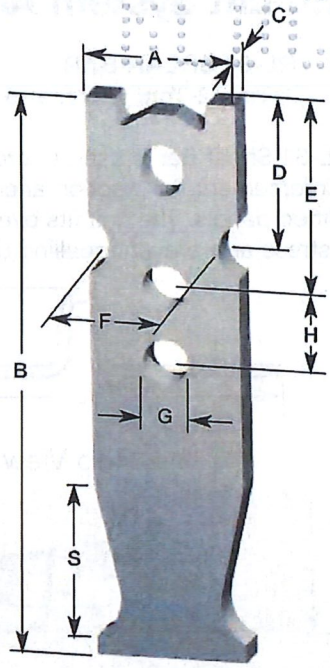


Shank Diameter, d_o (in.)	Shank Area, A_{se} (in.^{2})	Head Diameter, d_{hs} (in.)	Head Thickness, t_{hs} (in.)	Bearing Area, A_{brg} (in.^{2})
1/4	0.05	1/2	3/16	0.15
3/8	0.11	3/4	9/32	0.33
1/2	0.20	1	5/16	0.59
5/8	0.31	1 1/4	5/16	0.92
3/4	0.44	1 1/4	3/8	0.79
7/8	0.60	1 3/8	3/8	0.88



Rapid Lift System Anchors

(6010) RL-3 TECH ERECTION ANCHOR
2-Ton, 4-Ton and 8-Ton



The RL-3 Rapid Lift Tech Erection Anchor is designed for safe edge lifting and rotation of thin-wall precast elements. The anchor is designed with two ears on the head of the anchor to restrict the rotation of the ring clutch. As a result, lateral forces are transmitted directly to the anchor instead of to the concrete to prevent spalling.

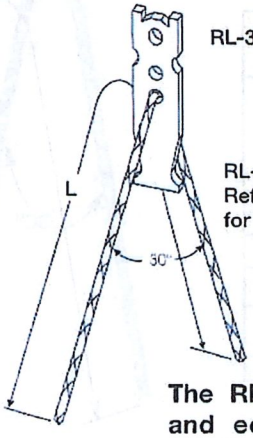
Due to the stress caused by the shear lift on the concrete; it is necessary to add reinforcement in the direction of the lift. The RL-31 Rapid Lift Shear Bar is designed for this purpose and is available. See sketch below. See additional Shear Bar information on page 38.

Anchor dimensions are shown in the table below. Safe working loads and other pertinent information is displayed in the table on the next page. The RL-3 Tech Erection Anchor is available in plain or hot dip galvanize finish.

See Ring Clutches on page 46, 47, and 48, the Recessing Members on 49, 50 and 53, and the Tension Bars on page 38.

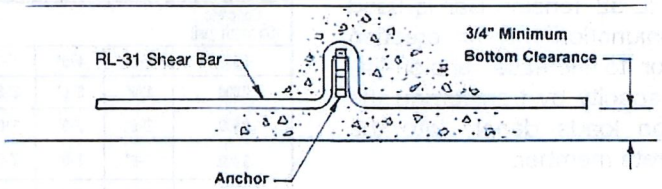
RL-3 RAPID LIFT TECH ERECTION ANCHOR DIMENSIONS

Ring Clutch System	Clutch I.D.	Item Number	A	B	C	D	E	F	G	H	S	Ultimate Mechanical Load (lbs)	Weight Each Piece (lbs)
2T/2.5T	2.5T	79527	2"	8"	3/8"	1 13/16"	2 1/4"	1 3/8"	9/16"	1 1/8"	2"	16,000	1.59
4T/5T	5T	79548	2 1/2"	9 1/2"	5/8"	2 1/2"	3 3/16"	1 13/16"	3/4"	1 1/4"	2 5/8"	32,000	4.21
8T/10T	10T	79589	3 3/4"	12 1/2"	3/4"	3 1/8"	4"	2 7/16"	1"	1 3/4"	3 5/8"	64,000	9.17



The RL-32 Tension Bar provides simple and economical reinforcement for the erection anchor during tension lifts.

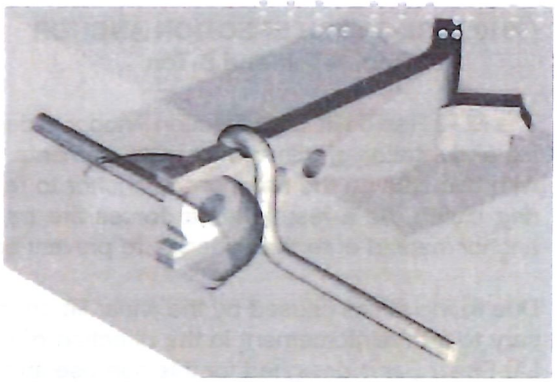
The RL-31 Shear Bar (.375 ø) provides additional shear strength to prevent spalling.

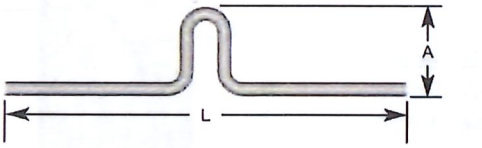


Rapid Lift System Anchors

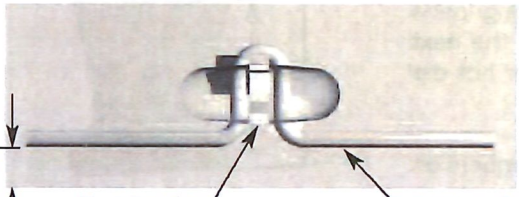
(6200) RL-31 SHEAR BAR
1-Ton, 2-Ton, 4-Ton and 8-Ton

The RL-31 Shear Bar is used to provide simple and economical reinforcement for erection anchors during the rotation of edge-lifted panels. The unit fits over the anchor to spread the shear stress and prevent spalling of the concrete.





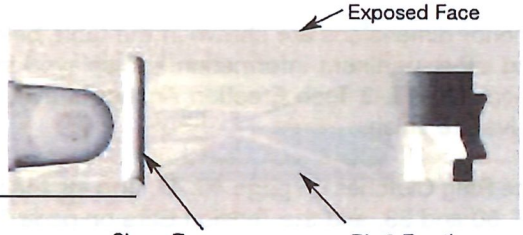
Top View



3/4" Clear Minimum Bottom Face

RL-6 Erection Anchor

RL-31Shear Bar



Exposed Face

Equal

Equal

Shear Bar

3/4" Clear Min. Bottom Face

RL-6 Erection Anchor

RL-31 SHEAR BAR DATA						
Ring Clutch System	Clutch I.D.	Item Number	A	L	Minimum Panel Thickness	Weight (lbs)
1T/1.25T	1.25T	79139	1 5/8"	13"	3"	0.44
2T/2.5T	2.5T	79140	2 1/2"	14-3/4"	4"	0.98
4T/5T	5T	79141	3 5/16"	14-3/4"	5 1/2"	1.07
8T/10T	10T	79142	4 15/16"	14-3/4"	7 1/2"	1.23

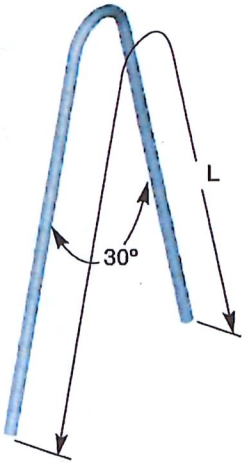
To Order, Specify: quantity, name and item number.

(6205) RL-32 TENSION BAR
2-Ton, 4-Ton, 8-Ton and 22-Ton

The RL-32 Tension Bar is used in conjunction with an erection anchor to increase tension-lifting capacity by transferring the tension loads deeply into the concrete member.

To Order, Specify: quantity, name, rebar size (inch) and length "L."

RL-32 TENSION BAR for STANDARD and HIGH CAPACITY								
Load Group		2-Ton	2.5-Ton	4-Ton	5-Ton	8-Ton	10-Ton	22-Ton
Rebar Size	Inch	#3	#4	#4	#5	#6	#7	#9
	Metric	#10	#13	#13	#16	#19	#22	#29
Concrete Strength (psi)		"L" Dimension						
1,500		3'-0"	4'-0"	4'-0"	4'-5"	6'-0"	7'-6"	12'-0"
2,000		2'-9"	3'-6"	3'-6"	3'-10"	5'-6"	6'-7"	9'-6"
2,500		2'-8"	3'-0"	3'-0"	3'-6"	5'-0"	6'-0"	9'-3"
3,000		2'-6"	3'-0"	3'-0"	3'-3"	4'-6"	5'-6"	9'-0"
4,000		2'-0"	2'-6"	2'-6"	3'-0"	3'-7"	4'-10"	9'-0"
5,000		2'-0"	2'-2"	2'-2"	2'-6"	3'-3"	4'-5"	9'-0"



Rapid Lift System Anchors

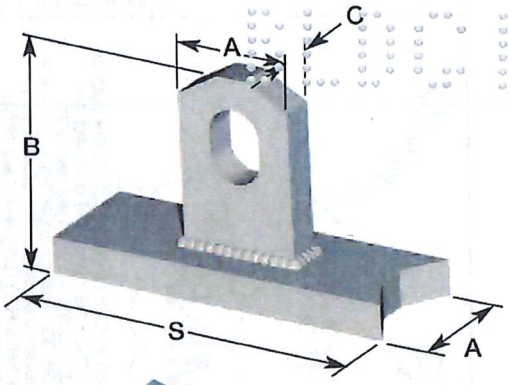
(6060) RL-24 PLATE ANCHOR
2-Ton, 4-Ton and 8-Ton

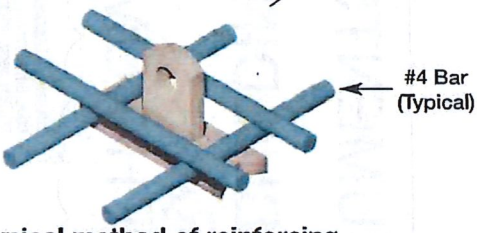
The RL-24 Plate Anchor is designed with a plate welded to the bottom to provide high pullout strength with a low profile. This design makes the anchor ideal for face and back lifts of thin-wall units and stripping, handling and erection applications. The Plate Anchor is available in the sizes shown in the table and in plain or hot dip galvanize finish.

See Ring clutches on page 46, 47, and 48, and the Recessing Members on page 49, 50 and 53.

Reinforcing Recommendation:
Crisscross the lower plate of the anchor with four (4) 18" long #4 rebar as shown in the sketch.

NOTE: A minimum 3/4" concrete cover below the anchor is required to achieve posted working loads.





Typical method of reinforcing Plate Anchors

NOTE: The Plate Anchor has allowable face shear loads that are equal to or greater than unreinforced face tension loads for anchors located in a panel or concrete unit at a distance of at least 2B+A from the edges.

RL-24 RAPID LIFT PLATE ANCHOR DATA

Ring Clutch System	Clutch I.D.	Item Number	A	B	C	S	Allowable Unreinforced Tension Load 4:1 SF (lbs)	Allowable Reinforced Tension Load 4:1 SF (lbs)	Ultimate Mechanical Load Tension (lbs)	Weight Per Piece (lbs)
2T/2.5T	2.5T	79128	1 1/4"	2 1/4"	3/8"	3 3/4"	952	4,000	16,000	0.71
4T/5T	5T	45846[3]	1 1/2"	3"	1/2"	3"	3,574	8,000	32,000	1.21
4T/5T	5T	45847[3]	1 1/2"	3 1/2"	1/2"	3"	4,700	8,000	32,000	1.31
4T/5T	5T	79044	1 1/2"	4 3/8"	5/8"	3 7/8"	4,732	8,000	32,000	1.91
8T/10T	10T	79054	2 1/2"	6 1/4"	3/4"	5"	6,350	12,000	64,000	4.29
8T/10T	10T	79043	–	7 1/8"	Discontinued: See Plate Anchor Item 79042					
8T/10T	10T	79042	2 1/2"	7 1/8"	3/4"	5"	10,000	16,000	64,000	5.55

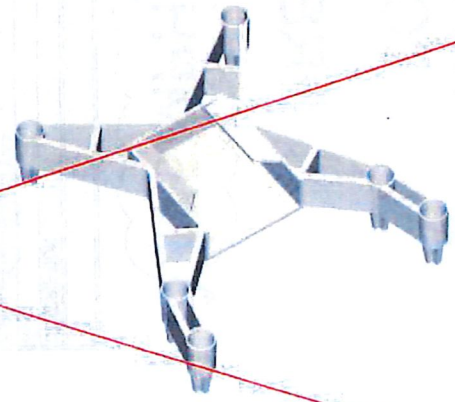
1) Table is based on dead load only, 150 PCF and a standard concrete compressive strength of 3,500 psi and a minimum edge distance of (2B+A)/2.
2) Tension values shown are based on 3,000 psi standard weight concrete, a minimum edge distance of 10" and #4 rebar cut to 18" lengths reinforcing the anchor as shown in the sketch.
3) Available with plate anchor base.

To Order, Specify: quantity, name, item number and finish.

(6190) RL-60 PLATE ANCHOR BASE
4-Ton

The RL-60 Plate Anchor Base s a plastic base designed for use with specific RL-24 Plate Anchor 4-Ton units (item numbers 45846 and 45847) to hold and position the anchors in face lift applications.

To Order, Specify: quantity, name and item number.



CONSTRUCTION DOCUMENTS
for
PRECAST RESTROOM BUILDING
OUTBACK SIERRA
175 MPH ULTIMATE & 136 MPH NOMINAL

80 w/ back

INDEX

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NOTES:

1. BUILDING IS DESIGNED TO BE IN COMPLIANCE WITH FLORIDA BUILDING CODE 5th EDITION (2014). WIND DESIGN IS IN COMPLIANCE WITH SECTION 1609, ASCE 7-10 FOR Vult = 175 MPH WIND SPEED, ENCLOSED STRUCTURE, RISK CATEGORY II, EXPOSURE C, Gcp = 0.18. FLORIDA FIRE PROTECTION CODE 5th EDITION INCLUDING NFPA 1 (FIRE) & NFPA 101 (LIFE SAFETY) (2012) NFPA70 NEC (2011).
2. THESE PLANS COMPLY WITH FLORIDA ADMINISTRATIVE CODE RULE FAC 61G-20-3 (PRODUCT APPROVAL). RAISED SEAL SET PD PLANS ARE ON FILE IN THE THIRD PARTY AGENCY'S OFFICE (AS DIRECTED BY DBCA).
3. TOTAL BUILDING WEIGHT DOES NOT INCLUDE EQUIPMENT BY OTHERS OR FIXTURE WEIGHT.
4. REQUIRED SEPARATION FROM OTHER STRUCTURES AND/OR PROPERTY LINES SHALL BE DETERMINED BY THE AUTHORITY HAVING JURISDICTION.
5. WINDOWS AND DOORS SHALL BE CERTIFIED WITH COMPLIANCE WITH WIND DESIGN PRESSURES FOR COMPONENTS AND CLADDING.
6. PLAN REVIEW INSPECTION REQUIRED BY CHAPTER 633 F.S. SHALL BE DONE ON SITE BY LOCAL FIRE SAFETY INSPECTOR.
7. BUILDING IS NEITHER HEATED OR COOLED.
8. PRODUCT APPROVAL:
   DOORS: FL4553
   WINDOWS: PER FL REG PE (SEE ATTACHED DETAIL)

GENERAL NOTE:
ALL SITE RELATED WORK SHALL BE PERFORMED BY OTHERS UNDER SEPARATE PERMIT AND IN COMPLIANCE WITH THE AUTHORITY HAVING JURISDICTION (AHJ) INCLUDING, BUT NOT LIMITED TO, FOUNDATION, TIE DOWN, GRADING, SIDEWALKS, ACCESSIBLE ROUTE, BUILDING SETBACKS, UTILITY CONNECTIONS AND IMPACT PROTECTION OF DOORS AND WINDOWS AS MAY BE REQUIRED PER FBCB 1710.5 EXCEPTION 3. (NOT REQUIRED IF BUILDING IS LOCATED IN ZONE WHERE VULT IS LESS THAN 140 MPH).

STRUCTURAL / SUBMITTAL REVIEW
APPROVED
APPROVED WITH CHANGES NOTED
REVISE AND RESUBMIT
SUBMITTAL WAS REVIEWED FOR DESIGN COMPATIBILITY AND GENERAL CONFORMANCE TO CONTRACT DOCUMENTS ONLY. THE CONTRACTOR IS RESPONSIBLE FOR CONFIRMING AND CORRELATING DIMENSIONS AT JOB SITES FOR TOLERANCES, CLEARANCES, QUANTITIES, FABRICATION PROCESSES AND TECHNIQUES OF CONSTRUCTION, COORDINATION OF HIS WORK WITH OTHER TRADES AND FULL COMPLIANCE WITH THE CONTRACT DOCUMENTS.
BY: DATE: 05-18-17
BELLON ARCHITECTURE, INC.
13131 SW 132 ST. SUITE 104, MIAMI, FL 33196

4/26/17

26 Apr 17

REVISIONS

1 DATE: 03/22/2017
CHANGED ALL WIND SPEED REFERENCES FROM 170 TO 175 & RECALCULATED ROOF & WALL PRESSURES

2 DATE: 04/28/2017
ADDED RESTROOM FINISH & ACCESSORIES TO SHEET A2 PLUS GENERAL NOTATIONS & CLARIFICATIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-18 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONCRETE STRENGHT FOR POST TENSIONING: 4,500CUL YARD:
POST TENSION FORCE: 28,000 PSI
CUL YARD: SQ. FT: WEIGHT:

Linden Parchment P.E.
8002 Glitter Court
Orlando, FL 32836
(407) 810-3322 P.E. # 54071

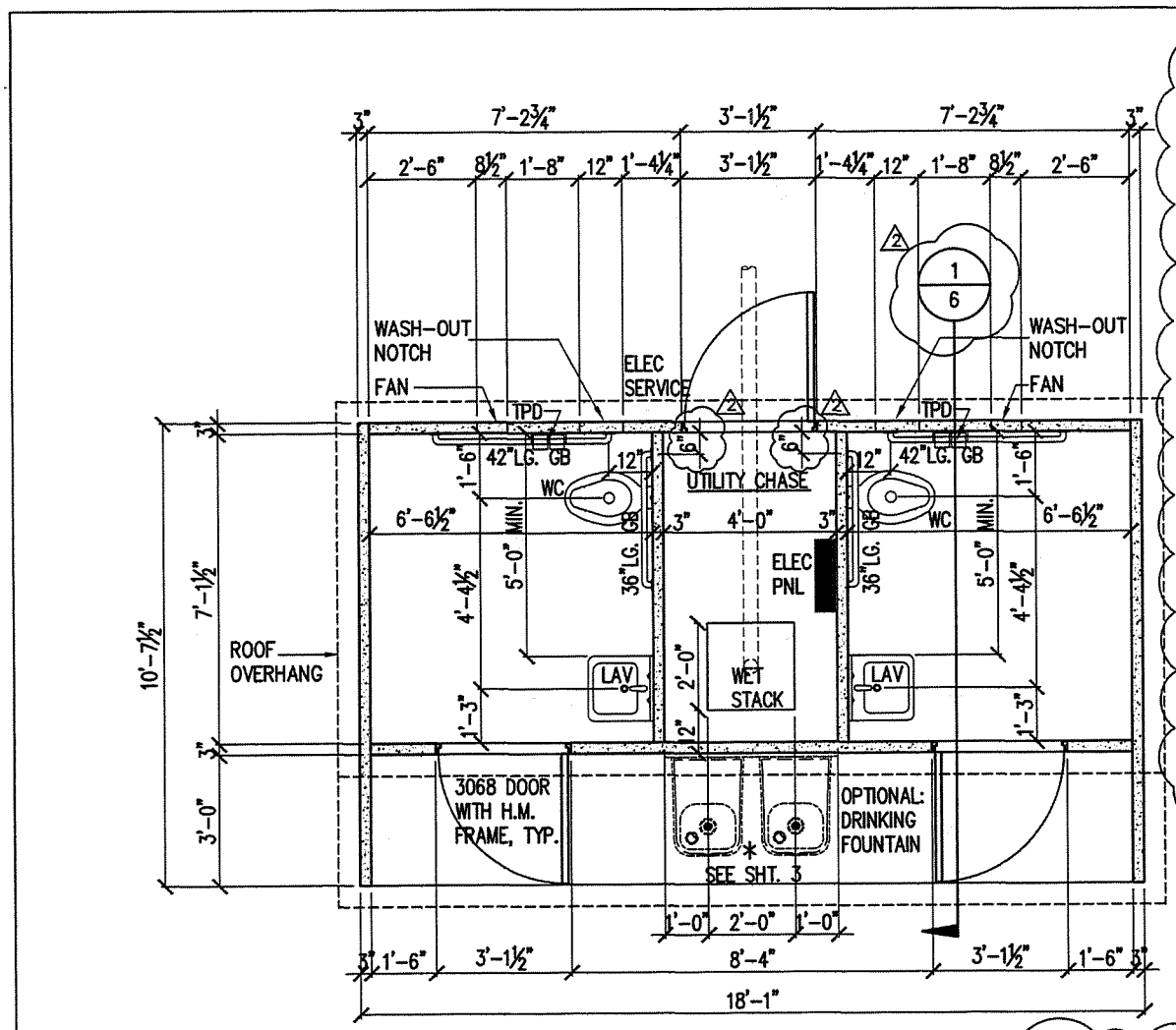
W. DAVID ARNOLD
ARCHITECT
106 PORTLAND STREET
EUSTIS, FL 32726
(352) 357-5807 AR0013558

SCALE: AS SHOWN		DATE: 10/6/16
DESIGN: WDA	DRAWN: JK/LP	CHECKED: WDA

CLIENT: LEESBURG CONCRETE

PROJECT: OUTBACK SIERRA RESTROOM
for
MONTESSORI SCHOOL
17800 OLD CUTLER RD.
PALMETTO BAY, FL. 33157

JOB NO: 16-57 SHEET: 1 OF: 42



FLOOR PLAN

SCALE: 3/8" = 1'-0"

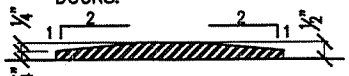
DRINKING FOUNTAIN NOT PROVIDED

INTERIOR FINISH SCHEDULE

1. ALL WALLS SHALL BE LOXON CONCRETE & MASONRY PRIMER/SEALER INT./EXT. LATEX A24W8300 & DURACRAFT EXTERIOR LATEX SATIN C7 SERIES BY SHERWIN WILLIAMS.
2. ALL FLOORS & CEILINGS SHALL BE COLORTOP SOLVENT-BASED SOLID COLOR CONCRETE SEALER BY H&C.
3. ALL DOORS SHALL BE PRO INDUSTRIAL WATERBASED ALKYD URETHANE ENAMEL & MACROPOXY 646 FAST CURE EPOXY BY SHERWIN WILLIAMS.

RESTROOM ACCESSORIES

1. GRAB BARS (GB) AS NOTED.
2. TOILET PAPER DISPENSER (TPD) DOUBLE ROLL TYPE.
3. PROVIDE ADA COMPLIANT ALUM. THRESHOLD FOR ALL DOORS.



TYPICAL THRESHOLD DETAIL

REVISIONS

1 DATE: 03/22/2017
CHANGED ALL WIND SPEED REFERENCES FROM 170 TO 175 & RECALCULATED ROOF & WALL PRESSURES

2 DATE: 04/26/2017
ADDED RESTROOM FINISH & ACCESSORIES TO SHEET A2 PLUS GENERAL NOTATIONS & CLARIFICATIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONCRETE STRENGHT FOR POST TENSIONING: 4,500OCIL YARD:
POST TENSION FORCE: 28,000 PSI

CIL YARD:	SQ. FT.	WEIGHT:

W. DAVID ARNOLD
ARCHITECT
106 PORTLAND STREET
EUSTIS, FL. 32726
(352) 357-5807 AR0013558

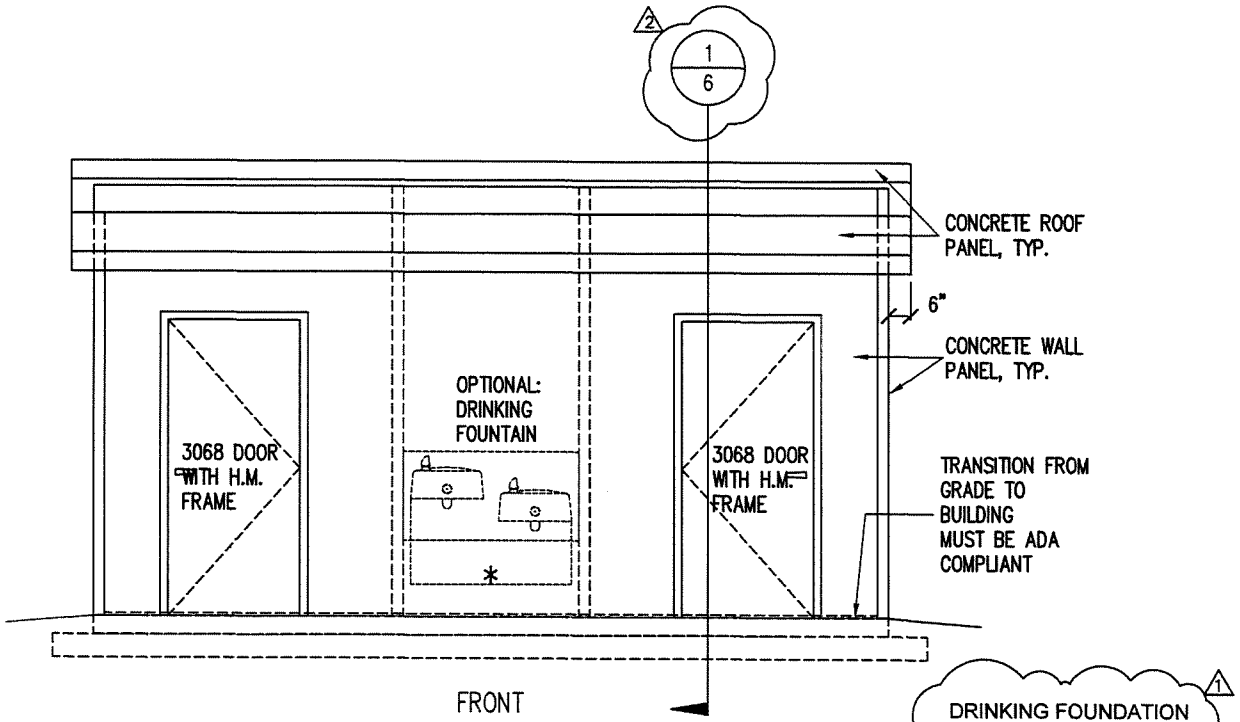
SCALE: AS SHOWN		DATE: 10/6/16
DESIGN: WDA	DRAWN: JK/LP	CHECKED: WDA

CLIENT: LEESBURG CONCRETE

PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL 33157

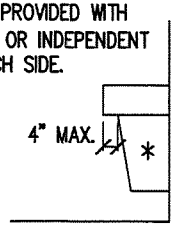
JOB NO: 16-57

SHEET: 2 OF: 42



DRINKING FOUNDATION NOT PROVIDED (1)

* CANE PROTECTION REQUIRED. MAYBE SHROWD PROVIDED WITH HI/LO FOUNTAIN OR INDEPENDENT BARRIER ON EACH SIDE.



EXTERIOR ELEVATION

SCALE: 3/8" = 1'-0"

REVISIONS

1 DATE: 03/22/2017
CHANGED ALL WIND SPEED REFERENCES FROM 170 TO 175 & RECALCULATED ROOF & WALL PRESSURES

2 DATE: 04/26/2017
ADDED RESTROOM FINISH & ACCESSORIES TO SHEET A2 PLUS GENERAL NOTATIONS & CLARIFICATIONS

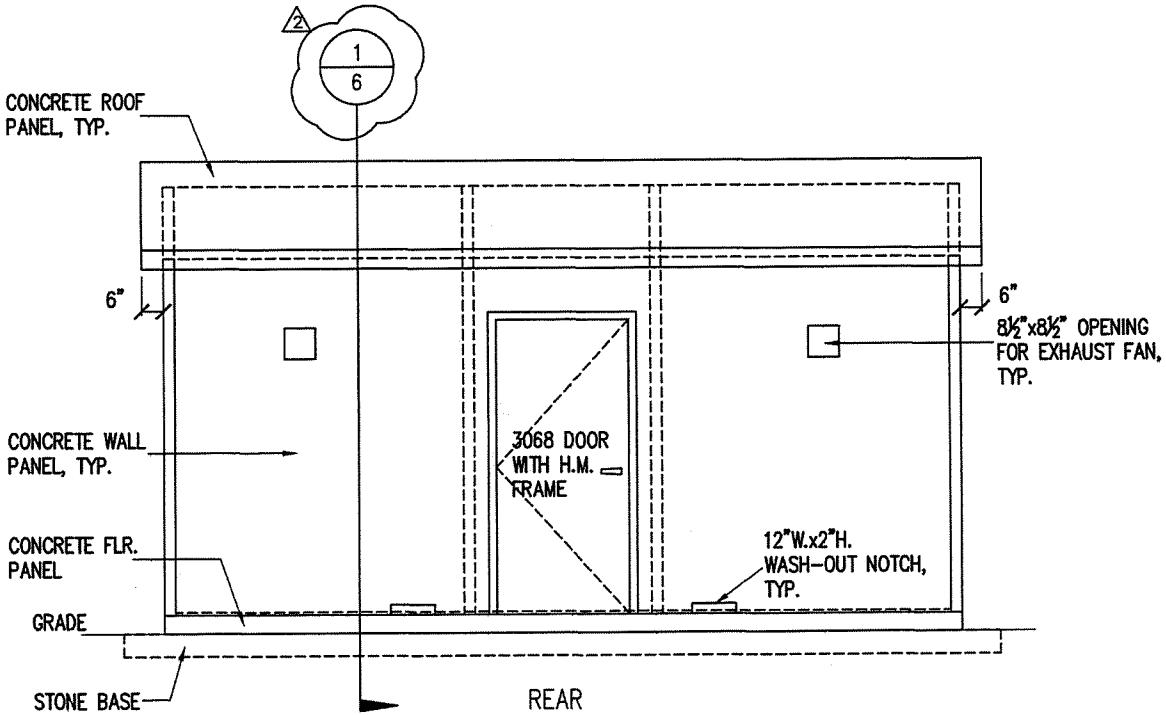
ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONCRETE STRENGHT FOR POST TENSIONING: 4,000/CU. YARD:
POST TENSION FORCE: 28,000 PSI
CU. YARD: | SQ. FT: | WEIGHT:

W. DAVID ARNOLD
ARCHITECT
106 PORTLAND STREET
EUSTIS, FL. 32726
(352) 357-5807 AR0013558

David Arnold
26 April 17

SCALE: AS SHOWN	DATE: 10/6/16
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 3 OF: 42



EXTERIOR ELEVATION

SCALE: 3/8" = 1'-0"

REVISIONS

1 DATE: 03/22/2017
CHANGED ALL WIND SPEED REFERENCES FROM 170 TO 175 & RECALCULATED ROOF & WALL PRESSURES

2 DATE: 04/26/2017
ADDED RESTROOM FINISH & ACCESSORIES TO SHEET A2 PLUS GENERAL NOTATIONS & CLARIFICATIONS

ITEM	QTY	DESCRIPTION
		BURKE HL-2 W/ HL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONCRETE STRENGHT FOR POST TENSIONING: 4,000PSI/CU. YARD:
POST TENSION FORCE: 28,000 PSI

CU. YARD:	SQ. FT:	WEIGHT:

W. DAVID ARNOLD
ARCHITECT
106 PORTLAND STREET
EUSTIS, FL. 32726
(352) 357-5807 AR0013558

David Arnold
26 Apr 17

SCALE:	DATE:
AS SHOWN	10/8/16

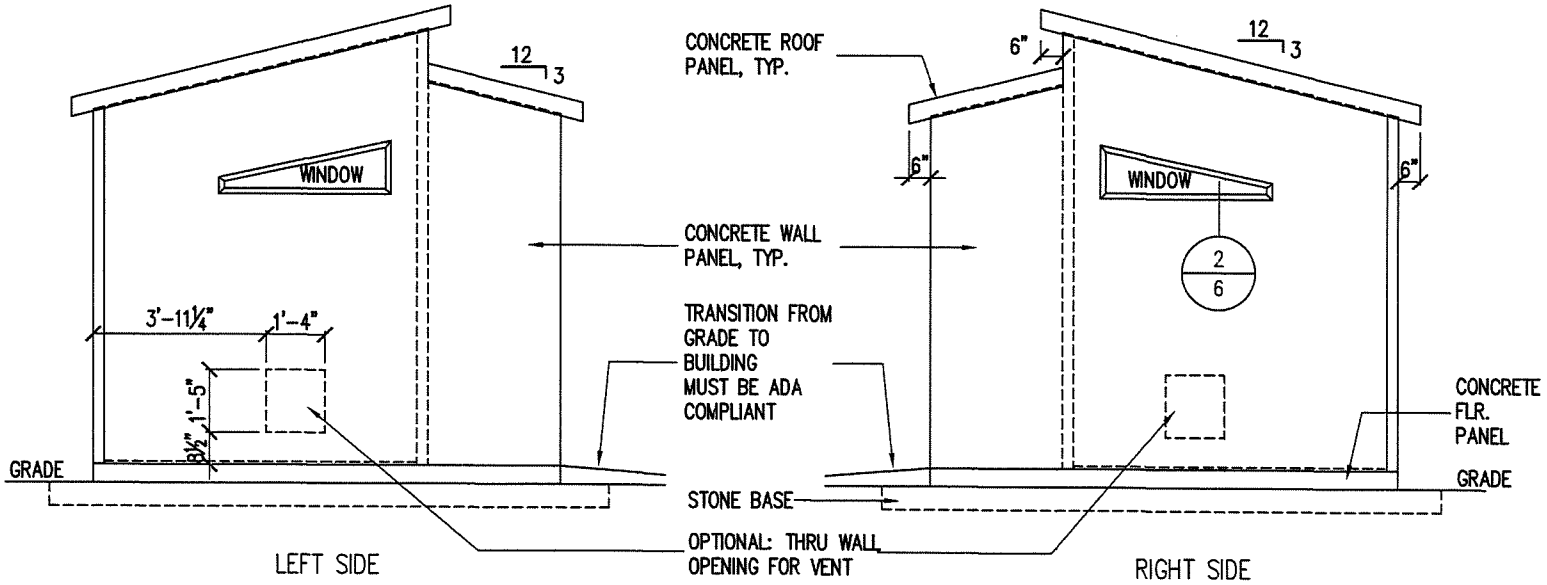
DESIGN:	DRAWN:	CHECKED:
WDA	JK/LP	WDA

CLIENT: LEESBURG CONCRETE

PROJECT: OUTBACK SIERRA RESTROOM
for
MONTESSORI SCHOOL
17800 OLD CUTLER RD.
PALMENTO BAY, FL. 33157

JOB NO: 16-57

SHEET: 4 OF: 42



EXTERIOR ELEVATIONS

SCALE: 3/8" = 1'-0"

REVISIONS

1 DATE: 03/22/2017
CHANGED ALL WIND SPEED REFERENCES FROM 170 TO 175 & RECALCULATED ROOF & WALL PRESSURES

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

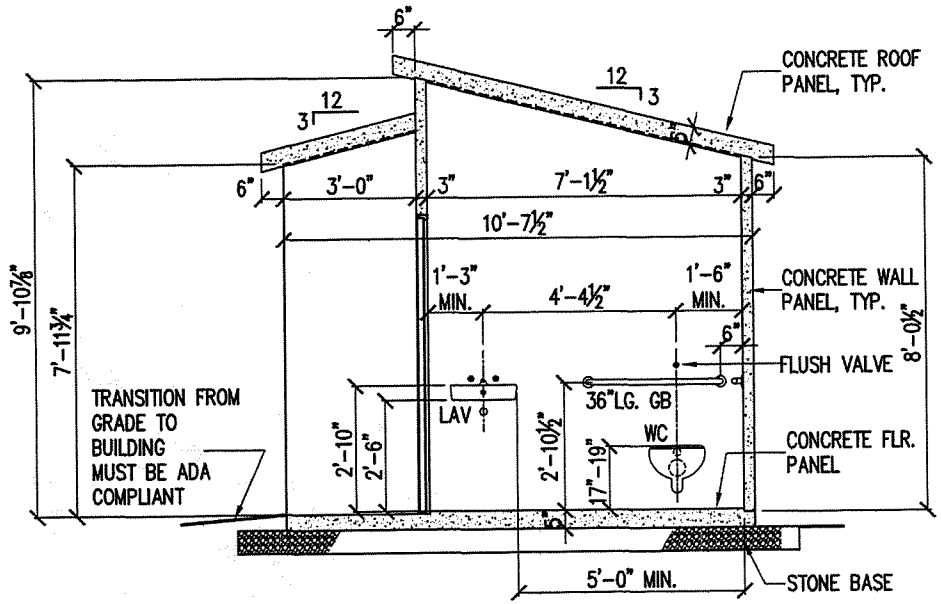
CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONCRETE STRENGHT FOR POST TENSIONING: 4,5000CU. YARD:
POST TENSION FORCE: 30,000 PSI

CU. YARD:	SQ. FT:	WEIGHT:

W. DAVID ARNOLD
ARCHITECT
106 PORTLAND STREET
EUSTIS, FL. 32726
(352) 357-5807 AR0013558

David Arnold
26 Apr 17

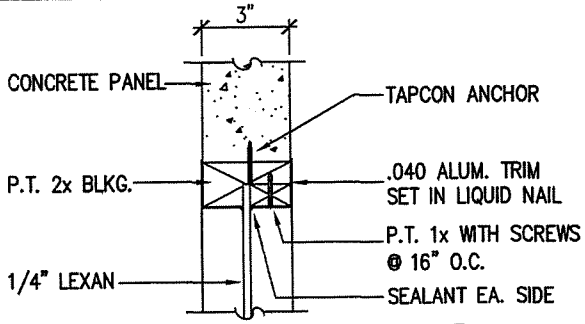
SCALE: AS SHOWN	DATE: 10/6/16
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 5 OF: 42



BUILDING CROSS SECTION

1 / 2 6

SCALE: 3/8" = 1'-0"



WINDOW DETAIL

2 / 5 6

SCALE: 3" = 1'-0"

REVISIONS

1 DATE: 03/22/2017
CHANGED ALL WIND SPEED REFERENCES FROM 170 TO 175 & RECALCULATED ROOF & WALL PRESSURES

ITEM	QTY	DESCRIPTION
		BUNKE BL-2 W/ BL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BUNKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONCRETE STRENGHT FOR POST TENSIONING: 4,500000 LBS. YARD:
POST TENSION FORCE: 28,000 PSI

CU. YARD:	SQ. FT:	WEIGHT:

W. DAVID ARNOLD
ARCHITECT
106 PORTLAND STREET
EUSTIS, FL. 32726
(352) 357-5807 AR0013558

David Arnold
26 Apr 17

SCALE:	DATE:
AS SHOWN	10/6/16

DESIGN:	DRAWN:	CHECKED:
WDA	JK/LP	WDA

CLIENT: LEESBURG CONCRETE

PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL
17800 OLD CUTLER RD.
PALMETTO BAY, FL. 33157

JOB NO: 16-57

SHEET: 6 OF: 42

GENERAL NOTES:

1. ALL DETAILS, SECTIONS AND NOTES SHOWN ON DRAWINGS ARE INTENDED TO BE TYPICAL SHALL APPLY TO SIMILAR SITUATIONS ELSEWHERE UNLESS NOTED OR SHOWN OTHERWISE.
2. THE MANUFACTURER SHALL VERIFY ALL DIMENSIONS AND CONDITION BEFORE START OF WORK.
3. THE DETAILS ON THESE DRAWINGS SHALL BE USED WHEREVER APPLICABLE UNLESS NOTED OR SHOWN OTHERWISE ON THE DRAWINGS. NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS.
4. ALL WORKMANSHIP AND MATERIALS SHALL CONFORM THE THE REQUIREMENTS OF THE FLORIDA BUILDING CODE, (BUILDING, PLUMBING, MECHANICAL) AND THE NATIONAL ELECTRICAL CODE AS APPLICABLE.
5. ALL CONSTRUCTION OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER OF RECORD BEFORE PROCEEDING WITH ANY WORK INVOLVED.
6. ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE 2014 FLORIDA BUILDING CODE AND NEC 2012.
7. MANUFACTURER SHALL BE RESPONSIBLE FOR SAFETY AND PROTECTION IN AND AROUND JOB SITE AND/OR ADJACENT PROPERTIES.
8. OBSERVATION VISITS TO THE SITE BY A FIELD REPRESENTATIVE OF THE ARCHITECT/ENGINEER OF RECORD SHALL NEITHER BE CONSTRUED AS INSPECTION NOR APPROVAL OF CONSTRUCTION UNLESS STATED OTHERWISE.
9. DURING AND AFTER CONSTRUCTION, MANUFACTURER AND/OR OWNER SHALL KEEP LOADS ON STRUCTURE WITHIN THE LIMITS OF DESIGN LOADS.
10. LIABILITY FOR MOVEMENT OF THIS STRUCTURE WILL BE LIMITED TO THE INITIAL SET AND LOCATION OF THIS STRUCTURE BY THE MANUFACTURER. NO OTHER WARRANTY OR LIABILITY IS EXPRESSED OR IMPLIED.
11. THIS STRUCTURE IS NOT DESIGNED TO BE MOVED WITH LIVE LOADS.
12. DURING THE CONSTRUCTION PERIOD THE MANUFACTURER SHALL BE RESPONSIBLE FOR THE SAFETY OF THE BUILDING. THE CONTRACTOR SHALL PROVIDE ADEQUATE SHORING AND BRACING IN ACCORDANCE WITH ALL NATIONAL STATE, AND LOCAL SAFETY ORDINANCES.
13. ALL CONSTRUCTION PROCEDURE SHALL CONFORM TO OSHA STANDARDS. ANY DEVIATION MUST BE APPROVED BY OSHA PRIOR TO.
14. PROVIDE SUPPORT FOR EQUIPMENT AS PER MANUFACTURES RECOMMENDATIONS.
15. ALL ELECTRICAL CONSTRUCTION BY OTHERS, MUST BE PERFORMED BY A LICENSED ELECTRICAL CONTRACTOR.
16. ALL PLUMBING CONSTRUCTION BY OTHERS, MUST BE PERFORMED BY LICENSED PLUMBING CONTRACTOR.
17. BUILDING IS DESIGNED TO BE IN COMPLIANCE WITH FLORIDA BUILDING CODE 2014. WIND DESIGN IS IN COMPLIANCE WITH SECTION 1609, ASCE 7-10 FOR VULT = 175 MPH ULTIMATE & 136 MPH NOMINAL WIND SPEED, ENCLOSED STRUCTURE, RISK CATEGORY II, EXPOSURE C, GCP = 0.18± 1
18. LOADS

ROOF LIVE LOAD = 20 PSF
FLOOR LIVE LOAD = 100 PSF
ROOF DEAD LOAD = 63 PSF
FLOOR DEAD LOAD = 63 PSF
WALL DEAD LOAD = 38 PSF 1

<u>WIND LOADS FOR COMPONENTS AND CLADDING</u>

ROOF ZONES
1. +28.3 PSF, -45 PSF
2. +28.3 PSF, -78 PSF
3. +28.3 PSF, -115.5 PSF

WALL ZONES
WINDOW (LEXAN) +49 PSF, -53 PSF
DOOR +47 PSF, -61 PSF

19. 4x4-W4xW4 WELDED WIRE REINFORCEMENT MAY BE SUBSTITUTED FOR #3 @ 12" O.C.
20. BUILDING OCCUPANCY: OCCUPANCY GROUP IS U (UTILITY AND MISCELLANEOUS) PLUS CONFORMING TO SECTION 428. CONSTRUCTION TYPE IS II-B.
21. FIRE RATING: WALLS: 45 MINUTE
ROOF: 1.5 HR
NO OPENING PROTECTION PROVIDED

REVISIONS

1 DATE: 03/22/2017
CHANGED ALL WIND SPEED REFERENCES FROM 170 TO 175 & RECALCULATED ROOF & WALL PRESSURES

ITEM	QTY	DESCRIPTION
		BURKE BL-2 W/ BL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SHIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONCRETE STRENGHT FOR POST TENSIONING: 4,000CUL YARD:
POST TENSION FORCE: 28,000 PSI

CU. YARD:	SQ. FT:	WEIGHT:

Linden Parchment P.E.
8002 Glitter Court
Orlando, FL 32836
(407) 810-3322 P.E. # 54071

W. DAVID ARNOLD
ARCHITECT
108 PORTLAND STREET
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(352) 357-5807 AR0013558

SCALE:	DATE:
AS SHOWN	10/6/16

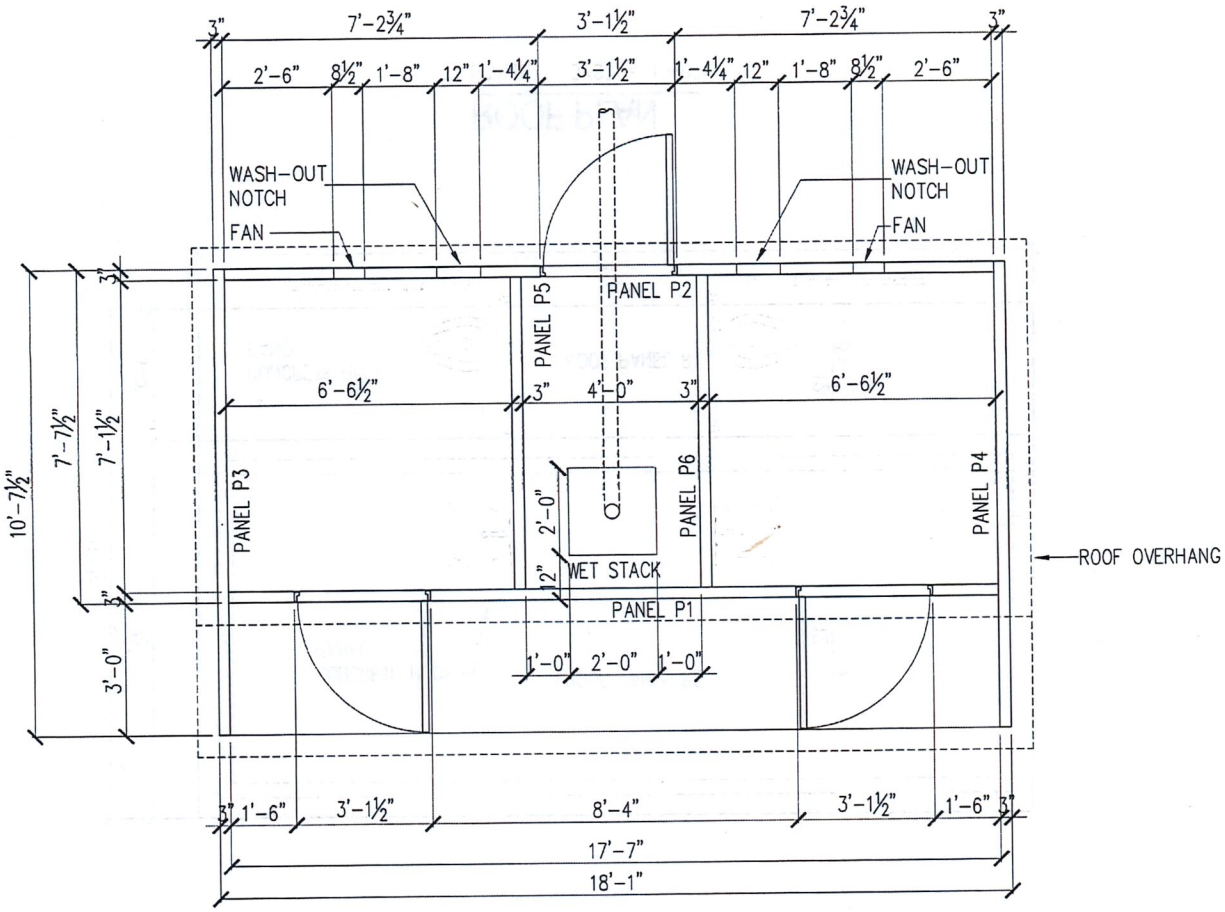
DESIGN:	DRAWN:	CHECKED:
WDA	JK/LP	WDA

CLIENT: LEESBURG CONCRETE

PROJECT: OUTBACK SIERRA RESTROOM
for
MONTESSORI SCHOOL
17800 OLD CUTLER RD.
PALMENTO BAY, FL. 33157

JOB NO: 16-57

SHEET: 7 OF: 42



FLOOR PLAN

SCALE: 3/8" = 1'-0"

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
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MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb
CU. YARD: SQ. FT: WEIGHT:

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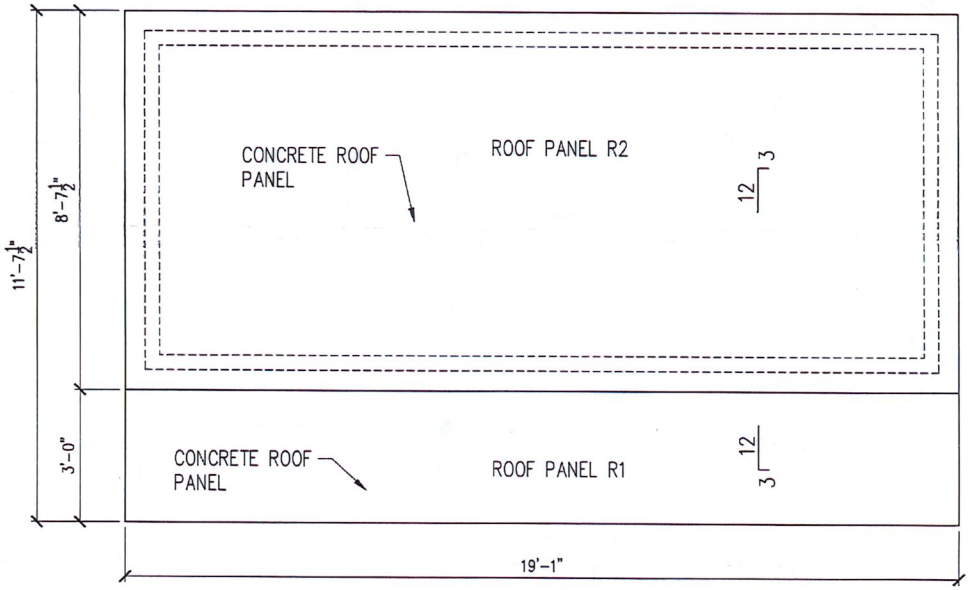
LINDEN C. PARCHMENT
LICENSE
No. 54071
STATE OF FLORIDA
PROFESSIONAL ENGINEER

4/20/17

SCALE: AS SHOWN	DATE: 4/20/2017
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 8 OF: 42



ROOF PLAN

SCALE: 3/8" = 1'-0"

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb

CU. YARD:	SQ. FT:	WEIGHT:

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LINDEN C. PARCHMENT LICENSE No. 54071 STATE OF FLORIDA PROFESSIONAL ENGINEER

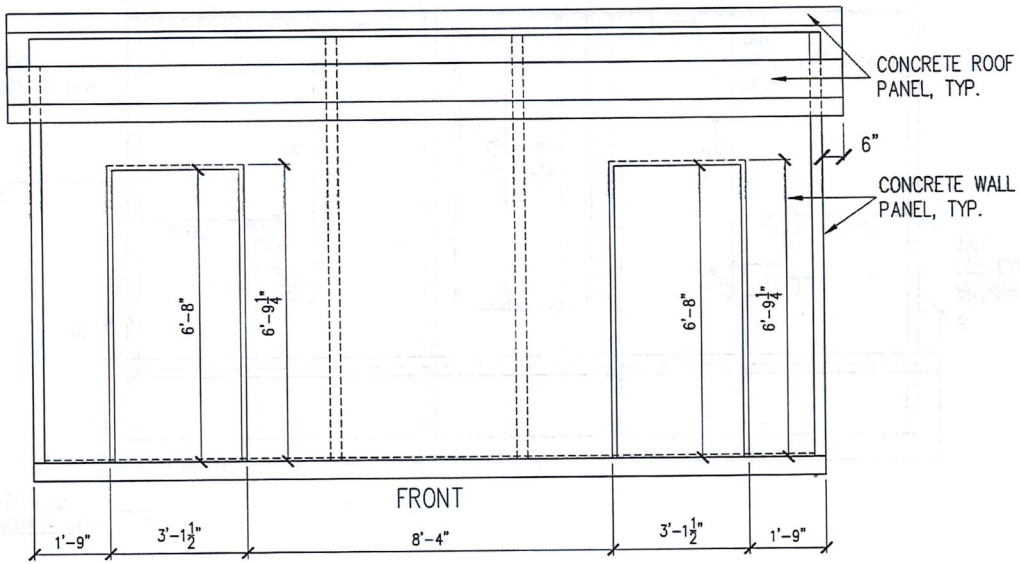
SCALE: AS SHOWN		DATE: 4/20/2017
DESIGN: WDA	DRAWN: JK/LP	CHECKED: WDA

CLIENT: LEESBURG CONCRETE

PROJECT: OUTBACK SIERRA RESTROOM
for
MONTESSORI SCHOOL
17800 OLD CUTLER RD.
PALMENTO BAY, FL. 33157

JOB NO: 16-57

SHEET: 9 OF: 42



EXTERIOR ELEVATION

SCALE: 3/8" = 1'-0"

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb

CU. YARD:	SQ. FT:	WEIGHT:

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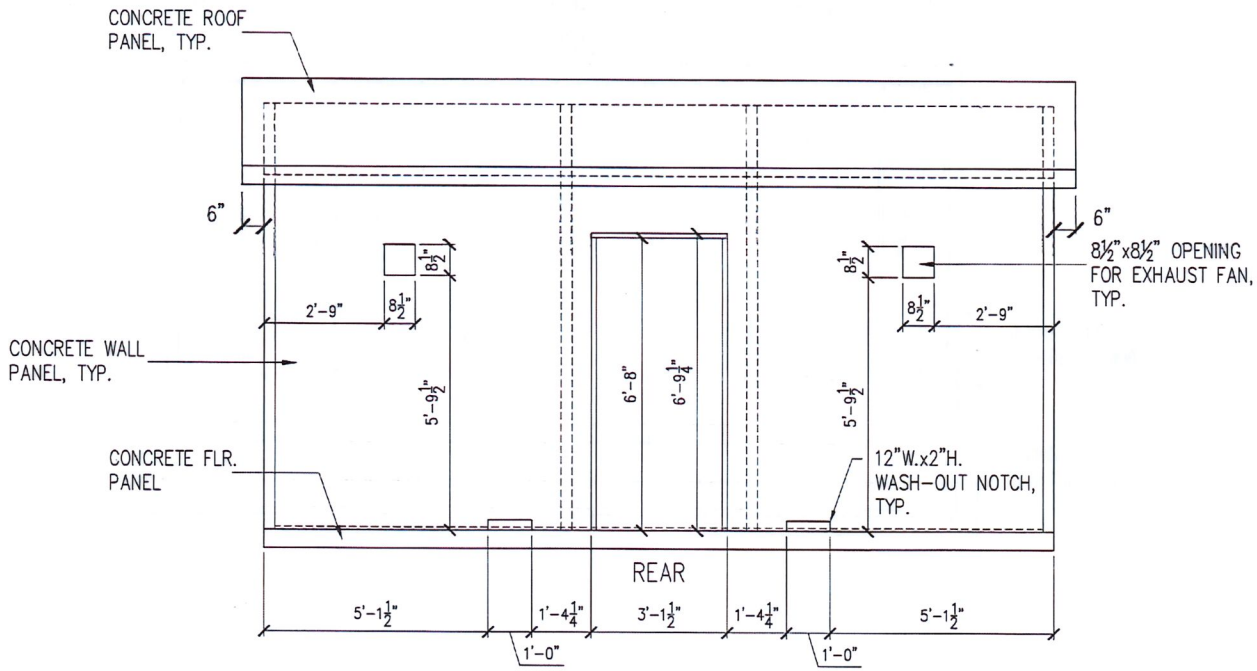
LINDEN C. PARCHMENT LICENSE No. 54071 STATE OF FLORIDA PROFESSIONAL ENGINEER

SCALE:		DATE:
AS SHOWN		4/20/2017
DESIGN: WDA	DRAWN: JK/LP	CHECKED: WDA

CLIENT: LEESBURG CONCRETE

PROJECT: OUTBACK SIERRA RESTROOM
for
MONTESSORI SCHOOL
17800 OLD CUTLER RD.
PALMENTO BAY, FL. 33157

JOB NO: 16-57 SHEET: 10 OF: 42



EXTERIOR ELEVATION

SCALE: 3/8" = 1'-0"

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY

EXT FINISH:

INT FINISH:

MIX DESIGN: 5,000 PSI

RELEASE @ 3000 PSI

CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:

POST TENSION FORCE: 28,000 lb

CU. YARD: | SQ. FT: | WEIGHT:

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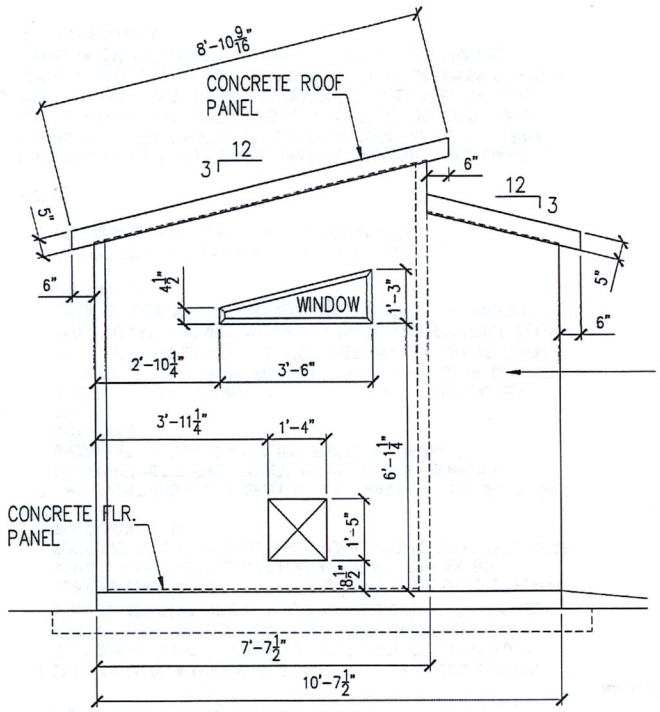
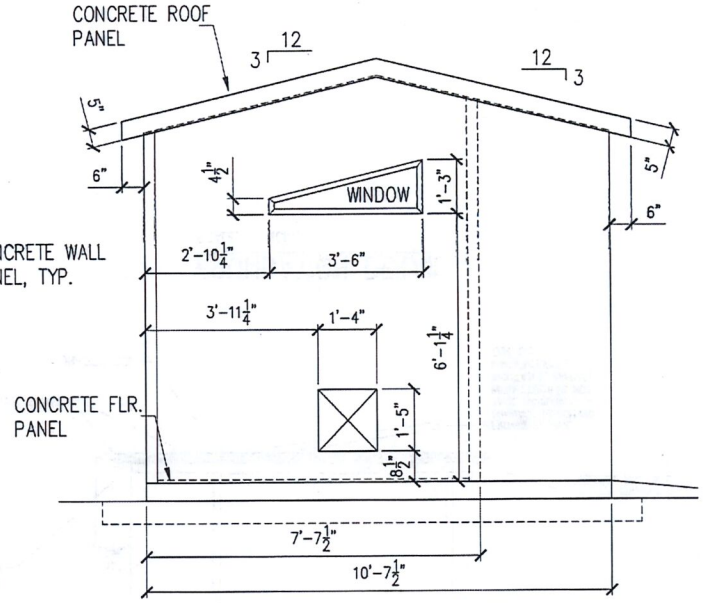
SCALE: AS SHOWN	DATE: 4/20/2017
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 11 OF: 42



LEFT SIDE



LEFT SIDE (ALTERNATE 1)

EXTERIOR ELEVATIONS

SCALE: 3/8" = 1'-0"

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

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EXT FINISH:
INT FINISH:
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RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb
CU. YARD: SQ. FT: WEIGHT:

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LINDEN C. PARCHMENT LICENSE No. 54071 STATE OF FLORIDA PROFESSIONAL ENGINEER

SCALE: AS SHOWN	DATE: 4/20/2017
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 12 OF: 42

FOUNDATION REQUIREMENTS

BUILDING DESIGN
THE FLOATING FOUNDATION OF CRUSHED STONE OR SAND SHOULD BE PLACED ON A WELL-DRAINED AND GRADED AREA, TO PRECLUDE THE RETENTION OF STANDING WATER.

SITE PREPARATION REQUIREMENTS
(MANUFACTURER'S RECOMMENDATION)

A. BUILDING SHALL BEAR FULLY ON A CRUSHED STONE BASE THAT IS AT LEAST TWO FEET LARGER THAN THE LENGTH AND WIDTH OF BUILDING.

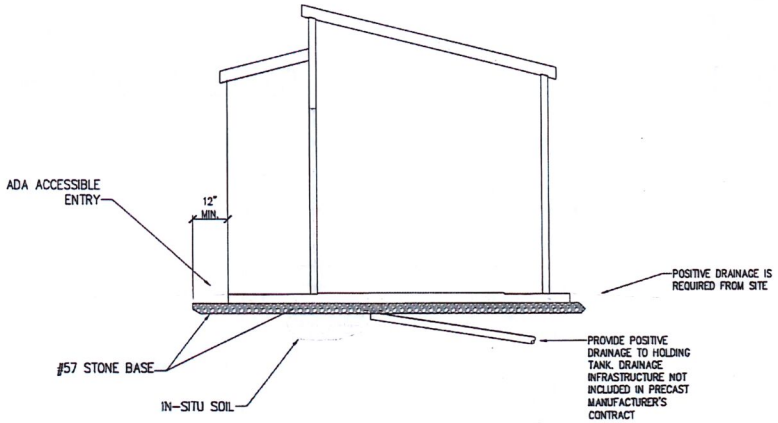
B. STONE SHALL BE A MINIMUM OF 6" THICK OR DOWN TO FIRM SUBGRADE. THE VERTICAL SOIL CAPACITY UNDER STONE SHALL BE COMPACTED TO HAVE MINIMUM BEARING OF 1,500 POUNDS PER SQUARE FOOT. STONE SHALL BE #57 STONE GRADE OR 3/8" MAXIMUM IN SIZE AND MUST BE SCREEDED LEVEL WITHIN ¼" IN BOTH DIRECTIONS. STONE SHALL BE PLACED WITHIN A PERIMETER FORM WITH FLAT AND LEVEL TOP EDGE FOR SCREEDING. FORMING MATERIAL SHALL REMAIN AROUND STONE UNTIL AFTER THE BUILDING IS SET.

C. THE CRUSHED STONE BASE SHALL BE KEPT WITHIN THE CONFINES OF THE SOIL OR PERIMETER FORM. DO NOT ALLOW THE BASE TO BECOME UNCONFINED SO THAT IT MAY WASH, ERODE, OR OTHERWISE BE UNDERMINED.
OR
SUBSTRATE BELOW PAVEMENT OR SLAB MUST HAVE A VERTICAL SOIL CAPACITY OF 1,500 POUNDS PER SQUARE FOOT. PLACE STONE OR SAND TO 1" ABOVE HIGHEST POINT OF AREA WHERE BUILDING WILL BE PLACED AND AT LEAST 1'-0" WIDE ALL AROUND THE BUILDING FOOTPRINT. RETAIN STONE OR SAND WITH A PERIMETER FORM TO PREVENT THE MATERIAL FROM WASHING OUT.

D. PROVIDE POSITIVE DRAINAGE FOR THE FILL AS REQUIRED.
E. ACCESS INTO THE BUILDING SHALL BE ADA COMPLIANT.

ACCESS

A. CONTRACTOR MUST PROVIDE LEVEL UNOBSTRUCTED AREA LARGE ENOUGH FOR A CRANE AND A TRACTOR-TRAILER TO PARK ADJACENT TO THE PAD. CRANE MUST BE ABLE TO PLACE OUTRIGGERS WITHIN 5'-0" OF EDGE OF PAD AND TRUCK AND CRANE MUST BE ABLE TO GET SIDE BY SIDE UNDER THEIR OWN POWER. NO OVERHEAD LINES MAY BE WITHIN 75' RADIUS OF CENTER OF PAD. A MINIMUM OF 24" CLEARANCE IS REQUIRED BETWEEN THIS BUILDING AND ADJACENT BUILDINGS.



FOUNDATION DETAIL

SCALE: NONE

REVISIONS

DRAFT

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb

CU. YARD:	SQ. FT:	WEIGHT:

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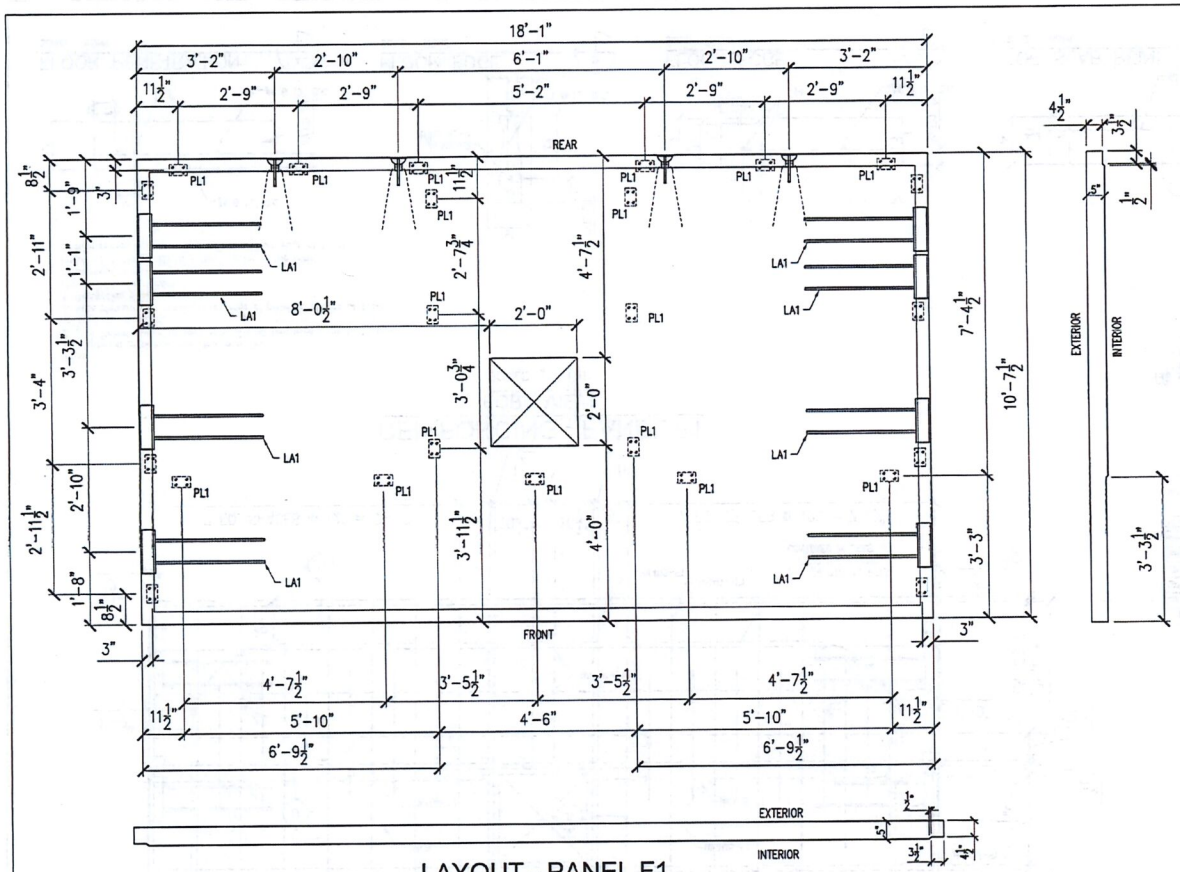
LINDEN C. PARCHMENT LICENSE No. 54071 STATE OF FLORIDA PROFESSIONAL ENGINEER
4/20/17

SCALE: AS SHOWN		DATE: 4/20/2017
DESIGN: WDA	DRAWN: JK/LP	CHECKED: WDA

CLIENT: LEESBURG CONCRETE

PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL
17800 OLD CUTLER RD.
PALMENTO BAY, FL. 33157

JOB NO: 16-57 SHEET: 13 OF: 42



LAYOUT - PANEL F1

FORM VIEW

SCALE: ⅜" = 1'-0"

MARK	DESCRIPTION	QUANTITY
	MEADOW BURKE RL-3 GALV. w/ RL-31 SHEAR BAR AND RL-32 TENSION BAR	4
	PL1	25
	PL2	
	PL3	
	SWIFT LIFT (2 TON)	
	LIFT ANCHOR	8
	CONCRETE VOLUME	3 CU YD
	PANEL WEIGHT	12,000 lb

REVISIONS

NOT FOR BLDG DIV

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY

EXT FINISH:

INT FINISH:

MIX DESIGN: 5,000 PSI

RELEASE @ 3000 PSI

CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:

POST TENSION FORCE: 28,000 lb

CU. YARD: | SQ. FT: | WEIGHT:

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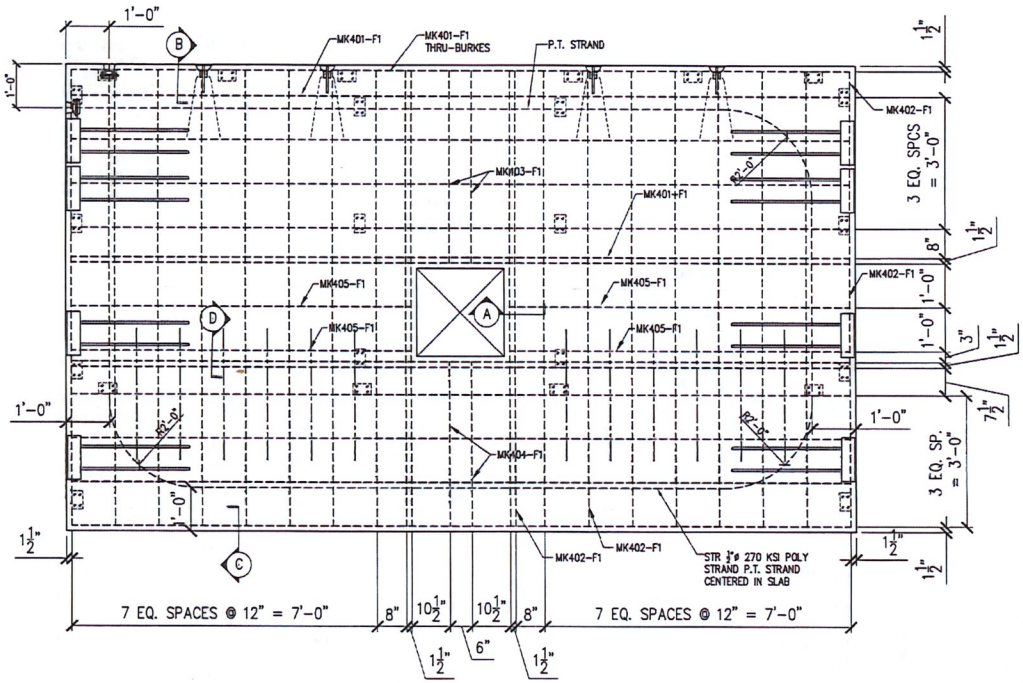
LINDEN C. PARCHMENT
LICENSE
No. 54071
STATE OF FLORIDA
PROFESSIONAL ENGINEER

SCALE: AS SHOWN	DATE: 4/20/2017
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 14 OF: 42

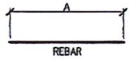


REINFORCING - PANEL F1

FORM VIEW

SCALE: 3/8" = 1'-0"

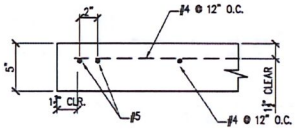
Notes:
1. Orientation of plan is from the form view.
2. Trim plastic sheath at both ends of post tensioned cable to fit tight between anchor plates.
3. Note clearance on drawing from edge of panel to outside of cable.
4. Reinforcing sequence:
Place chairs under mesh
Place mesh and install Burke lift
Install cable. Hold cable at vertical center of the panel
5. Hold reinforcing bars 1 1/2 inch clear from edge of forms.



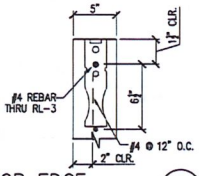
MARK	QUANTITY	LENGTH "A"
MK301-F1	12	3'-0"
MK401-F1	13	17'-10"
MK402-F1	20	10'-4 1/2"
MK403-F1	2	4'-4 1/2"
MK404-F1	2	3'-9"
MK405-F1	4	7'-9 1/2"

MK#02-R1 — PANEL REF.
└ INDICATES REBAR SIZE (4=#4 ETC.)

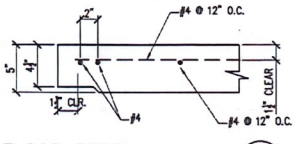
POST TENSION CASTING ANCHOR AND WEDGE FOR 48'-10" LONG 3/8"ø POLY STRAND TENDON. POST TENSION FORCE = 28,000 lbs



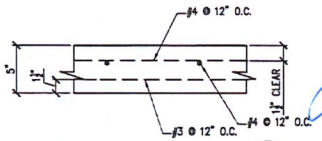
FLOOR PENETRATION (A)
SCALE: NONE



FLOOR EDGE (B)
SCALE: NONE



FLOOR EDGE (C)
SCALE: NONE



TOP SLAB REINF. (D)
SCALE: NONE

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb
CU. YARD: | SQ. FT: | WEIGHT:

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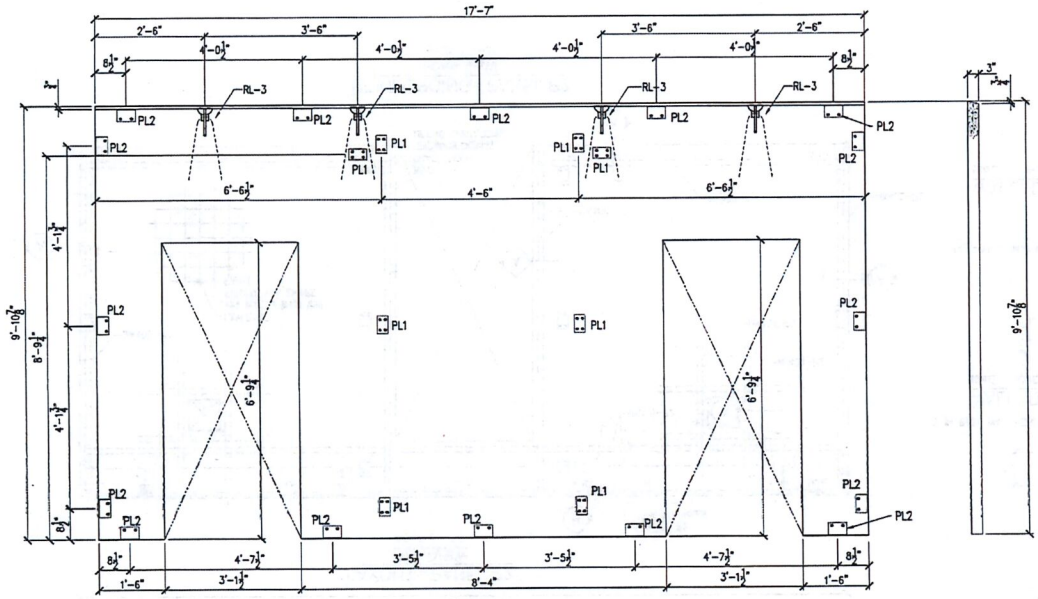
W. DAVID ARNOLD
ARCHITECT
106 PORTLAND STREET
EUSTIS, FL. 32726
(352) 357-5807 AR0013558

LINDEN C. PARCHMENT LICENSE No. 54071 STATE OF FLORIDA PROFESSIONAL ENGINEER

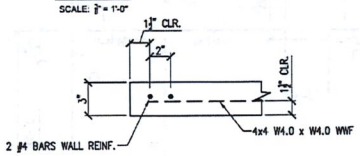
SCALE: AS SHOWN	DATE: 4/20/2017
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 15 OF: 42



LAYOUT - PANEL P1

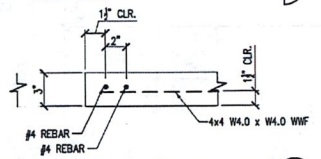
FORM VIEW

SCALE: ¼" = 1'-0"



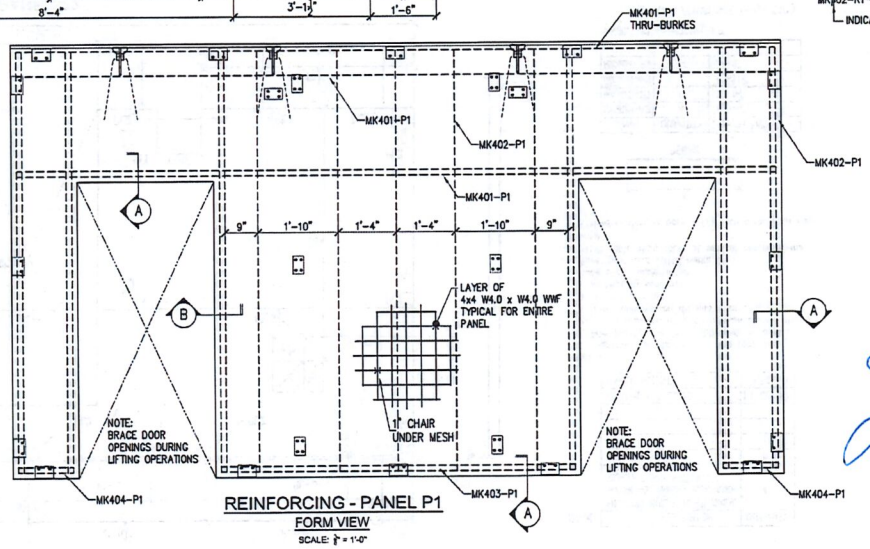
WALL DETAIL

SCALE: NONE



WALL DETAIL AT OPENING

SCALE: NONE



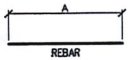
REINFORCING - PANEL P1

FORM VIEW

SCALE: ¼" = 1'-0"

MARK	DESCRIPTION	QUANTITY
	MEADOW BURKE RL-3 GALV. w/ RL-31 SHEAR BAR AND RL-32 TENSION BAR	4
	PL1	8
	PL2	16
	CONCRETE VOLUME	1.2 CU YD
	PANEL WEIGHT	4,920 lb

Notes:
1. Orientation of plan is from the form view.
2. Trim plastic sheath at both ends of post tensioned cable to fit tight between anchor plates.
3. Note clearance on drawing from edge of panel to outside of cable.
4. Reinforcing sequence:
Place chairs under mesh
Place mesh and install Burke lift
Install cable. Hold cable at vertical center of the panel
5. Hold reinforcing bars 1 1/2 inch clear from edge of forms.



REBAR

MARK	QUANTITY	LENGTH "A"
MK401-P1	4	17'-4"
MK402-P1	17	9'-7"
MK403-P1	2	8'-1"
MK404-P1	4	1'-3"

MK#02-R1 — PANEL REF.
└ INDICATES REBAR SIZE (4=#4 ETC.)

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb

CU. YARD:	SQ. FT:	WEIGHT:

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LINDEN C. PARCHMENT
LICENSE
NO. 54071
STATE OF FLORIDA
PROFESSIONAL ENGINEER

SCALE:	DATE:
AS SHOWN	4/20/2017

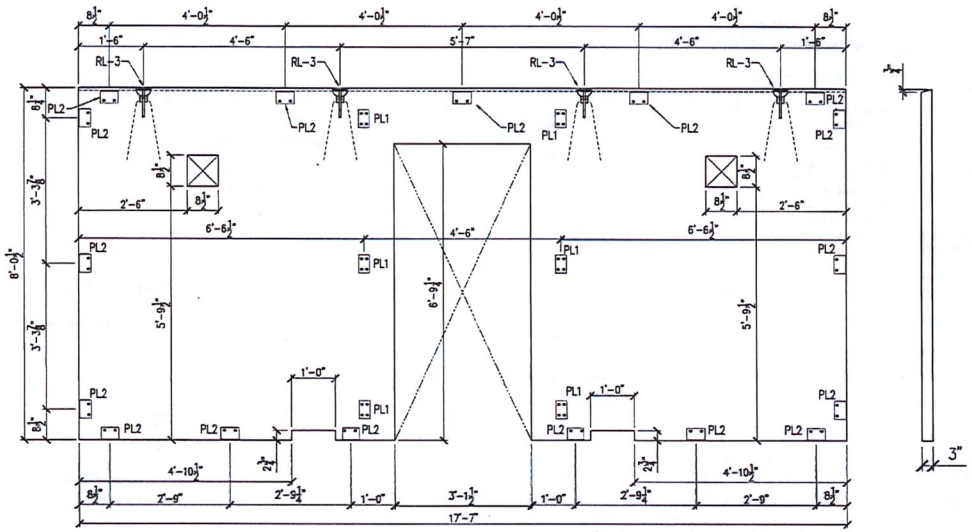
DESIGN:	DRAWN:	CHECKED:
WDA	JK/LP	WDA

CLIENT: LEESBURG CONCRETE

PROJECT: OUTBACK SIERRA RESTROOM
for
MONTESSORI SCHOOL
17800 OLD CUTLER RD.
PALMENTO BAY, FL. 33157

JOB NO: 16-57

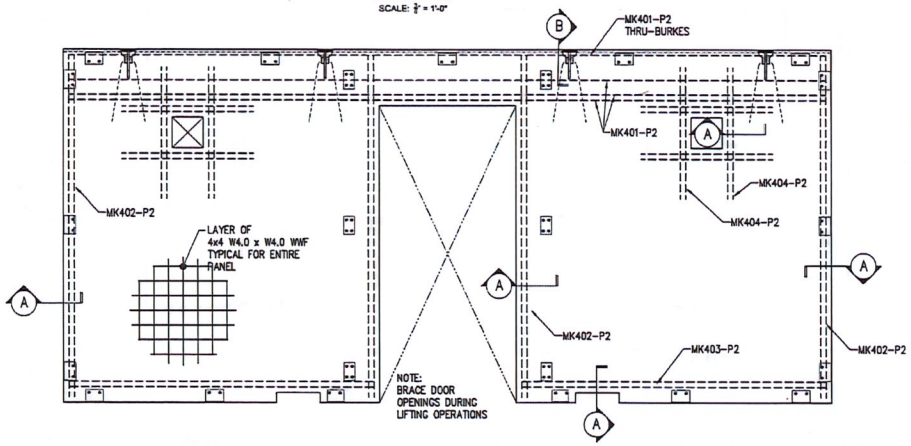
SHEET: 16 OF: 42



LAYOUT - PANEL P2

FORM VIEW

SCALE: 3/4" = 1'-0"



REINFORCING - PANEL P2

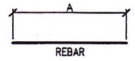
FORM VIEW

SCALE: 3/4" = 1'-0"

MARK

DESCRIPTION	QUANTITY
MEADOW BURKE RL-3 GALV. w/ RL-31 SHEAR BAR AND RL-32 TENSION BAR	4
PL1	6
PL2	17
CONCRETE VOLUME	1.1 CU YD
PANEL WEIGHT	4,500 lb

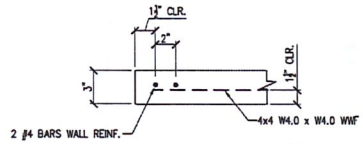
Notes:
1. Orientation of plan is from the form view.
2. Trim plastic sheath at both ends of post tensioned cable to fit tight between anchor plates.
3. Note clearance on drawing from edge of panel to outside of cable.
4. Reinforcing sequence:
Place chairs under mesh
Place mesh and install Burke lift
Install cable. Hold cable at vertical center of the panel
5. Hold reinforcing bars 1 1/2 inch clear from edge of forms.



REBAR

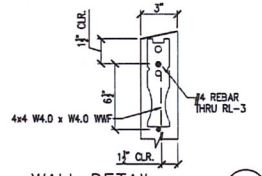
MARK	QUANTITY	LENGTH "A"
MK401-P2	4	17'-4"
MK402-P2	8	7'-9½"
MK403-P2	4	6'-11½"
MK404-P2	16	3'-6"

MK402-R1 — PANEL REF.
└ INDICATES REBAR SIZE (4=#4 ETC.)



WALL DETAIL

SCALE: NONE



WALL DETAIL

SCALE: NONE

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb

CU. YARD:	SQ. FT:	WEIGHT:

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LINDEN C. PARCHMENT LICENSED No. 54071 STATE OF FLORIDA PROFESSIONAL ENGINEER

SCALE:	DATE:
AS SHOWN	4/20/2017

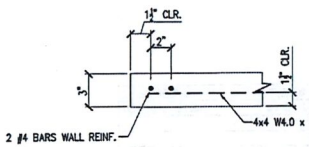
DESIGN:	DRAWN:	CHECKED:
WDA	JK/LP	WDA

CLIENT: LEESBURG CONCRETE

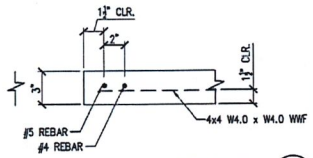
PROJECT: OUTBACK SIERRA RESTROOM
for
MONTESSORI SCHOOL
17800 OLD CUTLER RD.
PALMENTO BAY, FL. 33157

JOB NO: 16-57

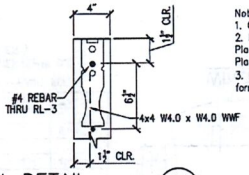
SHEET: 17 OF: 42



WALL DETAIL (A)
SCALE: NONE

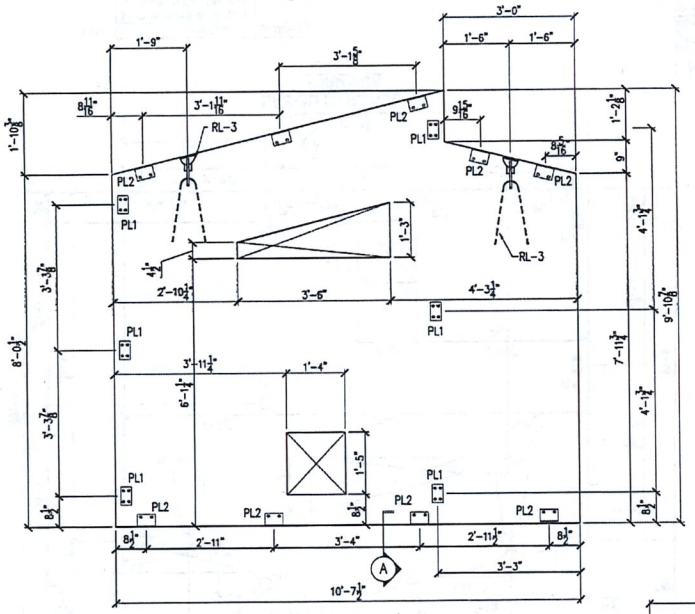


WALL DETAIL AT OPENING (B)
SCALE: NONE

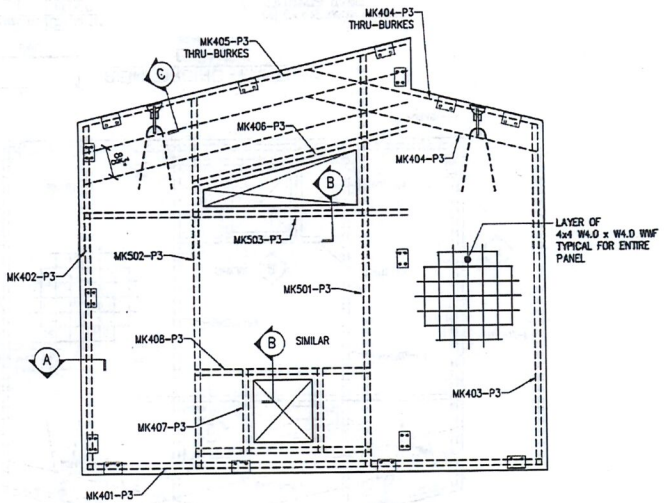


WALL DETAIL (C)
SCALE: NONE

Notes:
1. Orientation of plan is from the form view.
2. Reinforcing sequence:
Place chairs under mesh
Place mesh and install Burke lift
3. Hold reinforcing bars 1 1/2 inch clear from edge of forms.



LAYOUT - PANEL P3
FORM VIEW
SCALE: 1/8" = 1'-0"



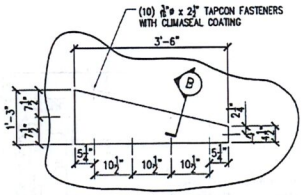
REINFORCING - PANEL P3
FORM VIEW
SCALE: 1/8" = 1'-0"

MARK	DESCRIPTION	QUANTITY
	MEADOW BURKE RL-3 GALV. w/ RL-31 SHEAR BAR AND RL-32 TENSION BAR	2
	PL1	6
	PL2	9
	CONCRETE VOLUME	0.84 CU YD
	PANEL WEIGHT	3,520 lb

A
REBAR

MARK	QUANTITY	LENGTH "A"
MK401-P3	2	10'-4 1/2"
MK402-P3	2	7'-9 1/2"
MK403-P3	2	7'-9"
MK404-P3	2	5'-4"
MK405-P3	3	7'-7"
MK406-P3	2	5'-0"
MK407-P3	4	2'-0"
MK408-P3	4	4'-0"
MK501-P3	2	9'-4 1/2"
MK502-P3	2	8'-5"
MK503-P3	2	7'-4"

MK402-P1 — PANEL REF.
└ INDICATES REBAR SIZE (4=#4 ETC.)



WINDOW DETAIL
SCALE: 3/4" = 1'-0"

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb
CU. YARD: SQ. FT: WEIGHT:

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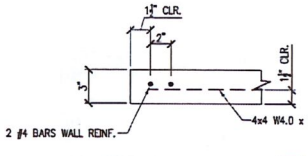
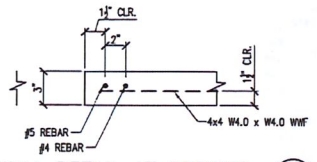
(352) 357-5807 AR0013558

LINDEN C. PARCHMENT
LICENSE
No. 54071
STATE OF FLORIDA
PROFESSIONAL ENGINEER

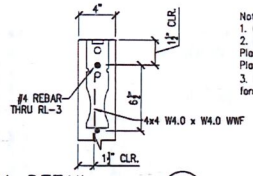
SCALE: AS SHOWN	DATE: 4/20/2017
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 18 OF: 42



WALL DETAIL (A)
SCALE: NONE

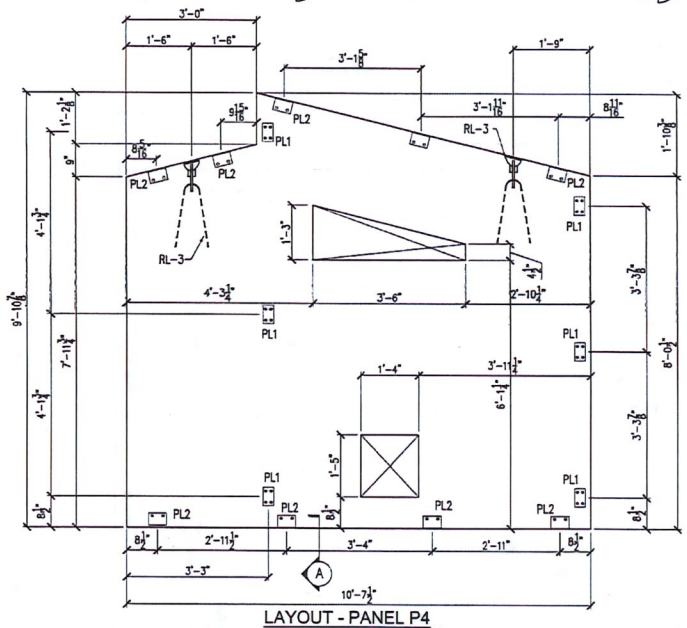


WALL DETAIL AT OPENING (B)
SCALE: NONE

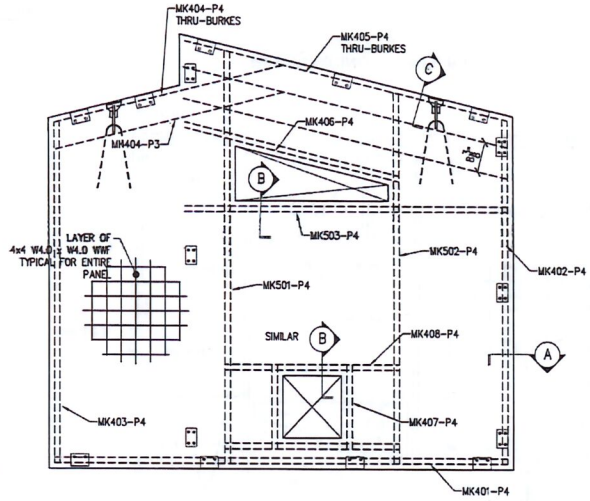


WALL DETAIL (C)
SCALE: NONE

Notes:
1. Orientation of plan is from the form view.
2. Reinforcing sequence:
Place chairs under mesh
Place mesh and install Burke lift
3. Hold reinforcing bars 1 1/2 inch clear from edge of forms.

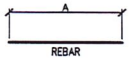


LAYOUT - PANEL P4
FORM VIEW
SCALE: ¼" = 1'-0"



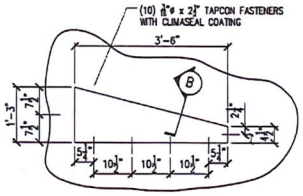
REINFORCING - PANEL P4
FORM VIEW
SCALE: ¼" = 1'-0"

MARK	DESCRIPTION	QUANTITY
	MEADOW BURKE RL-3 GALV. w/ RL-31 SHEAR BAR AND RL-32 TENSION BAR	2
	PL1	6
	PL2	9
	CONCRETE VOLUME	0.84 CU YD
	PANEL WEIGHT	3,520 lb



MARK	QUANTITY	LENGTH "A"
MK401-P4	2	10'-4 ½"
MK402-P4	2	7'-9 ½"
MK403-P4	2	7'-9"
MK404-P4	2	5'-4"
MK405-P4	3	7'-7"
MK406-P4	2	5'-0"
MK407-P4	4	2'-0"
MK408-P4	4	4'-0"
MK501-P4	2	9'-4½"
MK502-P4	2	8'-5"
MK503-P4	2	7'-4"

MK402-P1 — PANEL REF.
└ INDICATES REBAR SIZE (4=#4 ETC.)



WINDOW DETAIL
SCALE: ¾" = 1'-0"

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb
CU. YARD: | SQ. FT: | WEIGHT:

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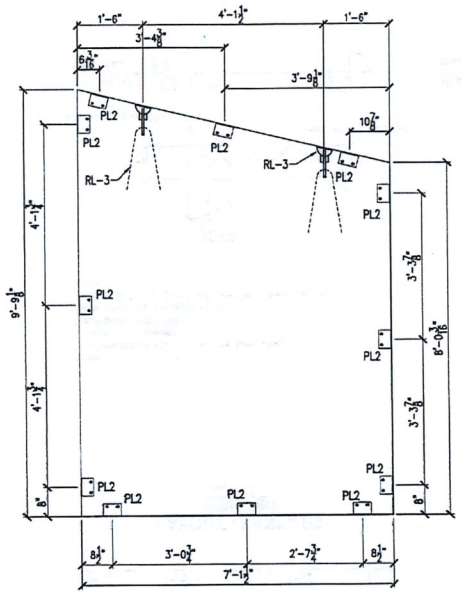
LINDEN C. PARCHMENT
LICENSED
No. 54071
STATE OF FLORIDA
PROFESSIONAL ENGINEER

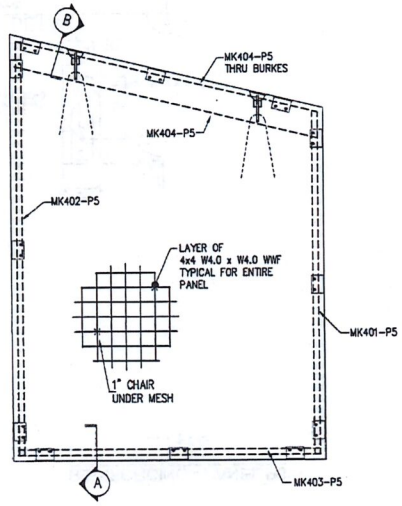
SCALE: AS SHOWN	DATE: 4/20/2017
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 19 OF: 42



LAYOUT - PANEL P5

FORM VIEW

SCALE: ¾" = 1'-0"



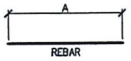
REINFORCING - PANEL P5

FORM VIEW

SCALE: ¾" = 1'-0"

MARK	DESCRIPTION	QUANTITY
	MEADOW BURKE RL-3 GALV. w/ RL-31 SHEAR BAR AND RL-32 TENSION BAR	2
	PL1	
	PL2	12
	CONCRETE VOLUME	0.58 CU YD
	PANEL WEIGHT	2,454 lb

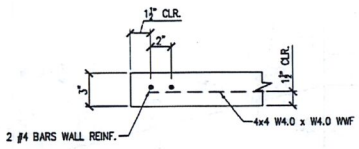
Notes:
1. Orientation of plan is from the form view.
2. Trim plastic sheath at both ends of post tensioned cable to fit light between anchor plates.
3. Note clearance on drawing from edge of panel to outside of cable.
4. Reinforcing sequence:
Place chairs under mesh
Place mesh and install Burke lift
Install cable. Hold cable at vertical center of the panel
5. Hold reinforcing bars 1 1/2 inch clear from edge of forms.



MARK	QUANTITY	LENGTH "A"
MK401-P5	2	7'-9"
MK402-P5	2	9'-5"
MK403-P5	2	6'-10"
MK404-P5	2	7'-0"

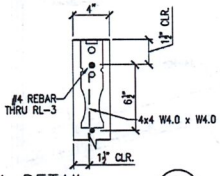
MK#02-R1 — PANEL REF.
└ INDICATES REBAR SIZE (4=#4 ETC.)

Notes:
1. Orientation of plan is from the form view.
2. Reinforcing sequence:
Place chairs under mesh
Place mesh and install Burke lift
3. Hold reinforcing bars 1 1/2 inch clear from edge of forms.



WALL DETAIL A

SCALE: NONE



WALL DETAIL B

SCALE: NONE

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb
CU. YARD: | SQ. FT: | WEIGHT:

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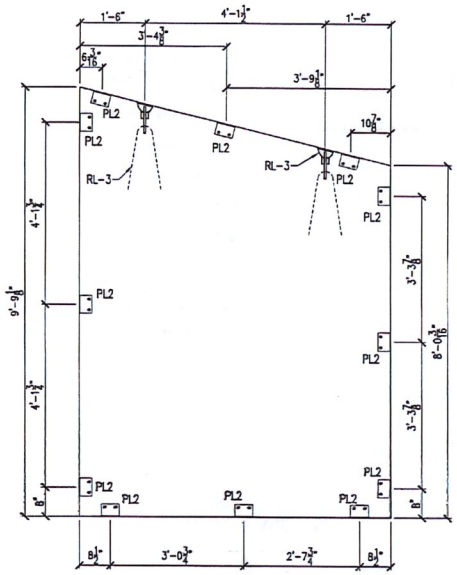
PROFESSIONAL ENGINEER

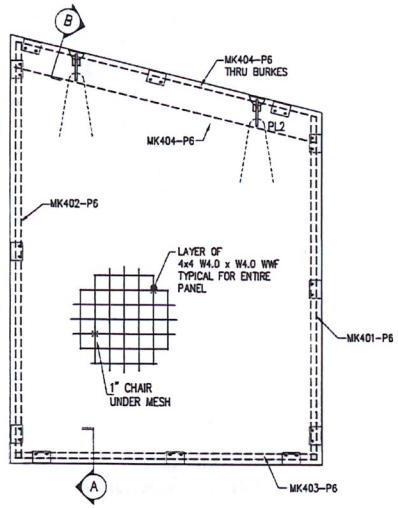
SCALE: AS SHOWN	DATE: 4/20/2017
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL 33157	
JOB NO: 16-57	SHEET: 20 OF: 42



LAYOUT - PANEL P6

FORM VIEW

SCALE: ¾" = 1'-0"



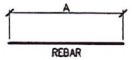
REINFORCING - PANEL P6

FORM VIEW

SCALE: ¾" = 1'-0"

MARK	DESCRIPTION	QUANTITY
	MEADOW BURKE RL-3 GALV. w/ RL-31 SHEAR BAR AND RL-32 TENSION BAR	2
	PL1	
	PL2	12
	CONCRETE VOLUME	0.58 CU YD
	PANEL WEIGHT	2,454 lb

Notes:
1. Orientation of plan is from the form view.
2. Trim plastic sheath at both ends of post tensioned cable to fit tight between anchor plates.
3. Note clearance on drawing from edge of panel to outside of cable.
4. Reinforcing sequence:
Place chairs under mesh
Place mesh and install Burke lift
Install cable. Hold cable at vertical center of the panel
5. Hold reinforcing bars 1 1/2 inch clear from edge of forms.

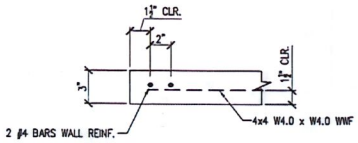


REBAR

MARK	QUANTITY	LENGTH "A"
MK401-P6	2	7'-9"
MK402-P6	2	9'-5"
MK403-P6	2	6'-10"
MK404-P6	2	7'-0"

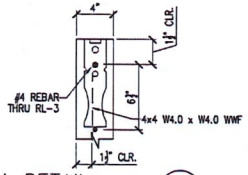
MK402-R1 — PANEL REF.
└ INDICATES REBAR SIZE (4=#4 ETC.)

Notes:
1. Orientation of plan is from the form view.
2. Reinforcing sequence:
Place chairs under mesh
Place mesh and install Burke lift
3. Hold reinforcing bars 1 1/2 inch clear from edge of forms.



WALL DETAIL A

SCALE: NONE



WALL DETAIL B

SCALE: NONE

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb
CU. YARD: | SQ. FT: | WEIGHT:

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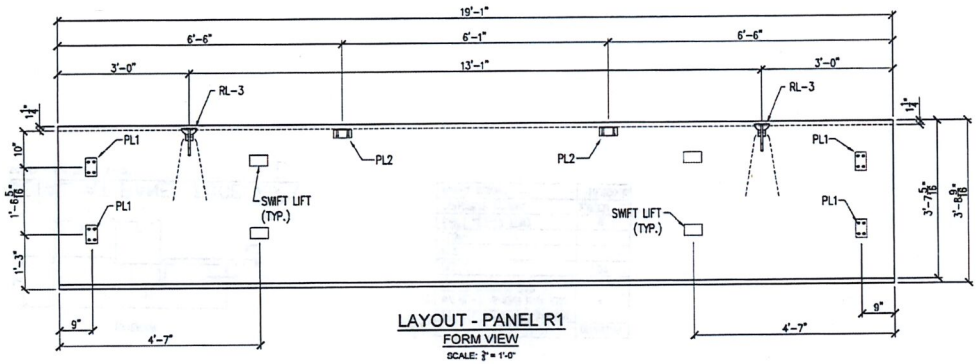
LINDEN S. PARCHMENT LICENSE No. 54071 STATE OF FLORIDA PROFESSIONAL ENGINEER

4/25/17

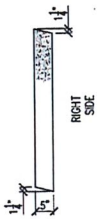
SCALE: AS SHOWN	DATE: 4/20/2017
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 21 OF: 42



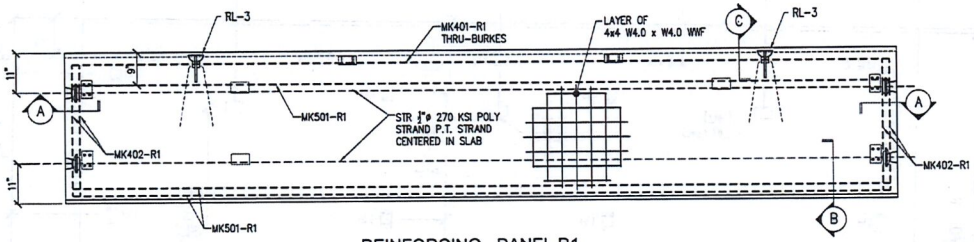
LAYOUT - PANEL R1

FORM VIEW

SCALE: 3/4" = 1'-0"



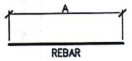
MARK	DESCRIPTION	QUANTITY
	MEADOW BURKE RL-3 GALV. w/ RL-31 SHEAR BAR AND RL-32 TENSION BAR	2
	PL1	4
	PL2	2
	PL3	
	SWIFT LIFT (2 TON)	4
	CONCRETE VOLUME	1.1 CU YD
	PANEL WEIGHT	4,500 lb



REINFORCING - PANEL R1

FORM VIEW

SCALE: 3/4" = 1'-0"

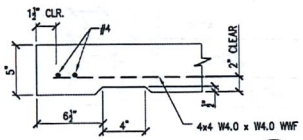


MARK	QUANTITY	LENGTH "A"
MK401-R1	1	18'-10"
MK402-R1	4	3'-5"
MK501-R1	3	18'-10"
STR	2	19'-1

MK#02-R1 —— PANEL REF.
└ INDICATES REBAR SIZE (4=#4 ETC.)

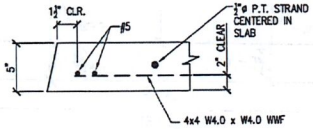
POST TENSION CASTING ANCHOR AND WEDGE FOR 19'-1" LONG 3/8"ø POLY STRAND TENDON. POST TENSION FORCE = 28,000 lbs

Notes:
1. Orientation of plan is from the form view.
2. Trim plastic sheath at both ends of post tensioned cable to fit tight between anchor plates.
3. Note clearance on drawing from edge of panel to outside of cable.
4. Reinforcing sequence:
Place chairs under mesh
Place mesh and install Burke lift
Install cable. Hold cable at vertical center of the panel
5. Hold reinforcing bars 1 1/2 inch clear from edge of forms.



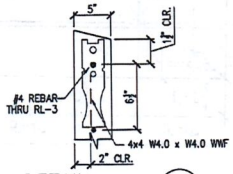
ROOF EDGE DETAIL (A)

SCALE: NONE



ROOF EDGE DETAIL (B)

SCALE: NONE



WALL DETAIL (C)

SCALE: NONE

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb

CU. YARD:	SQ. FT:	WEIGHT:

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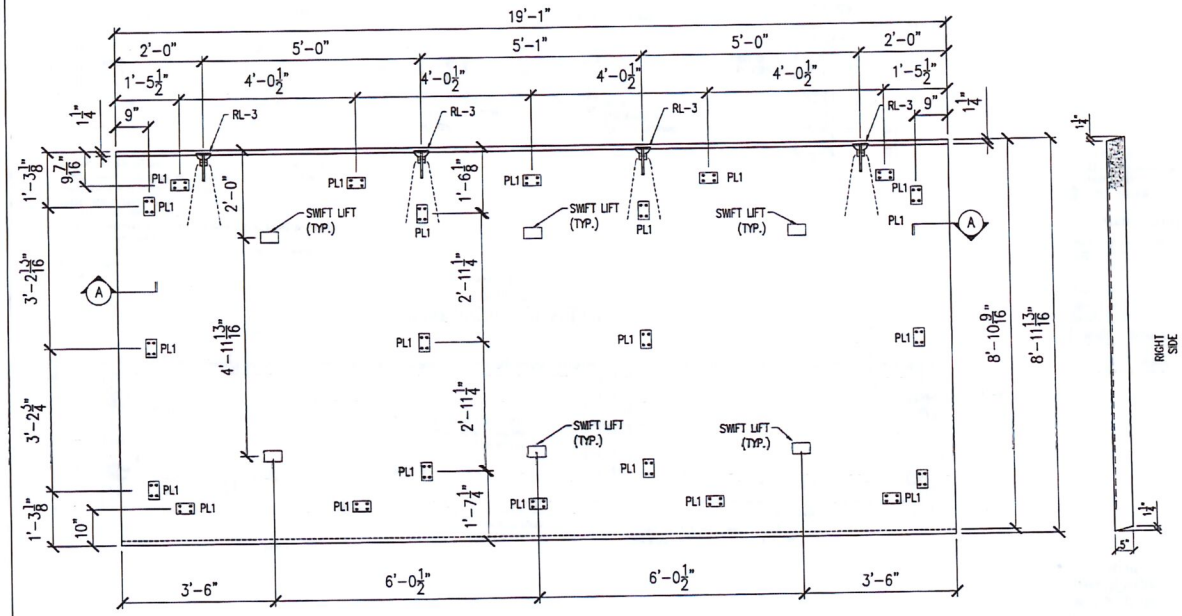
LINDEN C. PARCHMENT LICENSE No. 54071 STATE OF FLORIDA PROFESSIONAL ENGINEER

SCALE: AS SHOWN		DATE: 4/20/2017
DESIGN: WDA	DRAWN: JK/LP	CHECKED: WDA

CLIENT: LEESBURG CONCRETE

PROJECT: OUTBACK SIERRA RESTROOM
for
MONTESSORI SCHOOL
17800 OLD CUTLER RD.
PALMETTO BAY, FL. 33157

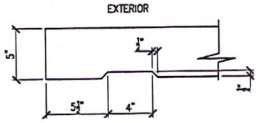
JOB NO: 16-57 SHEET: 22 OF: 42



LAYOUT - PANEL R2

FORM VIEW

SCALE: 3/8" = 1'-0"



DETAIL AT PANEL EDGE (A)

SCALE: 1 1/2" = 1'-0"

MARK

DESCRIPTION	QUANTITY
MEADOW BURKE RL-3 GALV. w/ RL-31 SHEAR BAR AND RL-32 TENSION BAR	4
PL1	22
PL2	
PL3	
SWIFT LIFT (2 TON)	6
CONCRETE VOLUME	2.6 CU YD
PANEL WEIGHT	10,600 lb

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY

EXT FINISH:

INT FINISH:

MIX DESIGN: 5,000 PSI

RELEASE @ 3000 PSI

CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:

POST TENSION FORCE: 28,000 lb

CU. YARD: | SQ. FT: | WEIGHT:

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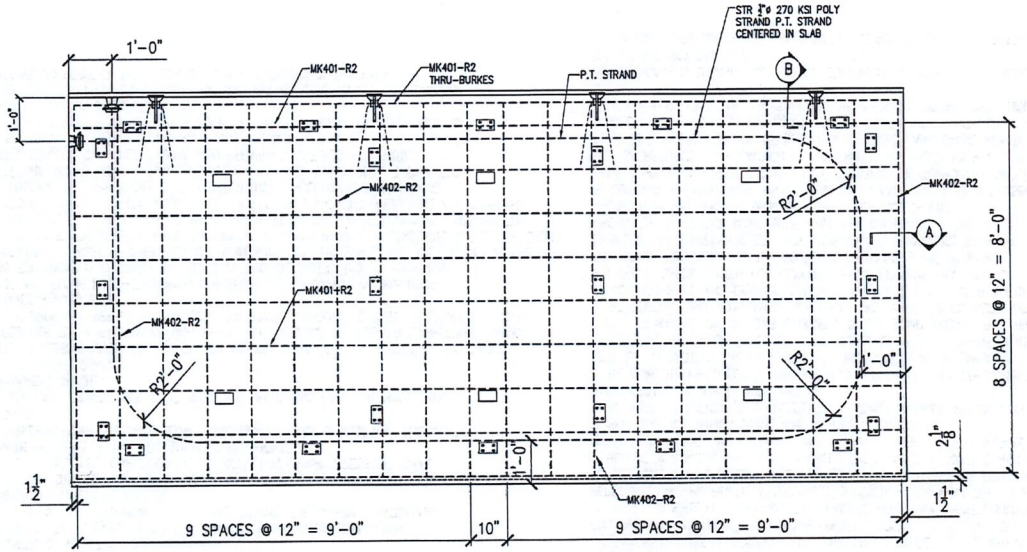
No. 54071
STATE OF FLORIDA
PROFESSIONAL ENGINEER

SCALE: AS SHOWN	DATE: 4/20/2017
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 23 OF: 42

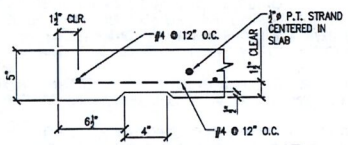


REINFORCING - PANEL R2

FORM VIEW

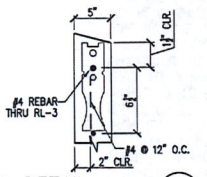
SCALE: ⅛" = 1'-0"

Notes:
1. Orientation of plan is from the form view.
2. Trim plastic sheath at both ends of post tensioned cable to fit tight between anchor plates.
3. Note clearance on drawing from edge of panel to outside of cable.
4. Reinforcing sequence:
Place chairs under mesh
Place mesh and install Burke lift
Install cable. Hold cable at vertical center of the panel
5. Hold reinforcing bars 1 1/2 inch clear from edge of forms.



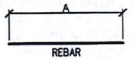
ROOF EDGE DETAIL (A)

SCALE: NONE



WALL DETAIL (B)

SCALE: NONE



MARK	QUANTITY	LENGTH "A"
MK401-R2	10	18'-10"
MK402-R2	20	8'-6"

MK402-R1 — PANEL REF.
└ INDICATES REBAR SIZE (4=#4 ETC.)

POST TENSION CASTING ANCHOR AND WEDGE FOR 47'-4" LONG ½"ø POLY STRAND TENDON. POST TENSION FORCE = 28,000 lbs

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb
CU. YARD: SQ. FT: WEIGHT:

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DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 24 OF: 42

TENDON STRESSING PROCEDURES FOR ROOFS AND FLOORS

1. REMOVE FORM RAILS. VERIFY CONCRETE STRENGTH WITH CYLINDER BREAKS. CONCRETE SHALL BE A MINIMUM OF 3,750 PSI BEFORE CABLE CAN BE STRESSED.
2. REMOVE RECESSED PLASTIC POCKET-FORMERS AT THE POST-TENSIONING ANCHOR.
3. REMOVE THE PLASTIC SHEATH FROM THE STRAND ALL THE WAY FROM THE BASE OF THE ANCHOR TO THE END OF THE STRAND. CLEAN OFF GREASE FROM EXPOSED PORTION OF CABLE. USE A SUITABLE SOLVENT. NEVER HEAT OR USE FLAME ON THE CABLE.
4. INSURE THAT THE TAPERED HOLES IN THE ANCHORS ARE CLEANED AND FREE OF ANY DEBRIS OR CEMENT PASTE, WHICH MIGHT INTERFERE WITH PROPER WEDGE SEATING.
5. INSTALL WEDGE SETS AROUND THE STRAND AT EACH ANCHOR. PRESS THE WEDGES IN WITH THE FINGERS EVENLY. TAP WEDGES IN SECURELY. MAKE SURE THE ENDS OF THE WEDGES ARE EVEN WITH EACH OTHER SO THAT THE STRESS WILL BE EVENLY DISTRIBUTED OVER THE WIRES OF THE STRAND.
6. SLIDE THE WEDGE-SETTER OVER THE CABLE AND PLACE AGAINST THE WEDGES INSTALLED IN THE CAST-IN ANCHORS (SEE DETAIL 4 ON SHEET 26).
7. INSTALL THE TEMPORARY REUSABLE ANCHOR ON THE STRAND. THE PLATES OF THE JACK WILL PUSH AGAINST THIS ANCHOR TO ELONGATE AND, THEREFORE, STRESS THE STRAND. SET THE TEMPORARY STRESSING ANCHOR APPROXIMATELY 5" BEYOND THE REAR OF THE WEDGE-SETTER (SEE DETAIL 6 ON SHEET 26).
8. INSTALL THE PLATES OF THE TWIN RAM STRESSING JACK OVER THE STRAND BETWEEN THE WEDGE-SETTER AND THE REUSABLE STRESSING ANCHOR.
9. STANDING BACK FROM THE JACK, USING A REMOTE SWITCH FOR THE PUMP, STRESS THE STRANDS UNTIL THE PUMP PRESSURE GAUGE INDICATES A PRESSURE LISTED ON THE CALIBRATION CHART OF YOUR STRESSING PUMP, THAT CORRESPONDS TO 28 K REQUIRED TENSION ON THE ½" CABLE. WHENEVER STRESSING BE SURE TO OBSERVE SAFETY PRECAUTIONS AND OPERATOR POSITIONING IN ANTICIPATION OF UNEXPECTED FAILURES. NOTE: AS A DOUBLE-CHECK TO INSURE PROPER ELONGATION OF THE STRAND, MEASURE THE DISTANCE THE JACK HAS STRETCHED THE CABLE. THIS DIMENSION SHOULD BE IN ACCORDANCE WITH THE FOLLOWING FORMULA:
LENGTH OF TOTAL CABLE X .0795 = THE LENGTH OF CABLE ELONGATION.
10. RELEASE PUMP PRESSURE SLOWLY AND REMOVE THE JACKING SYSTEM AND TEMPORARY STRESSING ANCHOR.
11. REPEAT STRESSING OPERATION AT OTHER END OF CABLE. TO INSURE THAT CABLE IS FULLY TENSIONED. LINE FRICTION MAY RESULT IN LOWER STRESS OF TOTAL LENGTH OF CABLE IF THIS PROCEDURE IS NOT DONE.
12. RELEASE PUMP PRESSURE SLOWLY AND REMOVE JACK SYSTEM AND TEMPORARY ANCHOR.
12. CUT OFF STRAND TAILS AT 1" FROM THE WEDGE AND PACK THE HOLES WITH PERMANENT GROUT.

SAFETY INSTRUCTIONS:
NOTE: STRESSING UNITS ARE CALIBRATED AGAINST LOAD CELLS TO DETERMINE THE GAUGE PRESSURES FOR A PARTICULAR UNIT WHICH WILL PRODUCE THE DESIRED ENGINEERING FORCE. GAUGE PRESSURE VERSUS ACTUAL LOAD CALIBRATION SHOULD BE CHECKED EVERY SIX MONTHS AGAINST A LOAD CELL.
ROOF AND FLOOR POST-TENSIONING STRESSING SYSTEM SAFETY INSTRUCTIONS
THIS EQUIPMENT IS DESIGNED AND BUILT TO PROVIDE SAFETY DURING EQUIPMENT OPERATION. IT MUST BE OPERATED AND MAINTAINED BY PERSONNEL WHO ARE TRAINED IN AND FOLLOW SAFE PROCEDURES. SINCE THE FORCES INVOLVED IN TENDON STRESSING ARE IN THE ORDER OF 28 TO 30 KIPS, IT IS OBVIOUS THAT FAILURE FROM ANY CAUSE COULD BE DAMAGING TO PERSONNEL AND PROPERTY. ACCORDINGLY, STRESSING EQUIPMENT OPERATORS ARE EXPECTED TO OBSERVE ALL APPLICABLE SAFETY PRECAUTIONS INCLUDING THE FOLLOWING:
1. BEFORE ACTIVATING THE PUMP, VISUALLY CHECK ALL ELASTOMERIC TUBING FOR NICKS, CRACKS, OR OTHER DAMAGE AND REPAIR OR REPLACE IF REQUIRED.
2. SEE THAT ALL TUBING JOINTS ARE TIGHTLY CONNECTED.
3. BE SURE THAT PUMP IS SUPPORTED IN A STABLE MANNER AND POSITIONED TO AVOID STRAIN ON CONNECTION LINES.
4. BE SURE THAT PROTECTIVE DUNNAGE AND APPROPRIATE SAFETY BARRIERS ARE ERECTED TO PROTECT THE OPERATOR AND ANY OTHERS IN THE AREA.
5. OBSERVE THE GAUGE PRESSURE THAT WILL DEVELOP THE REQUIRED TENSILE FORCE AS MARKED ON THE TAG ATTACHED TO THE PUMP. LOCATE THIS READING ON GAUGE FACE.
6. BE CERTAIN THAT TENSIONING TWIN-RAMS ARE POSITIVELY ENGAGED AND CORRECTLY ALIGNED.
7. ACTIVATE PUMP AND OBSERVE GAUGE. IF PRESSURE DOES NOT BUILD UP PROMPTLY, CHECK SYSTEM FOR LEAKAGE AND MAKE SURE THAT ANCHORS AND WEDGES ON BOTH ENDS OF THE POLYSTRAND®TENDONS ARE PROPERLY ENGAGED. WHEN PRESSURE REACHES APPROXIMATELY 1000 PSI, STOP AND CHECK FOR ALIGNMENT. MAKE SURE THAT THE STRAND IS AT THE TOP OF THE SLOT PROVIDED IN THE TENSION JACK CYLINDER CONNECTOR PLATES.
8. CONTINUE PUMPING UNTIL FINAL PRESSURE IS ATTAINED. WHEN STRESSING LONG TENDONS, MULTIPLE STROKES AND RESETTING THE TEMPORARY ANCHOR WILL BE REQUIRED. - - CAUTION - - PRESSURE WILL BUILD EXTREMELY FAST IF RAMS ARE FULLY EXTENDED. MONITOR RAM EXTENSION AND GAUGE READINGS CONTINUOUSLY.
9. WHEN THE CORRECT FORCE HAS BEEN APPLIED TO THE TENDON, STOP THE PUMP MOTOR AND RELEASE THE PRESSURE ON THE PACK BY OPENING THE HYDRAULIC VALVE.
10. USE CARE IN MOVING AND STORING EQUIPMENT TO INSURE THAT GAUGE AND TUBING ARE NOT DAMAGED.
11. OBSERVE SITE RULES PERTAINING TO FREQUENCY OF GAUGE CALIBRATION.

SAFETY INSTRUCTIONS:
NOTE: STRESSING UNITS ARE CALIBRATED AGAINST LOAD CELLS TO DETERMINE THE GAUGE PRESSURES FOR A PARTICULAR UNIT WHICH WILL PRODUCE THE DESIRED ENGINEERING FORCE. GAUGE PRESSURE VERSUS ACTUAL LOAD CALIBRATION SHOULD BE CHECKED EVERY SIX MONTHS AGAINST A LOAD CELL. THE SAMPLE TABLE ON A1.08 LISTS THE CURRENT EQUIPMENT IN SERVICE. NEVER USE EQUIPMENT WHICH HAS NOT BEEN TESTED IN THE LAST 6 MONTHS. THE QUALITY CONTROL DEPARTMENT HAS THE RESPONSIBILITY TO INSURE THE STRESSING UNIT IS CALIBRATED SEMI-ANNUALLY.
ROOF AND FLOOR POST-TENSIONING STRESSING SYSTEM SAFETY INSTRUCTIONS
THIS EQUIPMENT IS DESIGNED AND BUILT TO PROVIDE SAFETY DURING EQUIPMENT OPERATION. IT MUST BE OPERATED AND MAINTAINED BY PERSONNEL WHO ARE TRAINED IN AND FOLLOW SAFE PROCEDURES. SINCE THE FORCES INVOLVED IN TENDON STRESSING ARE IN THE ORDER OF 28 TO 30 KIPS, IT IS OBVIOUS THAT FAILURE FROM ANY CAUSE COULD BE DAMAGING TO PERSONNEL AND PROPERTY. ACCORDINGLY, STRESSING EQUIPMENT OPERATORS ARE EXPECTED TO OBSERVE ALL APPLICABLE SAFETY PRECAUTIONS INCLUDING THE FOLLOWING:
1. BEFORE ACTIVATING THE PUMP, VISUALLY CHECK ALL ELASTOMERIC TUBING FOR NICKS, CRACKS, OR OTHER DAMAGE AND REPAIR OR REPLACE IF REQUIRED.
2. SEE THAT ALL TUBING JOINTS ARE TIGHTLY CONNECTED.
3. BE SURE THAT PUMP IS SUPPORTED IN A STABLE MANNER AND POSITIONED TO AVOID STRAIN ON CONNECTION LINES.
4. BE SURE THAT PROTECTIVE DUNNAGE AND APPROPRIATE SAFETY BARRIERS ARE ERECTED TO PROTECT THE OPERATOR AND ANY OTHERS IN THE AREA.
5. OBSERVE THE GAUGE PRESSURE THAT WILL DEVELOP THE REQUIRED TENSILE FORCE AS MARKED ON THE TAG ATTACHED TO THE PUMP. LOCATE THIS READING ON GAUGE FACE.
6. BE CERTAIN THAT TENSIONING TWIN-RAMS ARE POSITIVELY ENGAGED AND CORRECTLY ALIGNED.
7. ACTIVATE PUMP AND OBSERVE GAUGE. IF PRESSURE DOES NOT BUILD UP PROMPTLY, CHECK SYSTEM FOR LEAKAGE AND MAKE SURE THAT ANCHORS AND WEDGES ON BOTH ENDS OF THE POLYSTRAND®TENDONS ARE PROPERLY ENGAGED. WHEN PRESSURE REACHES APPROXIMATELY 1000 PSI, STOP AND CHECK FOR ALIGNMENT. MAKE SURE THAT THE STRAND IS AT THE TOP OF THE SLOT PROVIDED IN THE TENSION JACK CYLINDER CONNECTOR PLATES.
8. CONTINUE PUMPING UNTIL FINAL PRESSURE IS ATTAINED. WHEN STRESSING LONG TENDONS, MULTIPLE STROKES AND RESETTING THE TEMPORARY ANCHOR WILL BE REQUIRED. - - CAUTION - - PRESSURE WILL BUILD EXTREMELY FAST IF RAMS ARE FULLY EXTENDED. MONITOR RAM EXTENSION AND GAUGE READINGS CONTINUOUSLY.
9. WHEN THE CORRECT FORCE HAS BEEN APPLIED TO THE TENDON, STOP THE PUMP MOTOR AND RELEASE THE PRESSURE ON THE PACK BY OPENING THE HYDRAULIC VALVE.
10. USE CARE IN MOVING AND STORING EQUIPMENT TO INSURE THAT GAUGE AND TUBING ARE NOT DAMAGED.
11. OBSERVE SITE RULES PERTAINING TO FREQUENCY OF GAUGE CALIBRATION.

LINDEN C. PARCHMENT LICENSE NO. 54071 STATE OF FLORIDA PROFESSIONAL ENGINEER

4/20/17

REVISIONS

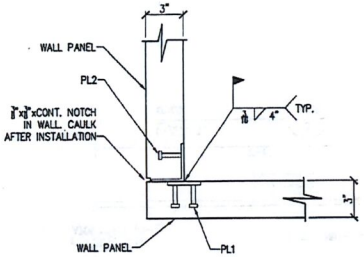
ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
	4	BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb
CU. YARD: SQ. FT: WEIGHT:

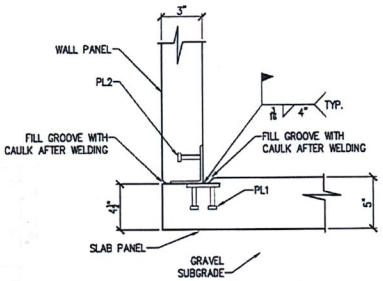
Linden Parchment P.E.
8002 Glitter Court
Orlando, FL 32836
(407) 810-3322 P.E. # 54071

W. DAVID ARNOLD
ARCHITECT
106 PORTLAND STREET
EUSTIS, FL. 32726
(352) 357-5807 AR0013558

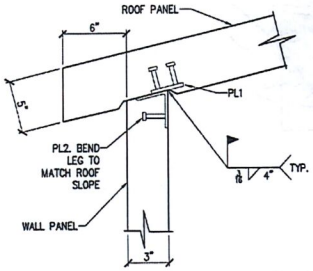
SCALE: AS SHOWN
DATE: 4/20/2017
DESIGN: WDA
DRAWN: JK/LP
CHECKED: WDA
CLIENT: LEESBURG CONCRETE
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL
17800 OLD CUTLER RD.
PALMENTO BAY, FL. 33157
JOB NO: 16-57
SHEET: 25
OF: 42



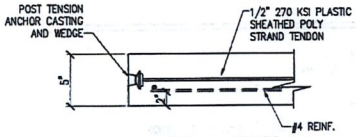
TYPICAL WALL TO WALL CONN. (1)
SCALE: 1 1/2" = 1'-0"



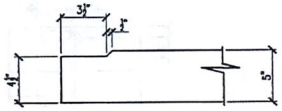
TYPICAL WALL TO FLOOR CONN. (2)
SCALE: 1 1/2" = 1'-0"



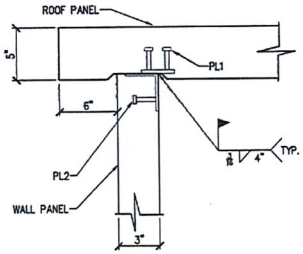
TYPICAL WALL TO ROOF CONN. (3)
SCALE: 1 1/2" = 1'-0"



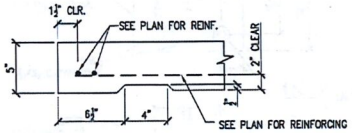
POST TENSION ANCHOR DETAIL (4)
SCALE: 1 1/2" = 1'-0"



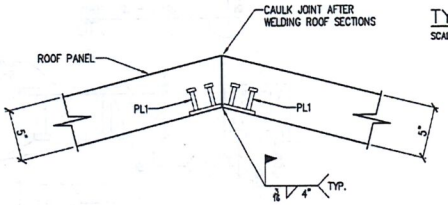
FLOOR RECESS DETAIL (5)
SCALE: 1 1/2" = 1'-0"



TYPICAL WALL TO ROOF CONN. (8)
SCALE: 1 1/2" = 1'-0"



ROOF EDGE DETAIL (6)
SCALE: NONE



ROOF TO ROOF CONN. AT ALTERNATE (7)
SCALE: 1 1/2" = 1'-0"

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb

CU. YARD:	SQ. FT:	WEIGHT:

Linden Parchment P.E.

8002 Glitter Court
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(407) 810-3322 P.E. # 54071

W. DAVID ARNOLD
ARCHITECT

106 PORTLAND STREET
EUSTIS, FL. 32726

(352) 357-5807 AR0013558

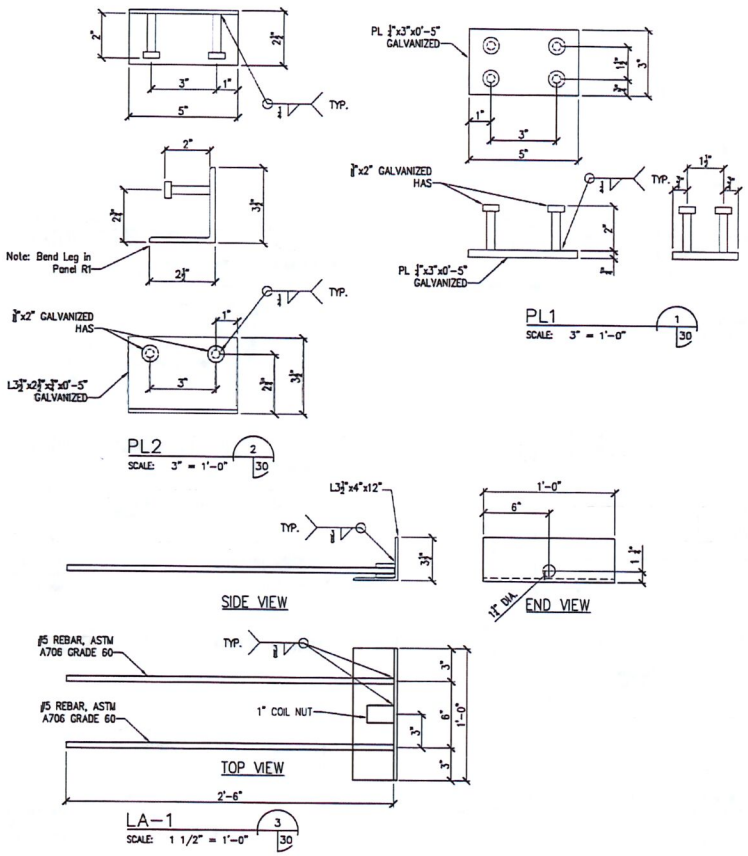
LINDEN C. PARCHMENT
LICENSE
No. 54071
STATE OF FLORIDA
PROFESSIONAL ENGINEER

SCALE:		DATE:
AS SHOWN		4/20/2017
DESIGN: WDA	DRAWN: JK/LP	CHECKED: WDA

CLIENT: LEESBURG CONCRETE

PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL
17800 OLD CUTLER RD.
PALMENTO BAY, FL. 33157

JOB NO: 16-57 SHEET: 26 OF: 42



REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
	4	BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb

CU. YARD:	SQ. FT:	WEIGHT:

Linden Parchment P.E.

8002 Glitter Court
Orlando, FL 32836

(407) 810-3322 P.E. # 54071

W. DAVID ARNOLD
ARCHITECT

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EUSTIS, FL. 32726

(352) 357-5807 AR0013558

LINDEN C. PARCHMENT
LICENSE
No. 54071
STATE OF
FLORIDA
PROFESSIONAL ENGINEER

SCALE: AS SHOWN		DATE: 4/20/2017
DESIGN: WDA	DRAWN: JK/LP	CHECKED: WDA

CLIENT: LEESBURG CONCRETE

PROJECT: OUTBACK SIERRA RESTROOM
for
MONTESSORI SCHOOL
17800 OLD CUTLER RD.
PALMENTO BAY, FL. 33157

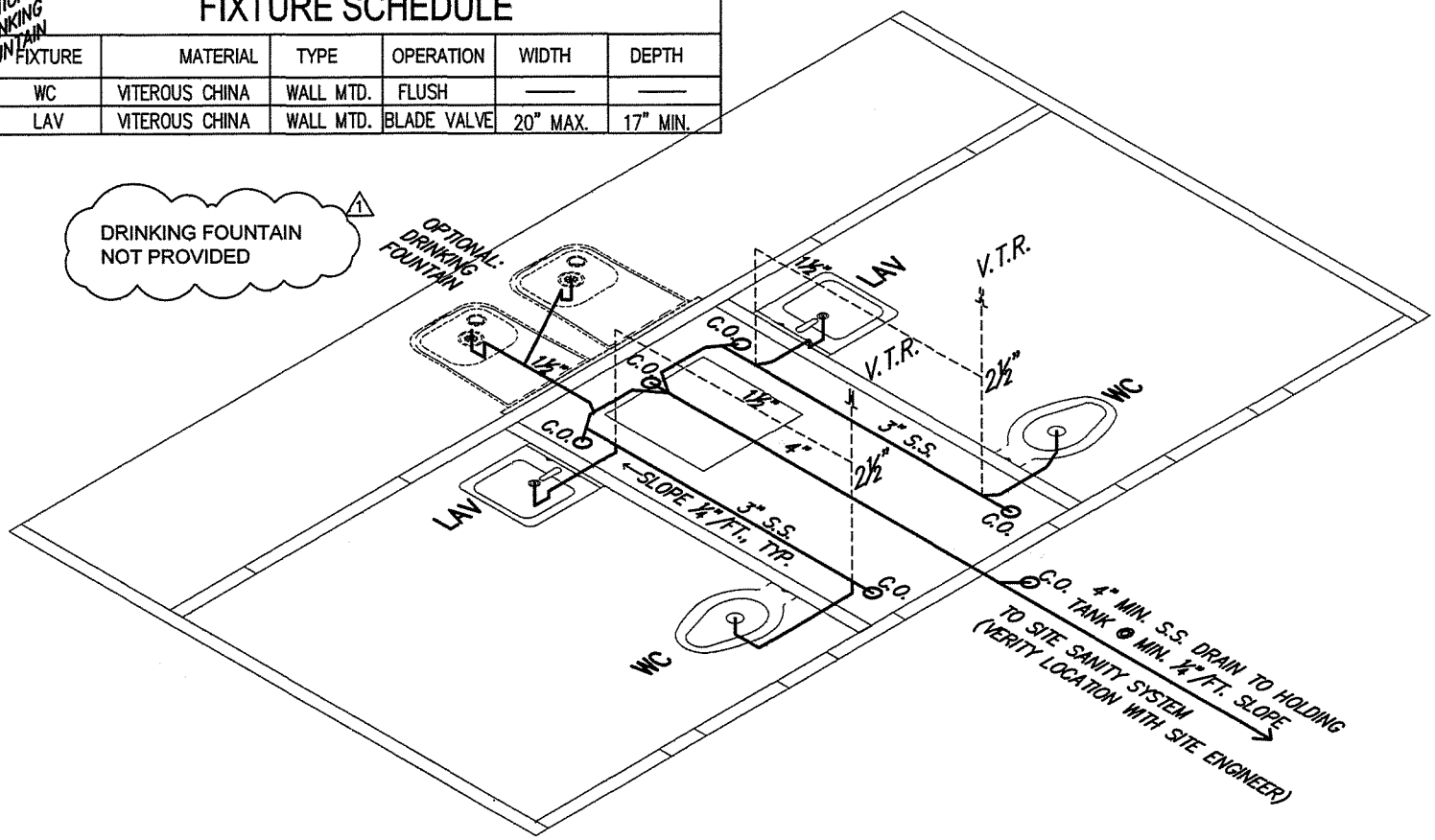
JOB NO: 16-57

SHEET: 27
OF: 42

OPTIONAL: DRINKING FOUNTAIN

FIXTURE SCHEDULE

FIXTURE	MATERIAL	TYPE	OPERATION	WIDTH	DEPTH
WC	VITEROUS CHINA	WALL MTD.	FLUSH	——	——
LAV	VITEROUS CHINA	WALL MTD.	BLADE VALVE	20" MAX.	17" MIN.



WET VENT WASTE RISER

SCALE: NONE

REVISIONS

1 DATE: 03/22/2017
CHANGED ALL WIND SPEED REFERENCES FROM 170 TO 175 & RECALCULATED ROOF & WALL PRESSURES

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SHIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONCRETE STRENGHT FOR POST TENSIONING: 4,000CCL YARD:
POST TENSION FORCE: 28,000 PSI
CIL YARD: | SQ. FT: | WEIGHT:

W. DAVID ARNOLD
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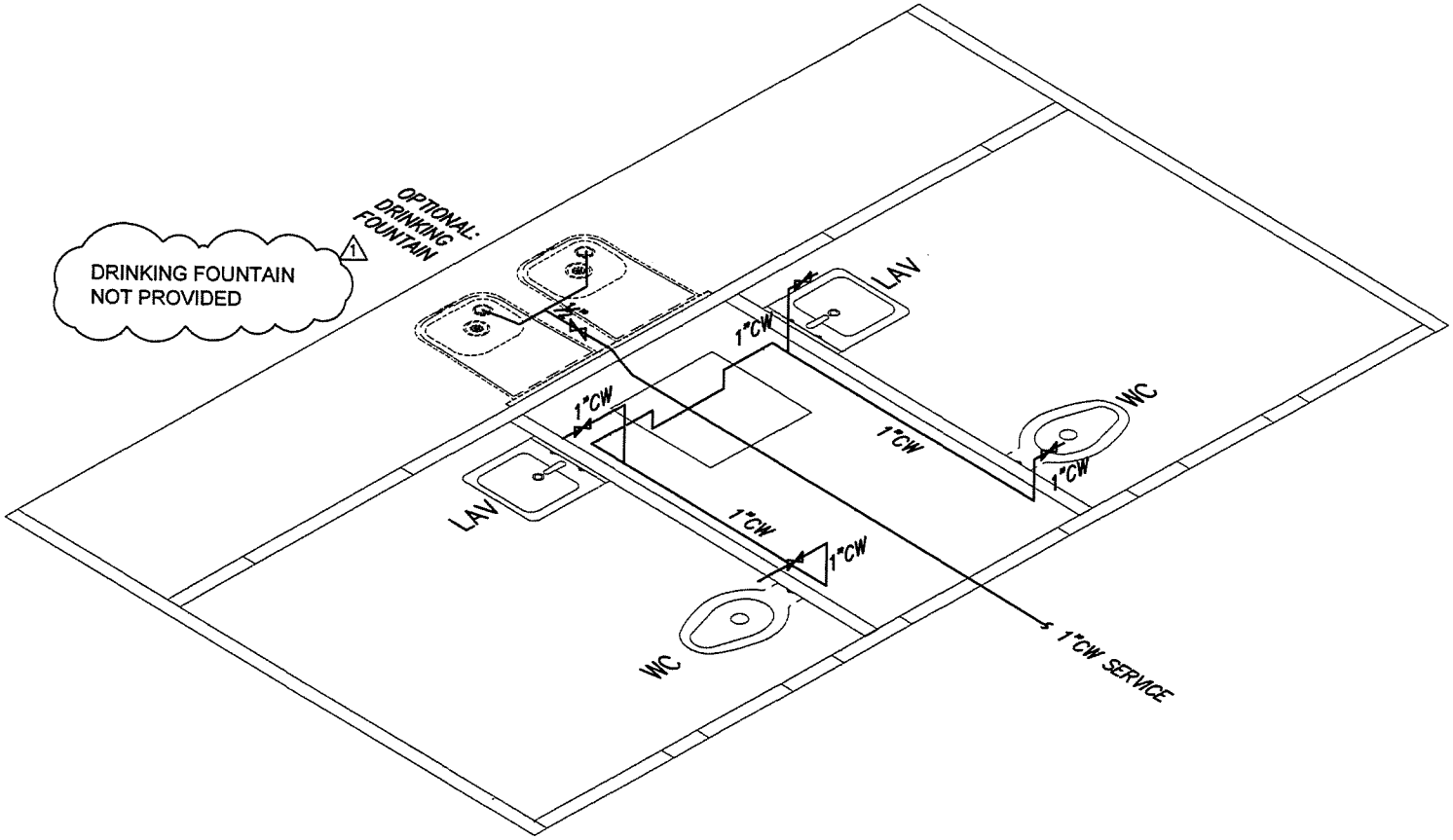
EUSTIS, FL 32726
(352) 357-5807 AR0013558

David Arnold 3/4/2017

SCALE: AS SHOWN	DATE: 10/6/16
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 30 OF: 42



SUPPLY RISER - CW

SCALE: NONE

REVISIONS

1 DATE: 03/22/2017
CHANGED ALL WIND SPEED REFERENCES FROM 170 TO 175 & RECALCULATED ROOF & WALL PRESSURES

ITEM	QTY	DESCRIPTION
		BURNE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURNE SHIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 2000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONCRETE STRENGHT FOR POST TENSIONING: 4,000CUL YARD
POST TENSION FORCE: 30,000 PSI

CU. YARD:	SQ. FT:	WEIGHT:

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David Arnold
24 Apr 17

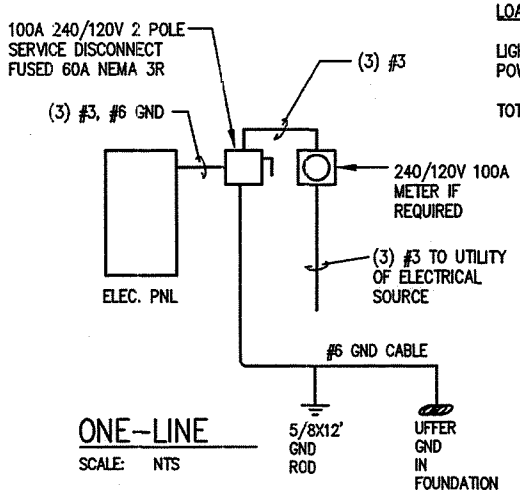
SCALE: AS SHOWN		DATE: 10/6/16
DESIGN: WDA	DRAWN: JK/LP	CHECKED: WDA

CLIENT: LEESBURG CONCRETE

PROJECT: OUTBACK SIERRA RESTROOM
for
MONTESSORI SCHOOL
17800 OLD CUTLER RD.
PALMENTO BAY, FL. 33157

JOB NO: 16-57

SHEET: 31 OF: 42



ONE-LINE

SCALE: NTS

LOAD CALCULATIONS

LIGHTING	100 VA
POWER	50 VA
TOTAL	150 VA
	1.2 AMPS

DUPLEX RECEPTACLE-WALL MOUNTED

TWIN LAMP EMERGENCY LIGHT CONNECT AHEAD OF LIGHT SWITCH

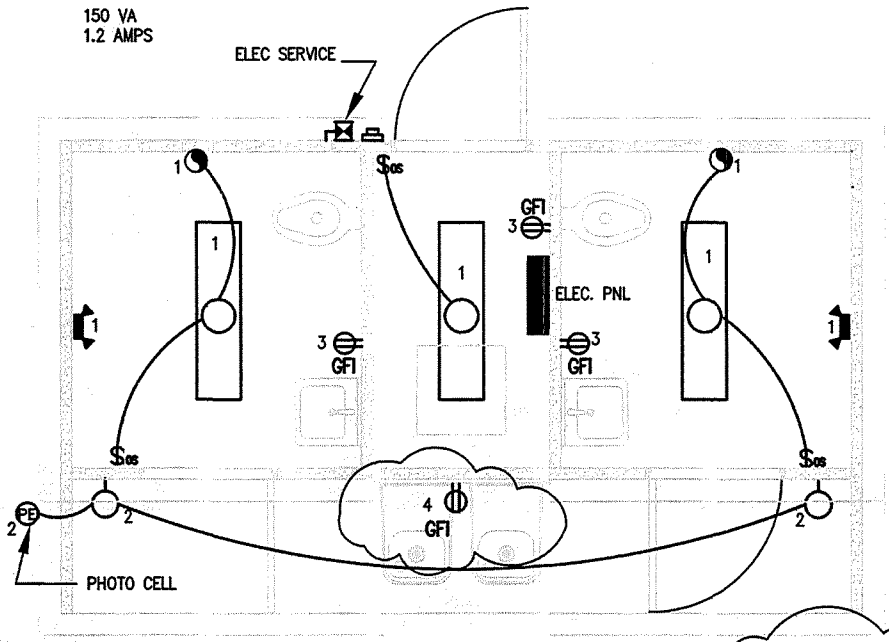
1x4 2 LAMP T-8 SURFACE FLUORESCENT FIXTURE

EXTERIOR LIGHT RAB 50 MOD

SWITCH-SPST (OS) OCCUPANCY SENSOR

PANELBOARD

WALL EXHAUST FAN



ELECTRICAL PLAN

SCALE: 3/8" = 1'-0"

WIRE SIZE CHART						(FOR 3% VOLTAGE DROP @ 80% END OF LINE LOAD)
WIRE SIZE	#12	#10	#8	#6	#4	
120V 20A	57'	91'	145'	229'	365'	
120V 30A	NA	80'	96'	153'	244'	
120V 40A	NA	NA	72'	115'	183'	
120V 50A	NA	NA	NA	92'	146'	

NOTE :
- WIRE SIZES FOR 120V BRANCH CIRCUITS SHALL BE AS SHOWN BY THE CHART ABOVE OR SHALL BE CALCULATED NOT TO EXCEED 3% VOLTAGE DROP.
- ALL CIRCUITS FOR VOLTAGES NOT SHOWN OR FOR DISTRIBUTIVE LOAD SHALL BE CALCULATED NOT TO EXCEED 3% VOLTAGE DROP.
- ALL FEEDER CIRCUITS SHALL BE CALCULATED NOT TO EXCEED 2% VOLTAGE DROP.
- FEEDERS & BRANCH CIRCUIT COMPLIES WITH FBC & NEC (2% VD FOR FEEDERS & 3% VD FOR BRANCH CIRCUITS)

VOLTS L-N : 120 PANEL : "A" ENCLOSURE DATA
VOLTS PH. : 240 MCB : AMPS AIC RATING 10 KA NEMA : 1
PHASE : 1 MLO : 100 AMPS SECTIONS : 1

DESCRIPTION	C.B. AMPS	C.B. POLES	CKT. NUM.	CKT. NUM.	C.B. POLES	C.B. AMPS	DESCRIPTION
LIGHTING	20	1	1	2	1	20	LIGHTS
RECEPTS	20	1	3	4	1	20	DRINKING FOUNTAIN
SPACE	20	1	5	6	1	20	SPACE
SPACE	20	1	7	8	1	20	SPACE
SPACE	20	1	9	10	1	20	SPACE
SPACE	20	1	11	12	1	20	SPACE

ROBERT C. MUNROE P.E.

REVISIONS

7/17/2012 - REVISE INTERIOR LIGHTS AND LOAD CALCULATIONS TO MEET FL ENERGY CODE TABLE 505.5.3

REV DRINKING FOUNTAIN & WIND SPEED

RCM-BBR
ENGINEERING LLC
CERT of AUTH # 26442
P.O. BOX 121721
CLERMONT
FL 34712
407-443-1853
ROBERT C. MUNROE
FL PE #33868

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONCRETE STRENGHT FOR POST TENSIONING: 4,5000CU. YARD:
POST TENSION FORCE: 28,000 PSI

CU. YARD:	SQ. FT:	WEIGHT:

W. DAVID ARNOLD
ARCHITECT
106 PORTLAND STREET
EUSTIS, FL 32726
(352) 357-5807 AR0013558

SCALE: AS SHOWN	DATE: 10/6/16
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL 33157	
JOB NO: 16-57	SHEET: 32 OF: 42

PRECAST RESTROOM BUILDING
OUTBACK SIERRA 175 MPH

BASIC MATERIALS AND METHODS

A. RACEWAYS AND FITTINGS: ALL RACEWAYS AND FITTINGS SHALL BE GALVANIZED RIGID STEEL OR INTERMEDIATE METAL CONDUIT WITH LOCKNUTS AND BUSHINGS, WITH THE EXCEPTION THAT WHERE SPECIFICALLY ALLOWED BY THE NATIONAL ELECTRICAL CODE AND APPLICABLE LOCAL CODES. ELECTRICAL METALLIC TUBING (E.M.T) MAY BE USED FOR ALL INTERIOR EXPOSED AND CONCEALED WORK WHERE IT IS NOT SUBJECT TO PHYSICAL DAMAGE OR CORROSION. FITTINGS SHALL BE STEEL SET SCREW TYPE. PVC SCHEDULE 40 SHALL BE USED FOR BELOW GRADE INSTALLATIONS. INSTALL EXPANSION FITTINGS IN RACEWAYS EVERY 200' LINEAR RUN OR WHEREVER STRUCTURAL EXPANSION JOINTS ARE CROSSED.

B. CONDUCTORS: FEEDER AND BRANCH CIRCUIT CONDUCTORS SHALL BE #12 (UNLESS OTHERWISE NOTED) TYPE THWN/THHN STRANDED COPPER, AND SHALL MEET FBC 13-413 AND NEC 210.19(A)(1). ALL OFFICE INTERIOR RECEPTACLE OUTLETS AND LIGHTING CIRCUITS SHALL BE CONNECTED UTILIZING TYPE "MC" CABLE OR 1/2" EMT, WITH MINIMUM 2#12, 1#12G TYPE, THWN/THHN WIRE, UNLESS OTHERWISE NOTED NO ALUMINUM 2/0 AND SMALLER SHALL BE PERMITTED UNLESS SPECIFICALLY NOTED. AN EQUIPMENT GROUNDING CONDUCTOR SHALL BE PROVIDED IN ALL RACEWAY SYSTEMS. INSTALL ALL WIRING IN CONDUIT OR APPROVED RACEWAYS, UNLESS OTHERWISE INDICATED. ALL BRANCH CIRCUITS SHALL BE WIRED WITH COLOR-CODED WIRE WITH THE SAME COLOR USED FOR A PHASE THROUGHOUT.

C. FINAL CONNECTIONS TO MOTORS OR EQUIPMENT SUBJECT TO VIBRATION SHALL BE MADE WITH GREENFIELD IN DRY LOCATIONS AND WITH JACKETED, LIQUID-TIGHT, FLEXIBLE, GALVANIZED STEEL CONDUIT IN WET OR DAMP LOCATIONS. LOCATE ALL DEVICES AND EQUIPMENT WHERE SHOWN ON DRAWINGS. ALL DIMENSIONS SHOWN ON THE DRAWINGS ARE FROM FINISHED FLOOR TO THE CENTER OF THE DEVICE, UNLESS OTHERWISE INDICATED. STANDARD MOUNTING HEIGHTS ARE AS FOLLOWS:

D. PROVIDE PANELBOARDS AND DISTRIBUTION PANELS OF THE DEAD FRONT TYPE. CIRCUIT BREAKERS: MOLDED CASE, THERMAL-MAGNETIC QUICK-MAKE, QUICK-BREAK TYPE WITH TRIP-FREE HANDLE.

E. FURNISH AND INSTALL COMPLETE LIGHTING SYSTEM, FIXTURES, LAMPS, ACCESSORIES AND SUPPORT, AS SPECIFIED AND SHOWN. CEILING GRID LAYOUTS ON PLANS ARE FOR CONVENIENCE ONLY. THE CONTRACTOR SHALL COORDINATE THE LIGHT FIXTURE LAYOUT WITH ARCHITECT, GENERAL CONTRACTOR, MECHANICAL CONTRACTOR & OWNER. ALL LIGHT FIXTURES SHALL BE APPROVED BY OWNER. LIGHTING FIXTURES SHALL BE AS SHOWN ON THE DRAWING AND SHALL BE FURNISHED WITH LAMPS INSTALLED.

F. FURNISH AND INSTALL COMPLETE GROUNDING SYSTEM, GROUND CONDUCTOR, GROUND ELECTRODES, GROUND BUS & GROUND ROD TO EFFECTIVELY PROVIDE SERVICE GROUND. GROUND ALL PANELS, WIREWAYS, CONDUITS, LIGHT FIXTURES, RECEPTACLES IN ACCORDANCE WITH CODE REQUIREMENTS. GROUND CONDUCTORS SHALL BE INSTALLED IN ALL FEEDERS AND BRANCH CIRCUIT CONDUITS.

G. ALL SINGLE-PHASE BRANCH CIRCUITS, 120 VOLT LIGHTING, RECEPTACLES, & MOTORS SHALL CONSIST OF PHASE, NEUTRAL & GROUND. MILTIWIRE BRANCH CIRCUIT IF USED SHALL BE PROVIDED WITH MEANS THAT WILL SIMULTANEOUSLY DISCONNECT ALL UNGROUNDED CONDUCTORS AT THE POINT WHERE THE BRANCH CIRCUIT ORIGINATES.

H. WIRING DEVICES: ALL DEVICES SHALL BE SPECIFICATION GRADE OF THE SAME MANUFACTURER. WALL SWITCHES - SINGLE POLE 20A, 120/277 VOLT, RECEPTACLES DUPLEX 2 WIRE, 3 POLE GROUNDED 15A, 120 VOLT (20A, 120 VOLT IF ONLY ONE RECEPTACLE ON 20A CIRCUIT). RECEPTACLES MOUNTED IN FIRE OR SMOKE RATED WALLS SHALL BE MINIMUM 25" APART.

GENERAL PROVISIONS

A. PROVIDE ALL LABOR, MATERIALS AND EQUIPMENT FOR A COMPLETE AND PROPERLY OPERATING ELECTRICAL SYSTEM. INCLUDE NECESSARY FEES AND PERMITS.

B. THE INSTALLATION SHALL COMPLY WITH THE 2015 FBC AND THE LATEST EDITION OF ALL GOVERNING FEDERAL, STATE AND LOCAL CODES OR ORDINANCES AND THE 2011 NATIONAL ELECTRICAL CODE (NFPA 70).

C. ALL MATERIALS SHALL BE FREE OF DEFECTS AND SHALL BEAR THE UNDERWRITER'S LABORATORIES (U.L.) LABEL OR BE LABELED OR LISTED WITH AN APPROVED NATIONALLY RECOGNIZED ELECTRICAL TESTING AGENCY.

D. THE ELECTRICAL DRAWINGS ARE SCHEMATIC ONLY. COORDINATE EXACT LOCATION OF ALL ELECTRICAL EQUIPMENT AND DEVICES WITH THE ARCHITECTURAL, CIVIL, MECHANICAL, STRUCTURAL DRAWINGS AND THE OWNER PRIOR TO BEGINNING INSTALLATION.

E. ALL ELECTRICAL WORK SHALL BE DONE IN A NEAT AND WORKMANLIKE MANNER.

F. PROVIDE PHENOLIC NAMEPLATES ON ALL PANELBOARDS, SWITCHBOARDS, DISCONNECTS, TRANSFORMERS, ETC, INDICATING DESIGNATION, VOLTAGE AND WHERE UNIT IS FED FROM. SECURE NAMEPLATES WITH AT LEAST TWO SCREWS OR RIVETS. CEMENTING AND ADHESIVE INSTALLATION NOT ACCEPTABLE.

G. CONTRACTOR SHALL COORDINATE WITH UTILITY COMPANY AND TELECOMMUNICATION COMPANY FOR LOCATIONS AND REQUIREMENTS PRIOR TO COMMENCEMENT OF WORK. ELECTRICAL CONTRACTOR SHALL COMPLETE AND FILE UTILITY SERVICE INFORMATION FORMS AND BUILDING DEPARTMENT ELECTRICAL PERMIT FORMS.

REVISIONS

REV DRINKING FOUNTAIN & WIND SPEED

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CERT of AUTH # 26442
P.O. BOX 121721
CLERMONT
FL 34712
407-443-1853
ROBERT C. MUNROE
FL PE #33868

ITEM	QTY	DESCRIPTION
		BURKE BL-2 W/ BL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY

EXT FINISH:

INT FINISH:

MIX DESIGN: 5,000 PSI

RELEASE @ 3000 PSI

CONCRETE STRENGHT FOR POST TENSIONING: 4,500(CU. YARD:

POST TENSION FORCE: 28,000 PSI

CU. YARD:	SQ. FT:	WEIGHT:

W. DAVID ARNOLD
ARCHITECT
106 PORTLAND STREET
EUSTIS, FL 32726
(352) 357-5807 AR0013558

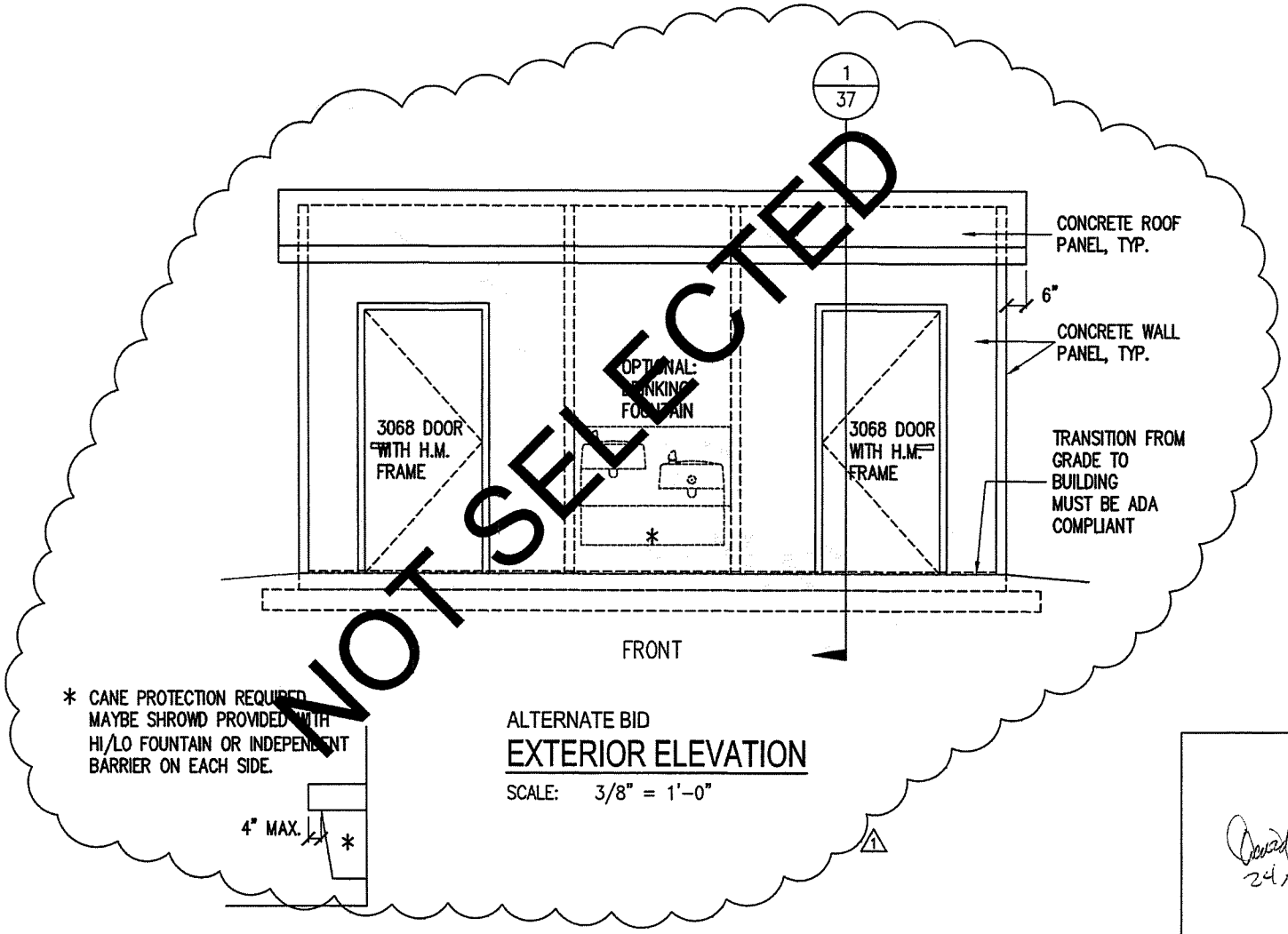
SCALE:		DATE:
AS SHOWN		10/6/16
DESIGN: WDA	DRAWN: JK/LP	CHECKED: WDA

CLIENT: LEESBURG CONCRETE

PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMETO BAY, FL 33157

JOB NO: 16-57 SHEET: 33 OF: 42

for ROBERT C. MUNROE P.E.



ALTERNATE BID

EXTERIOR ELEVATION

SCALE: 3/8" = 1'-0"

REVISIONS

1 DATE: 03/22/2017
CHANGED ALL WIND SPEED REFERENCES FROM 170 TO 175 & RECALCULATED ROOF & WALL PRESSURES

ITEM	QTY	DESCRIPTION
		SIMPS HL-2 W/ HL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		SIMPS SHIFT LFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONCRETE STRENGHT FOR POST TENSIONING: 4,500CCL YARD:
POST TENSION FORCE: 38,000 PSI

CU. YARD:	SQ. FT:	WEIGHT:

W. DAVID ARNOLD
ARCHITECT
106 PORTLAND STREET
EUSTIS, FL. 32726
(352) 357-5807 AR0013558

David Arnold
24 Apr 17

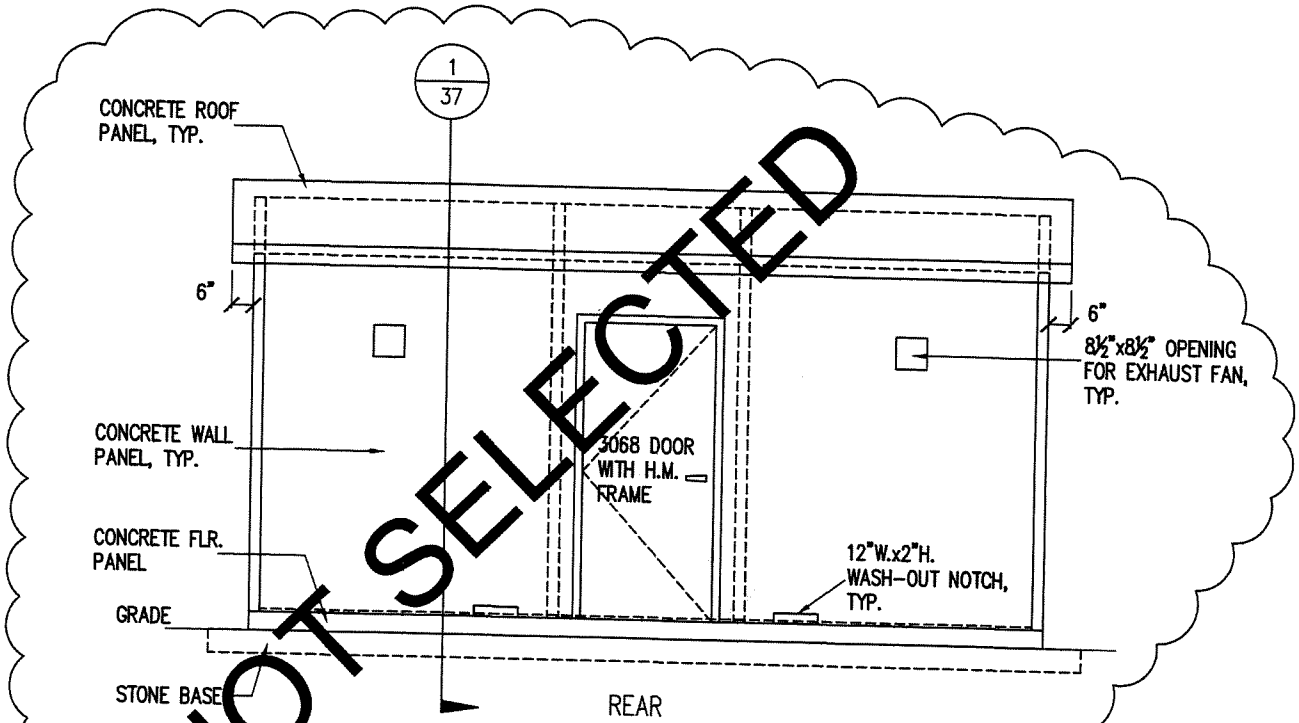
SCALE: AS SHOWN		DATE: 10/6/16
DESIGN: WDA	DRAWN: JK/LP	CHECKED: WDA

CLIENT: LEESBURG CONCRETE

PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL
17800 OLD CUTLER RD.
PALMENTO BAY, FL. 33157

JOB NO: 16-57

SHEET: 34 OF: 42



ALTERNATE BID

EXTERIOR ELEVATION

SCALE: 3/8" = 1'-0"

1

REVISIONS

1 DATE: 03/22/2017
CHANGED ALL WIND SPEED REFERENCES FROM 170 TO 175 & RECALCULATED ROOF & WALL PRESSURES

ITEM	QTY	DESCRIPTION
		BURGE BL-2 W/ BL-3B & SHEAR BAR
		PL1
		PL2
		PL3
		BURGE SHIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONCRETE STRENGHT FOR POST TENSIONING: 4,500CCL YARD:
POST TENSION FORCE: 30,000 PSI

CU. YARD:	SQ. FT:	WEIGHT:

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David Arnold 24 Apr 17

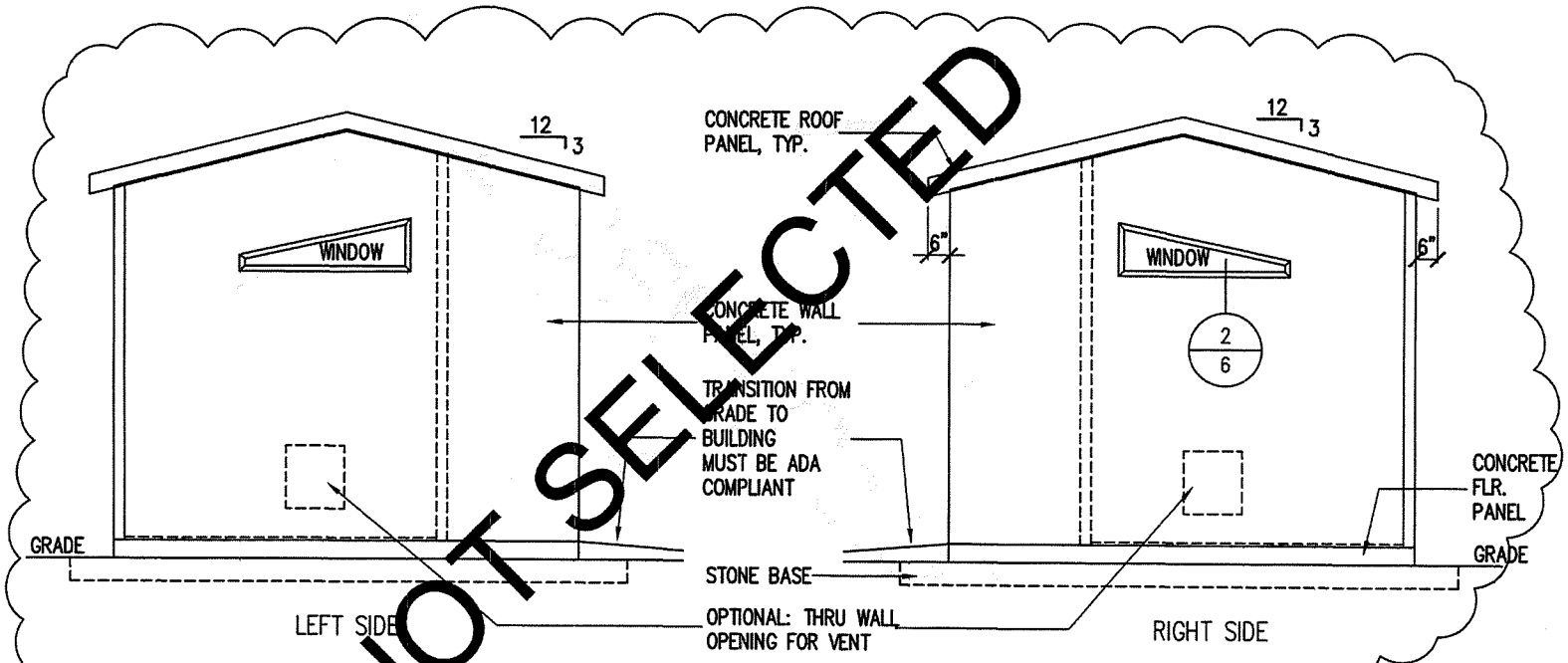
SCALE: AS SHOWN		DATE: 10/6/16
DESIGN: WDA	DRAWN: JK/LP	CHECKED: WDA

CLIENT: LEESBURG CONCRETE

PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL
17800 OLD CUTLER RD.
PALMETTO BAY, FL. 33157

JOB NO: 16-57

SHEET: 35 OF: 42



ALTERNATE BID

EXTERIOR ELEVATIONS

SCALE: 3/8" = 1'-0"

1

REVISIONS

1 DATE: 03/22/2017
CHANGED ALL WIND SPEED REFERENCES FROM 170 TO 175 & RECALCULATED ROOF & WALL PRESSURES

ITEM	QTY	DESCRIPTION
		BURNE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURNE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 8000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 6,000 PSI
RELEASE @ 3000 PSI
CONCRETE STRENGHT FOR POST TENSIONING: 4,500CCL YARD
POST TENSION FORCE: 38,000 PSI

CU. YARD:	SQ. FT:	WEIGHT:

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David Arnold
24 Apr 17

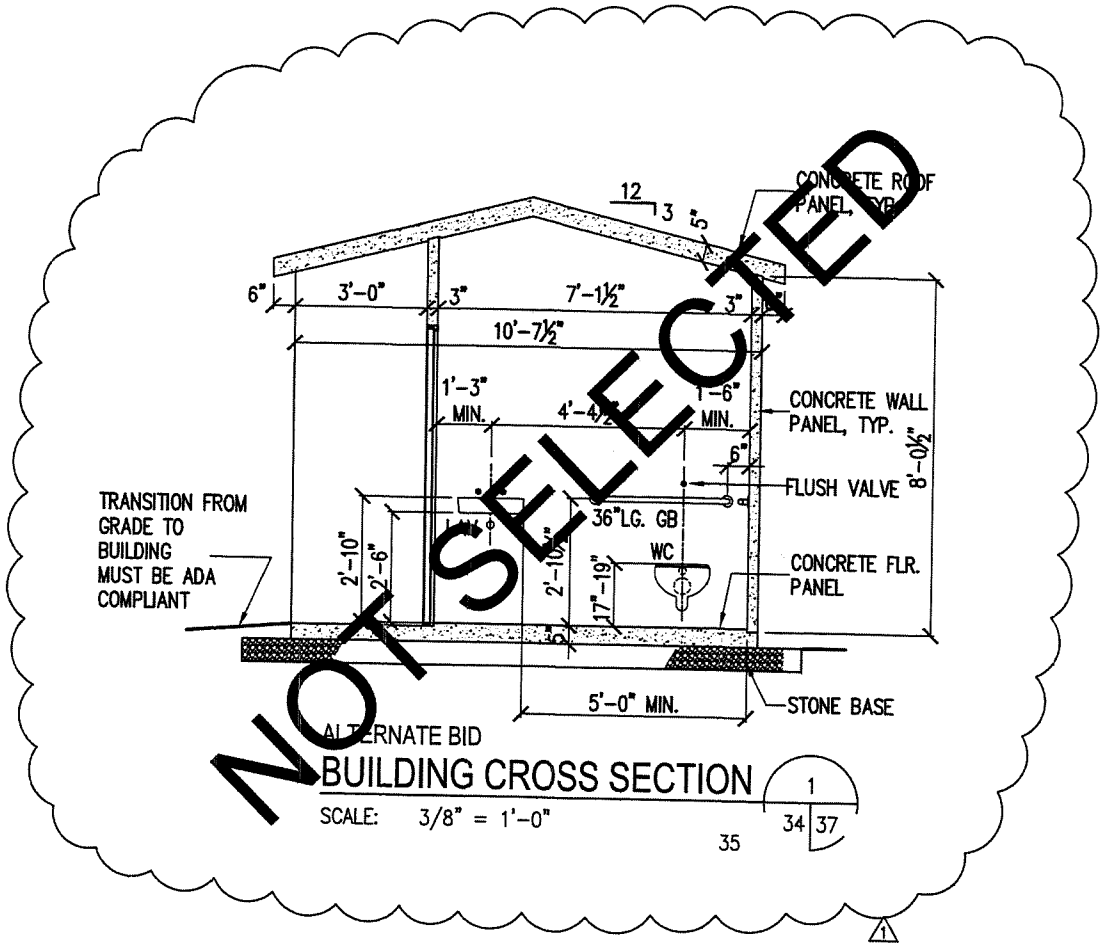
SCALE: AS SHOWN		DATE: 10/6/16
DESIGN: WDA	DRAWN: JK/LP	CHECKED: WDA

CLIENT: LEESBURG CONCRETE

PROJECT: OUTBACK SIERRA RESTROOM
for
MONTESSORI SCHOOL
17800 OLD CUTLER RD.
PALMETTO BAY, FL. 33157

JOB NO: 16-57

SHEET: 36 OF: 42



ALTERNATE BID

BUILDING CROSS SECTION

SCALE: 3/8" = 1'-0"

1 / 34 37

35

1

REVISIONS

1 DATE: 03/22/2017
CHANGED ALL WIND SPEED REFERENCES FROM 170 TO 175 & RECALCULATED ROOF & WALL PRESSURES

ITEM	QTY	DESCRIPTION
		BURNE BL-2 W/ BL-32 & SHEAR BAR
		PL3
		PL3
		PL3
		BURNE SHIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONCRETE SUBMITTAL FOR POST TENSIONING: 4,000CCL YARD
POST TENSION FORCE: 20,000 PSI

CU. YARD:	SQ. FT:	WEIGHT:

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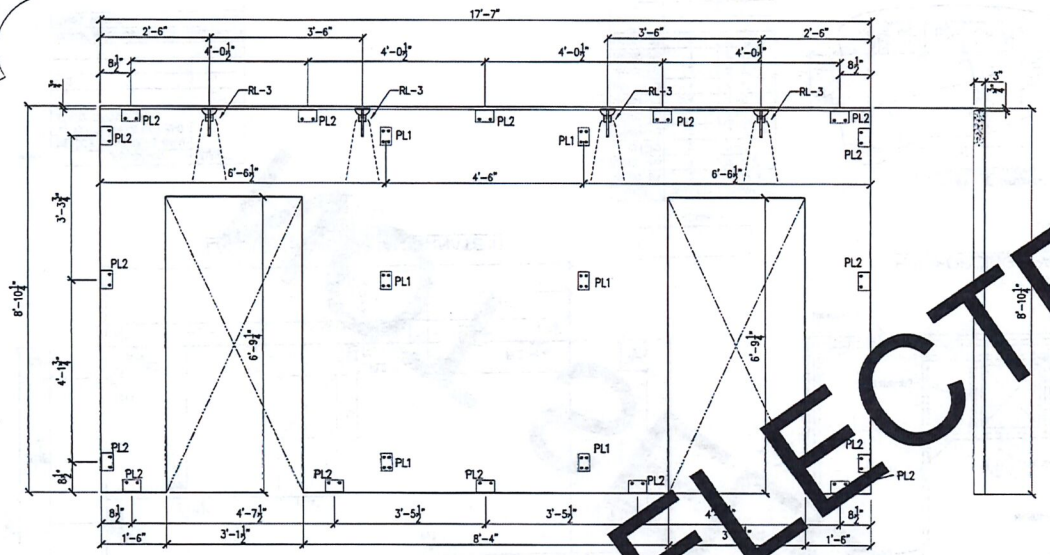
David Arnold
24 Apr 17

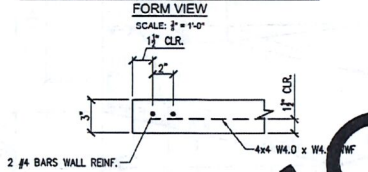
SCALE: AS SHOWN	DATE: 10/6/16
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 37 OF: 42



LAYOUT - PANEL P1 (ALTERNATE 1)

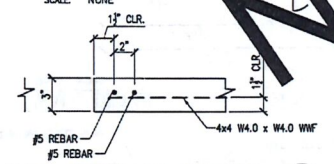
FORM VIEW

SCALE: ¾" = 1'-0"



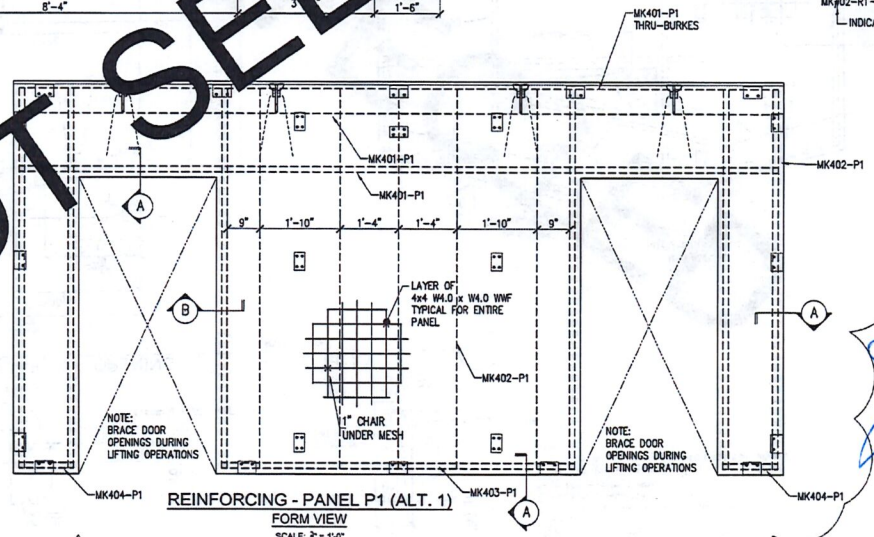
WALL DETAIL

SCALE: NONE



WALL DETAIL AT OPENING

SCALE: NONE



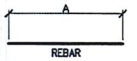
REINFORCING - PANEL P1 (ALT. 1)

FORM VIEW

SCALE: ¾" = 1'-0"

MARK	DESCRIPTION	QUANTITY
	MEADOW BURKE RL-3 GALV. w/ RL-31 SHEAR BAR AND RL-32 TENSION BAR	4
	PL1	6
	PL2	16
	CONCRETE VOLUME	1.04 CU YD
	PANEL WEIGHT	4,390 lb

Notes:
1. Orientation of plan is from the form view.
2. Trim plastic sheath at both ends of post tensioned cable to fit light between anchor plates.
3. Note clearance on drawing from edge of panel to outside of cable.
4. Reinforcing sequence:
Place chairs under mesh
Place mesh and install Burke lift
Install cable. Hold cable at vertical center of the panel
5. Hold reinforcing bars 1 1/2 inch clear from edge of forms.



REBAR

MARK	QUANTITY	LENGTH "A"
MK401-P1	4	17'-4"
MK402-P1	17	8'-8"
MK403-P1	2	8'-1"
MK404-P1	4	1'-3"

MK402-R1 — PANEL REF.
└ INDICATES REBAR SIZE (4=#4 ETC.)

NOT SELECTED

REVISIONS

1. Dated 4/20/2017
Labeled sheet "Not Selected".

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb

CU. YARD:	SQ. FT:	WEIGHT:

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W. DAVID ARNOLD
ARCHITECT

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EUSTIS, FL. 32726

(352) 357-5807 AR0013558

LINDEN C. PARCHMENT LICENSE No. 54071 STATE OF FLORIDA PROFESSIONAL ENGINEER

4/20/17

SCALE:	DATE:
AS SHOWN	4/20/2017

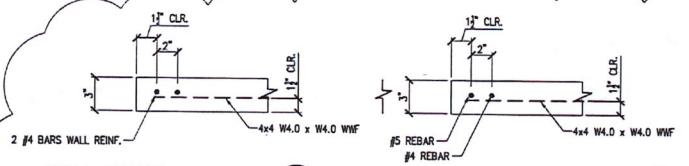
DESIGN:	DRAWN:	CHECKED:
WDA	JK/LP	WDA

CLIENT: LEESBURG CONCRETE

PROJECT: OUTBACK SIERRA RESTROOM
for
MONTESSORI SCHOOL
17800 OLD CUTLER RD.
PALMENTO BAY, FL. 33157

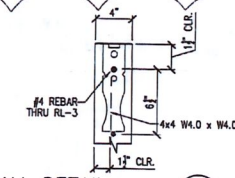
JOB NO: 16-57

SHEET: 38 OF: 42



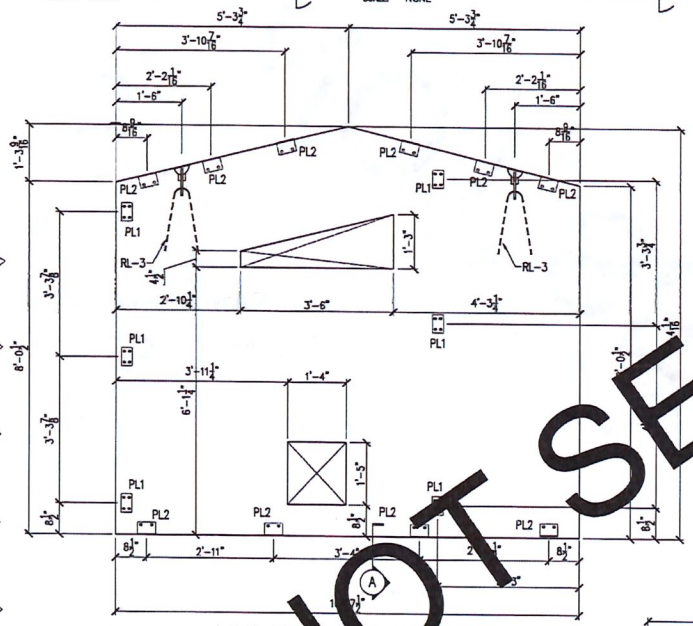
WALL DETAIL (A)
SCALE: NONE

WALL DETAIL AT OPENING (B)
SCALE: NONE

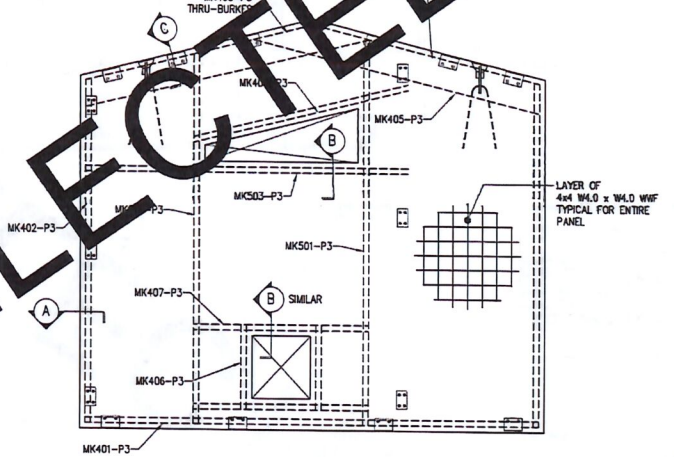


WALL DETAIL (C)
SCALE: NONE

Notes:
1. Orientation of plan is from the form view.
2. Reinforcing sequence:
Place chairs under mesh
Place mesh and install Burke lift
3. Hold reinforcing bars 1 1/2 inch clear from edge of forms.



LAYOUT - PANEL P3 (ALTERNATE 1)
FORM VIEW
SCALE: ¼" = 1'-0"



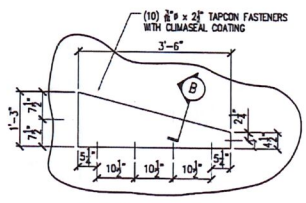
REINFORCING - PANEL P3 (ALTERNATE 1)
FORM VIEW
SCALE: ¼" = 1'-0"

MARK	DESCRIPTION	QUANTITY
	MEADOW BURKE RL-3 GALV. w/ RL-31 SHEAR BAR AND RL-32 TENSION BAR	2
	PL1	6
	PL2	9
	CONCRETE VOLUME	0.83 CU YD
	PANEL WEIGHT	3,380 lb

A
REBAR

MARK	QUANTITY	LENGTH "A"
MK401-P3	2	10'-4 ½"
MK402-P3	4	7'-9 ½"
MK403-P3	2	5'-4"
MK404-P3	2	5'-0"
MK405-P3	2	6'-7"
MK406-P3	4	2'-0"
MK407-P3	4	4'-0"
MK501-P3	2	8'-9"
MK502-P3	2	8'-5"
MK503-P3	2	7'-4"

MK402-P1 — PANEL REF.
INDICATES REBAR SIZE (4=#4 ETC.)



WINDOW DETAIL
SCALE: ½" = 1'-0"

NOT SELECTED

REVISIONS

1. Dated 4/20/2017
Labeled sheet "Not Selected".

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb
CU. YARD: | SQ. FT: | WEIGHT:

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(352) 357-5807 AR0013558

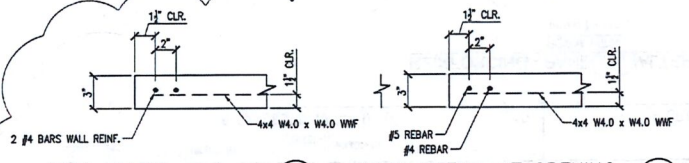
LINDEN C. PARCHMENT LICENSE No. 54071 STATE OF PROFESSIONAL ENGINEER

SCALE: AS SHOWN		DATE: 4/20/2017
DESIGN: WDA	DRAWN: JK/LP	CHECKED: WDA

CLIENT: LEESBURG CONCRETE

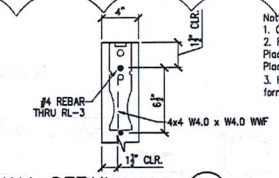
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL
17800 OLD CUTLER RD.
PALMENTO BAY, FL. 33157

JOB NO: 16-57 | SHEET: 39 | OF: 42



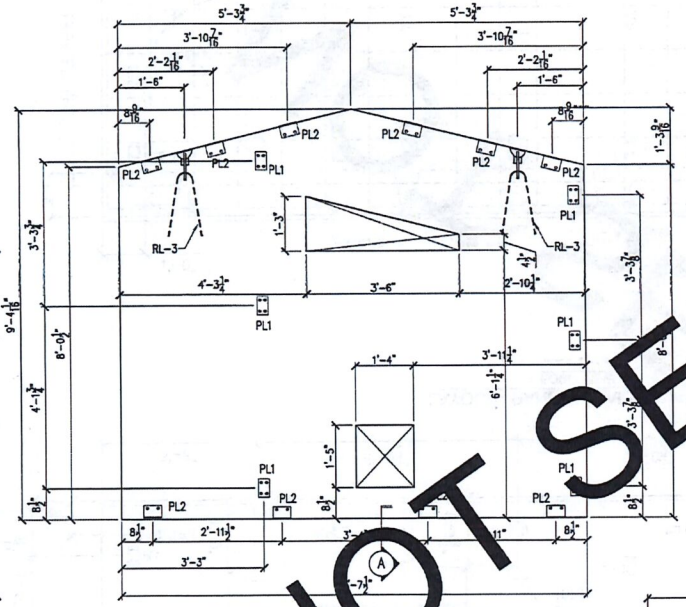
WALL DETAIL (A)
SCALE: NONE

WALL DETAIL AT OPENING (B)
SCALE: NONE

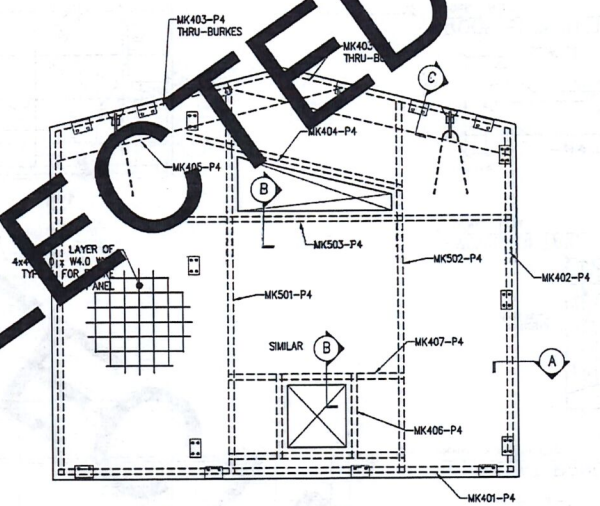


WALL DETAIL (C)
SCALE: NONE

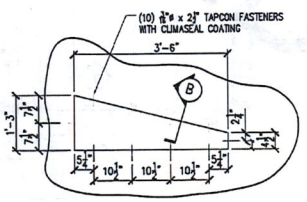
Notes:
1. Orientation of plan is from the form view.
2. Reinforcing sequence:
Place chairs under mesh
Place mesh and install Burke lift
3. Hold reinforcing bars 1 1/2 inch clear from edge of forms.



LAYOUT - PANEL P4 (ALTERNATE 1)
FORM VIEW
SCALE: ¾" = 1'-0"



REINFORCING - PANEL P4 (ALTERNATE 1)
FORM VIEW
SCALE: ¾" = 1'-0"



WINDOW DETAIL
SCALE: ¾" = 1'-0"

MARK	DESCRIPTION	QUANTITY
	MEADOW BURKE RL-3 GALV. w/ RL-31 SHEAR BAR AND RL-32 TENSION BAR	2
	PL1	6
	PL2	9
	CONCRETE VOLUME	0.83 CU YD
	PANEL WEIGHT	3,380 lb

REBAR

MARK	QUANTITY	LENGTH "A"
MK401-P4	2	10'-4 ¾"
MK402-P4	4	7'-9 ¾"
MK403-P4	2	5'-4"
MK404-P4	2	5'-0"
MK405-P4	2	6'-7"
MK406-P4	4	2'-0"
MK407-P4	4	4'-0"
MK501-P4	2	8'-9"
MK502-P4	2	8'-5"
MK503-P4	2	7'-4"

MK#02-P1 — PANEL REF.
INDICATES REBAR SIZE (4=#4 ETC.)

NOT SELECTED

REVISIONS

1. Dated 4/20/2017
Labeled sheet "Not Selected".

NOT SELECTED
BLDG DIV

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb
CU. YARD: SQ. FT: WEIGHT:

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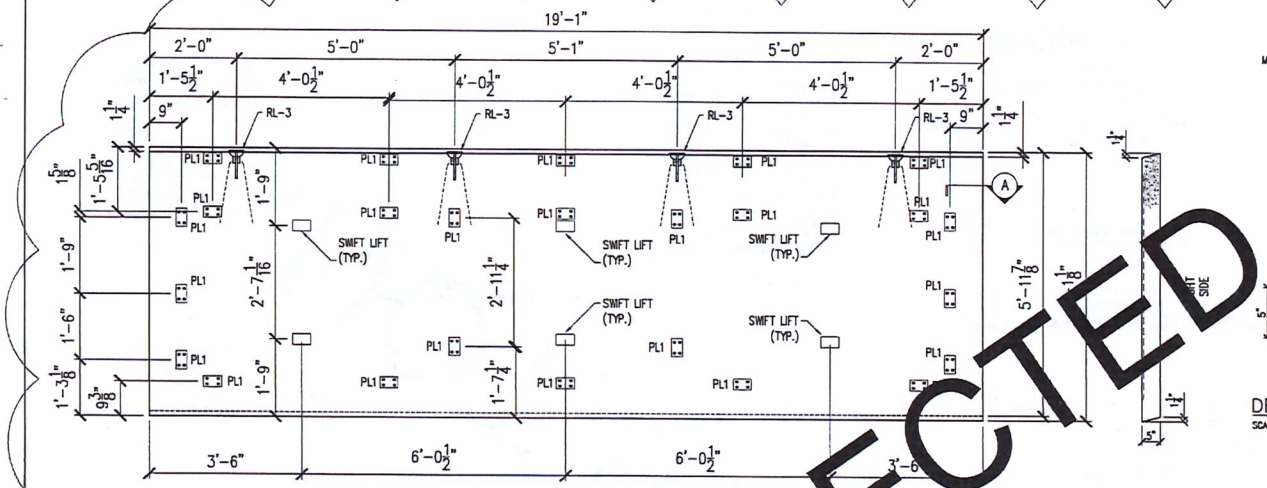
LINDEN C. PARCHMENT LICENSE No. 54071 STATE OF FLORIDA PROFESSIONAL ENGINEER

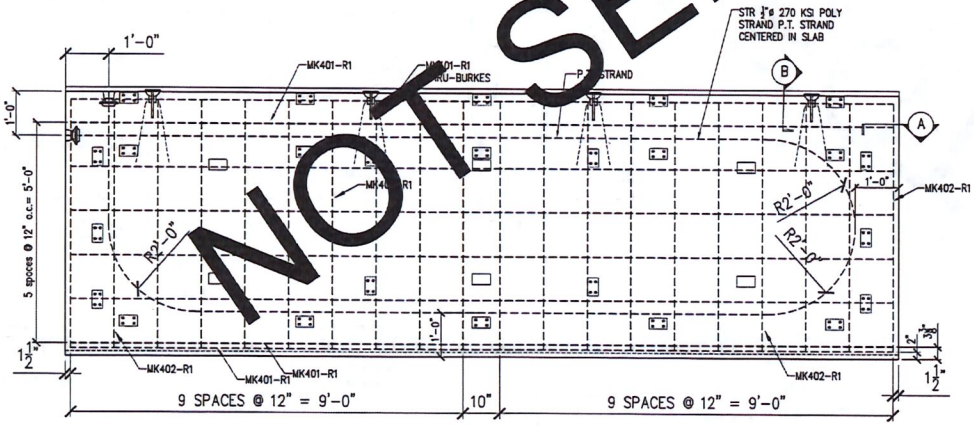
SCALE: AS SHOWN	DATE: 4/20/2017
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 40 OF: 42



LAYOUT - PANEL R1 (ALTERNATE 1)

FORM VIEW

SCALE: ¼" = 1'-0"

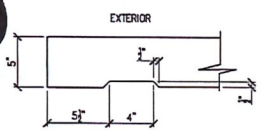


REINFORCING - PANEL R1 (ALTERNATE 1)

FORM VIEW

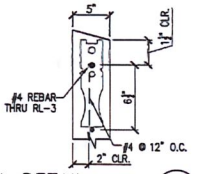
SCALE: ¼" = 1'-0"

MARK	DESCRIPTION	QUANTITY
	MEADOW BURKE RL-3 GALV. w/ RL-31 SHEAR BAR AND RL-32 TENSION BAR	4
	PL1	25
	PL2	
	PL3	
	SWIFT LIFT (2 TON)	6
	CONCRETE VOLUME	1.8 CU YD
	PANEL WEIGHT	7,550 lb



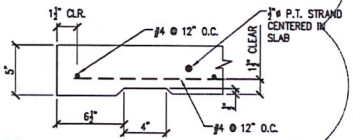
DETAIL AT PANEL EDGE (A)

SCALE: 1 1/2" = 1'-0"



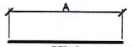
WALL DETAIL (B)

SCALE: NONE



ROOF EDGE DETAIL (C)

SCALE: NONE



MARK	QUANTITY	LENGTH "A"
MK401-R1	8	18'-10"
MK402-R1	20	5'-7¾"

MK#02-R1 — PANEL REF.

INDICATES REBAR SIZE (4=#4 ETC.)

POST TENSION CASTING ANCHOR AND WEDGE FOR 41'-7" LONG ½"ø POLY STRAND TENDON. POST TENSION FORCE = 28,000 lbs

NOT SELECTED

REVISIONS

1. Dated 4/20/2017
Labeled sheet "Not Selected"

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY

EXT FINISH:

INT FINISH:

MIX DESIGN: 5,000 PSI

RELEASE @ 3000 PSI

CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:

POST TENSION FORCE: 28,000 lb

CU. YARD: | SQ. FT: | WEIGHT:

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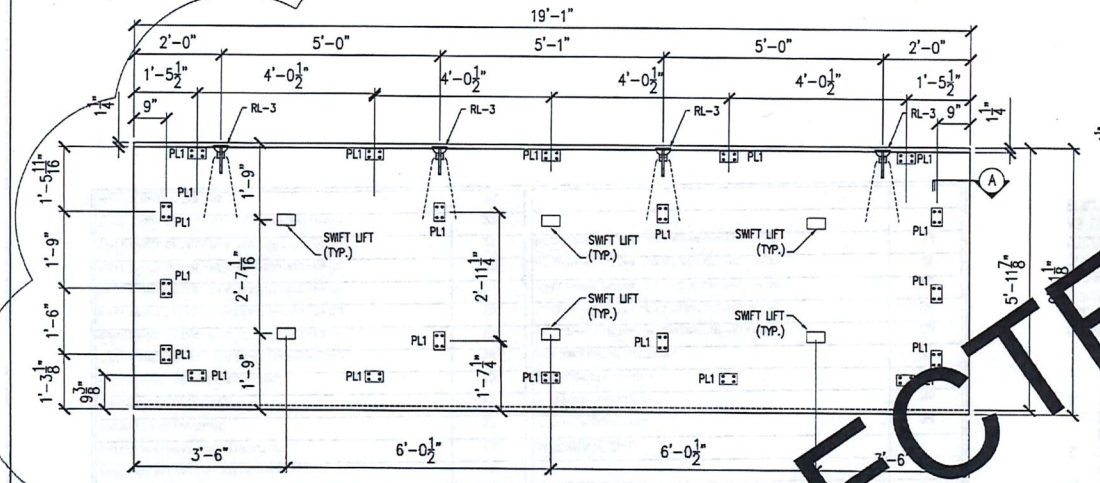
LINDEN C. PARCHMENT LICENSE No. 54071 STATE OF FLORIDA PROFESSIONAL ENGINEER

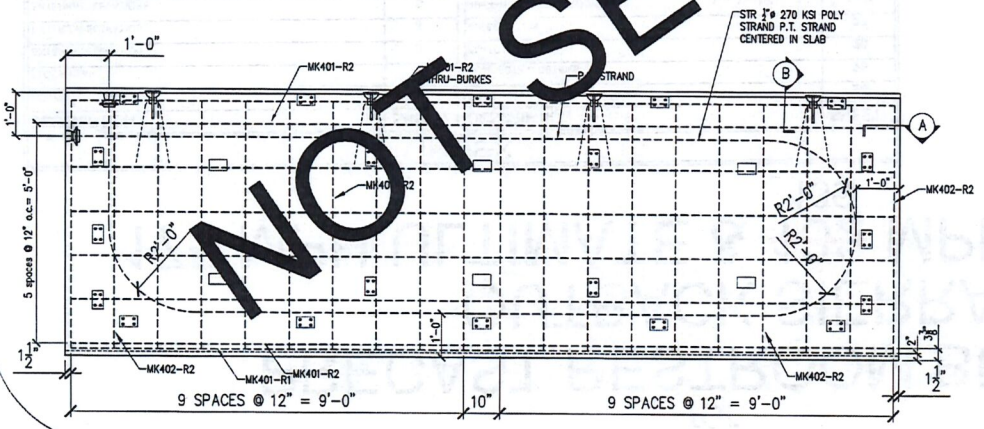
SCALE: AS SHOWN	DATE: 4/20/2017
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 41 OF: 42



LAYOUT - PANEL R2 (ALTERNATE 1)

FORM VIEW

SCALE: ¼" = 1'-0"

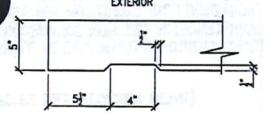


REINFORCING - PANEL R2 (ALTERNATE 1)

FORM VIEW

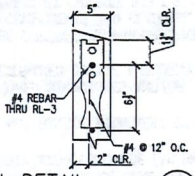
SCALE: ¼" = 1'-0"

MARK	DESCRIPTION	QUANTITY
	MEADOW BURKE RL-3 GALV. w/ RL-31 SHEAR BAR AND RL-32 TENSION BAR	4
	PL1	20
	PL2	
	PL3	
	SWIFT LIFT (2 TON)	6
	CONCRETE VOLUME	1.8 CU YD
	PANEL WEIGHT	7,524 lb



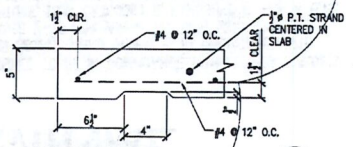
DETAIL AT PANEL EDGE (A)

SCALE: 1 1/2" = 1'-0"



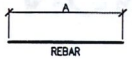
WALL DETAIL (B)

SCALE: NONE



ROOF EDGE DETAIL (C)

SCALE: NONE



MARK	QUANTITY	LENGTH "A"
MK401-R2	8	18'-10"
MK402-R2	20	5'-7½"

MK402-R1 — PANEL REF.

INDICATES REBAR SIZE (4=#4 ETC.)

POST TENSION CASTING ANCHOR AND WEDGE FOR 41'-7" LONG ½"ø POLY STRAND TENDON. POST TENSION FORCE = 28,000 lbs

NOT SELECTED

REVISIONS

1. Dated 4/20/2017
Labeled sheet "Not Selected".

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY

EXT FINISH:

INT FINISH:

MIX DESIGN: 5,000 PSI

RELEASE @ 3000 PSI

CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:

POST TENSION FORCE: 28,000 lb

CU. YARD: | SQ. FT: | WEIGHT:

Linden Parchment P.E.

8002 Glitter Court
Orlando, FL 32836

(407) 810-3322 P.E. # 54071

W. DAVID ARNOLD
ARCHITECT

106 PORTLAND STREET
EUSTIS, FL. 32726

(352) 357-5807 AR0013558

LINDEN C. PARCHMENT LICENSED No. 54071 STATE OF FLORIDA PROFESSIONAL ENGINEER

SCALE:	DATE:
AS SHOWN	4/20/2017

DESIGN:	DRAWN:	CHECKED:
WDA	JK/LP	WDA

CLIENT: LEESBURG CONCRETE

PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL. 33157

JOB NO: 16-57 | SHEET: 42 | OF: 42

CONSTRUCTION DOCUMENTS
for
PRECAST RESTROOM BUILDING
OUTBACK SIERRA
175 MPH ULTIMATE & ~~132~~ MPH NOMINAL
136

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NOTES:

1. BUILDING IS DESIGNED TO BE IN COMPLIANCE WITH FLORIDA BUILDING CODE 5th EDITION (2014). WIND DESIGN IS IN COMPLIANCE WITH SECTION 1609, ASCE 7-10 FOR Vult = 175 MPH WIND SPEED, ENCLOSED STRUCTURE, RISK CATEGORY II, EXPOSURE C, Gcp = 0.18. FLORIDA FIRE PROTECTION CODE 5th EDITION INCLUDING NFPA 1 (FIRE) & NFPA 101 (LIFE SAFETY) (2012) NFPA70 NEC (2011).
2. THESE PLANS COMPLY WITH FLORIDA ADMINISTRATIVE CODE RULE FAC 61G-20-3 (PRODUCT APPROVAL). RAISED SEAL SET PD PLANS ARE ON FILE IN THE THIRD PARTY AGENCY'S OFFICE (AS DIRECTED BY DBCA).
3. TOTAL BUILDING WEIGHT DOES NOT INCLUDE EQUIPMENT BY OTHERS OR FIXTURE WEIGHT.
4. REQUIRED SEPARATION FROM OTHER STRUCTURES AND/OR PROPERTY LINES SHALL BE DETERMINED BY THE AUTHORITY HAVING JURISDICTION.
5. WINDOWS AND DOORS SHALL BE CERTIFIED WITH COMPLIANCE WITH WIND DESIGN PRESSURES FOR COMPONENTS AND CLADDING.
6. PLAN REVIEW INSPECTION REQUIRED BY CHAPTER 633 F.S. SHALL BE DONE ON SITE BY LOCAL FIRE SAFETY INSPECTOR.
7. BUILDING IS NEITHER HEATED OR COOLED.
8. PRODUCT APPROVAL:
   DOORS: FL4553
   WINDOWS: PER FL REG PE (SEE ATTACHED DETAIL)

GENERAL NOTE:
ALL SITE RELATED WORK SHALL BE PERFORMED BY OTHERS UNDER SEPARATE PERMIT AND IN COMPLIANCE WITH THE AUTHORITY HAVING JURISDICTION (AHJ) INCLUDING, BUT NOT LIMITED TO, FOUNDATION, TIE DOWN, GRADING, SIDEWALKS, ACCESSIBLE ROUTE, BUILDING SETBACKS, UTILITY CONNECTIONS AND IMPACT PROTECTION OF DOORS AND WINDOWS AS MAY BE REQUIRED PER FBCB 1710.5 EXCEPTION 3. (NOT REQUIRED IF BUILDING IS LOCATED IN ZONE WHERE VULT IS LESS THAN 140 MPH).

David Arnold 23 Mar 17

LARRY C. PARCHMENT LICENSE No. 54071 STATE OF FLORIDA PROFESSIONAL ENGINEER

REVISIONS

1 DATE: 03/22/2017
CHANGED ALL WIND SPEED REFERENCES FROM 170 TO 175 & RECALCULATED ROOF & WALL PRESSURES

BLDG DIV

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SHIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

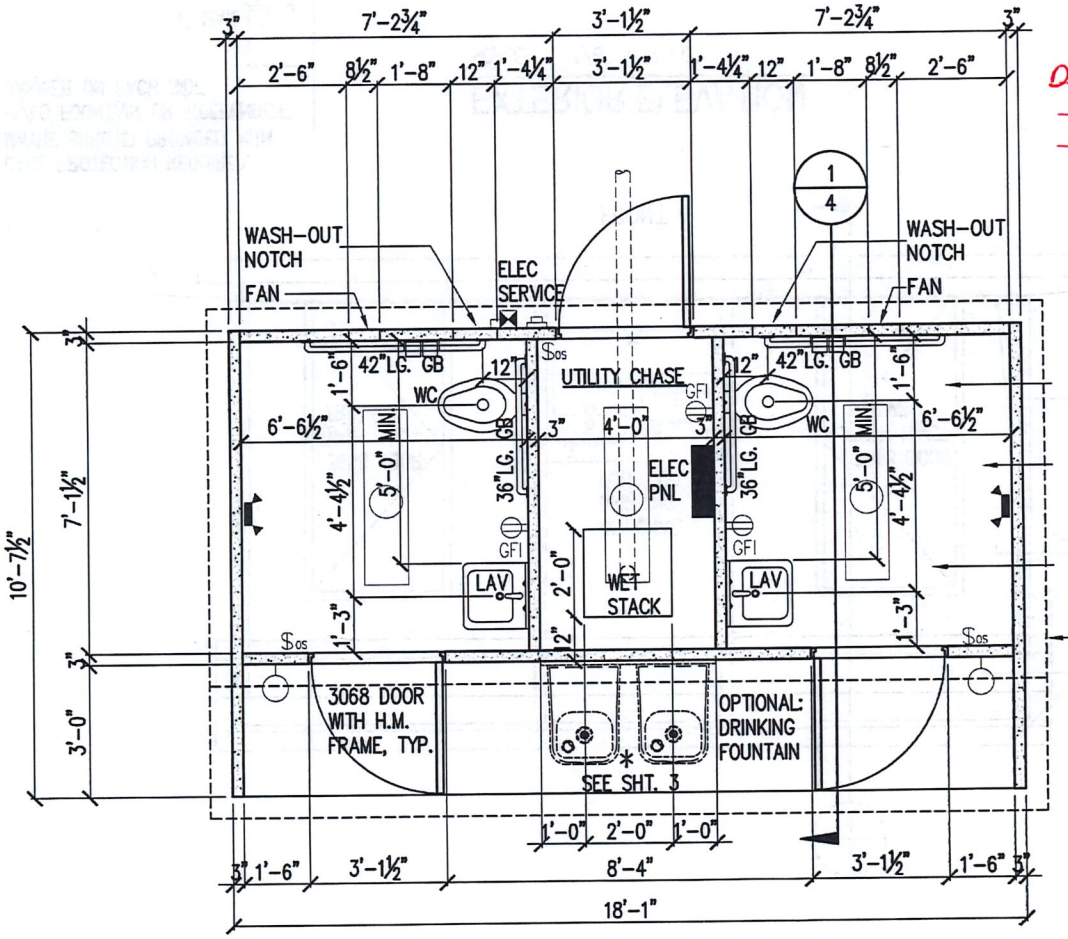
CONCRETE: 6000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONCRETE STRENGHT FOR POST TENSIONING: 4,000CUL YARD:
POST TENSION FORCE: 33,000 PSI
CU. YARD: | SQ. FT: | WEIGHT:

W. DAVID ARNOLD
ARCHITECT
106 PORTLAND STREET
EUSTIS, FL. 32726
(352) 357-5807 AR0013558

SCALE:		DATE:
AS SHOWN		10/6/16
DESIGN: WDA	DRAWN: JK/LP	CHECKED: WDA

CLIENT: LEESBURG CONCRETE
PROJECT: OUTBACK SIERRA RESTROOM
for
MONTESSORI SCHOOL
17800 OLD CUTLER RD.
PALMETTO BAY, FL. 33157

JOB NO: 16-57 SHEET: 1 OF: 42



FLOOR PLAN

SCALE: 3/8" = 1'-0"

THE FOLLOWING LIFE SAFETY FEATURES SHALL BE PROVIDED AT TIME OF OCCUPANCY:

DESIGNED PER FFPC. 5th EDITION.

APPROVED
MIAMI-DADE COUNTY FIRE DEPARTMENT
BLDG. PERMIT — FIRE ALARM
FIRE SUPPRESSION WATER MAIN EXT.
DATE: 4-27-17 BY:

REVISIONS

1 DATE: 03/22/2017
CHANGED ALL WIND SPEED REFERENCES FROM 170 TO 175 & RECALCULATED ROOF & WALL PRESSURES

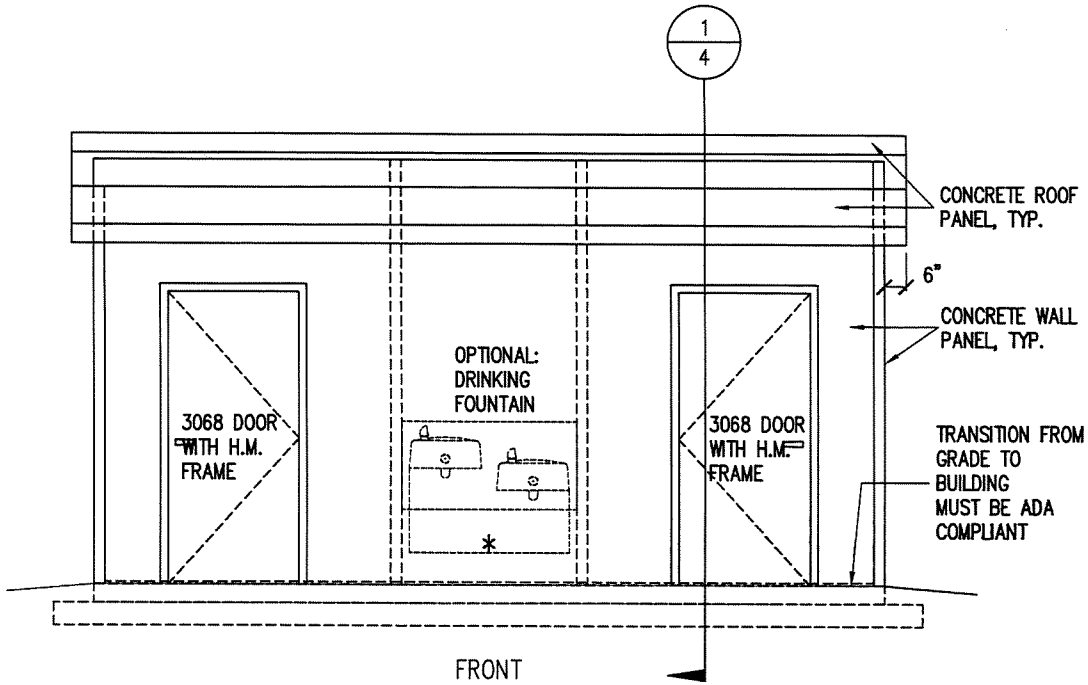
ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONCRETE STRENGHT FOR POST TENSIONING: 4,500CCIL YARD:
POST TENSION FORCE: 28,000 PSI
CU. YARD: SQ. FT: WEIGHT:

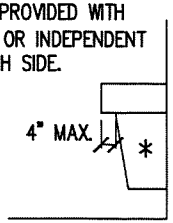
W. DAVID ARNOLD
ARCHITECT
106 PORTLAND STREET
EUSTIS, FL. 32726
(352) 357-5807 AR0013558

David Arnold 23 Mar 17

SCALE: AS SHOWN	DATE: 10/6/16
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMETTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 2 OF: 42



* CANE PROTECTION REQUIRED. MAYBE SHROWD PROVIDED WITH HI/LO FOUNTAIN OR INDEPENDENT BARRIER ON EACH SIDE.



EXTERIOR ELEVATION

SCALE: 3/8" = 1'-0"

REVISIONS

1 DATE: 03/22/2017
CHANGED ALL WIND SPEED REFERENCES FROM 170 TO 175 & RECALCULATED ROOF & WALL PRESSURES

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 8000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONCRETE STRENGHT FOR POST TENSIONING: 4,5000CU. YARD:
POST TENSION FORCE: 38,000 PSI
CU. YARD: | SQ. FT: | WEIGHT:

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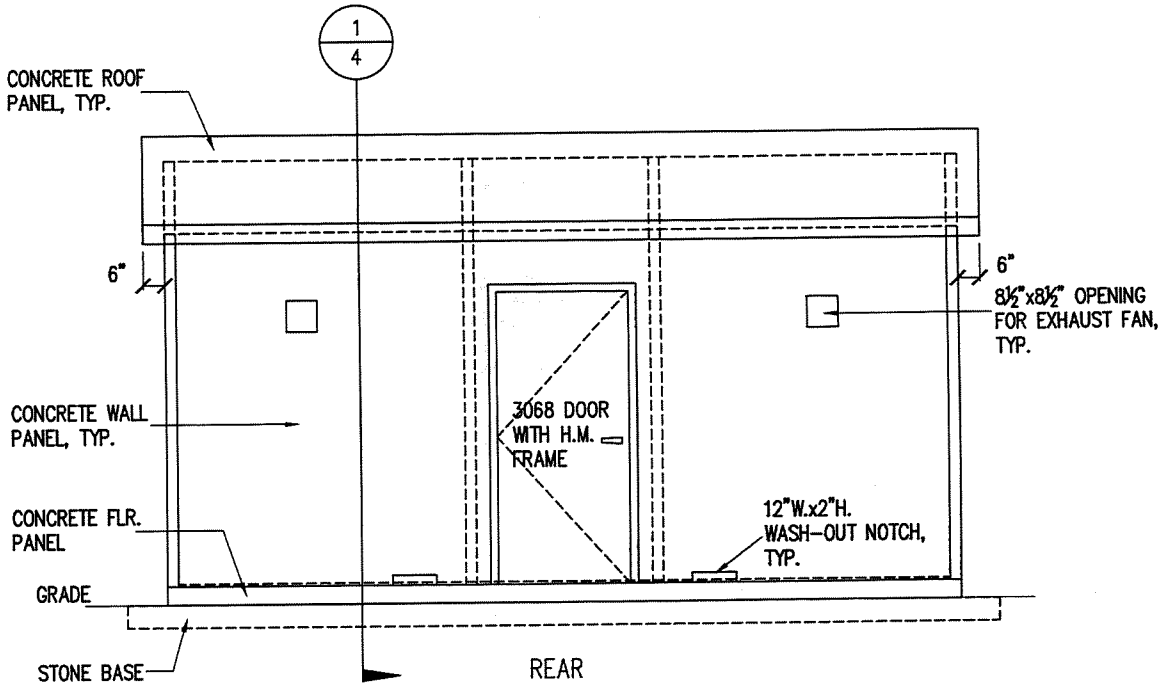
EUSTIS, FL 32726
(352) 357-5807 AR0013558

David Arnold 23 Mar 17

SCALE: AS SHOWN	DATE: 10/6/16
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL 33157	
JOB NO: 16-57	SHEET: 3 OF: 42



EXTERIOR ELEVATION

SCALE: 3/8" = 1'-0"

REVISIONS

1 DATE: 03/22/2017
CHANGED ALL WIND SPEED REFERENCES FROM 170 TO 175 & RECALCULATED ROOF & WALL PRESSURES

ITEM	QTY	DESCRIPTION
		BURNE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURNE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 8000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONCRETE STRENGHT FOR POST TENSIONING: 4,500CCL YARD:
POST TENSION FORCE: 28,000 PSI

CU. YARD:	SQ. FT:	WEIGHT:

W. DAVID ARNOLD
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David Arnold 23 Mar 17

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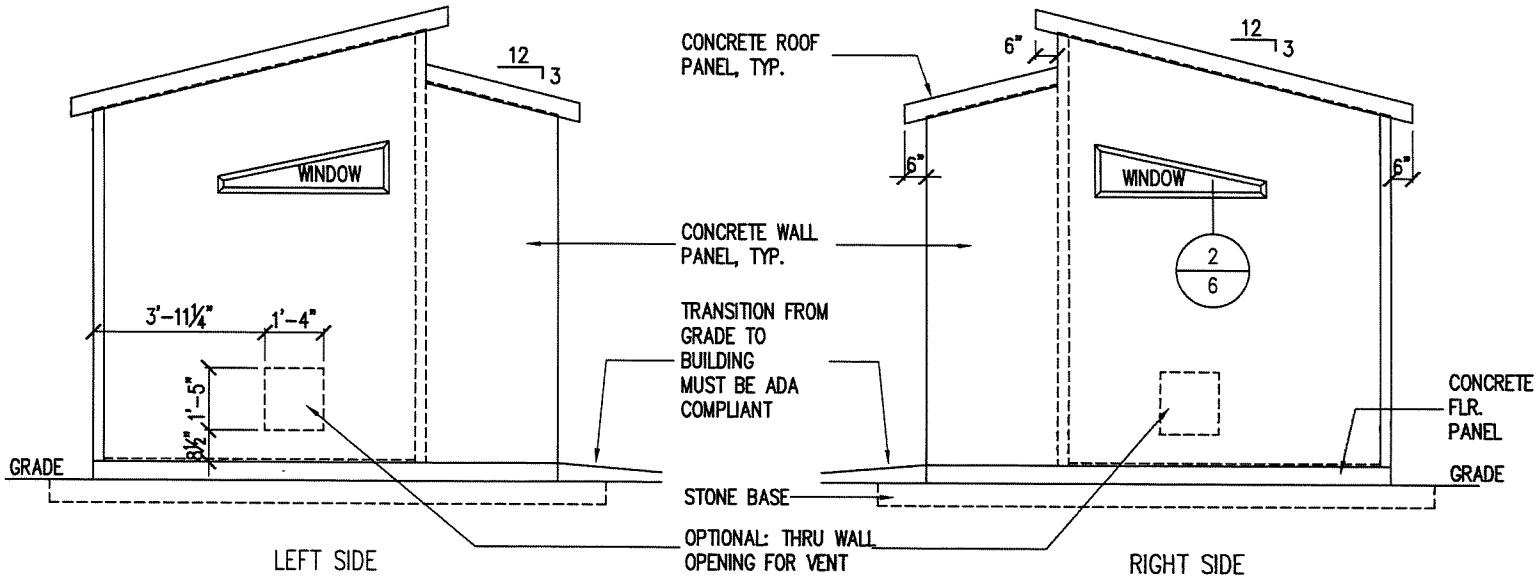
JOB NO: 16-57	SHEET: 4 OF: 42



EXTERIOR ELEVATIONS

SCALE: 3/8" = 1'-0"

REVISIONS

1 DATE: 03/22/2017
CHANGED ALL WIND SPEED REFERENCES FROM 170 TO 175 & RECALCULATED ROOF & WALL PRESSURES

ITEM	QTY	DESCRIPTION
		BURKE BL-2 W/ BL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 6000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE: @ 3000 PSI
CONCRETE STRENGHT FOR POST TENSIONING: 4,500CCL YARD:
POST TENSION FORCE: 36,000 PSI

CU. YARD:	SQ. FT:	WEIGHT:

W. DAVID ARNOLD
ARCHITECT
106 PORTLAND STREET
EUSTIS, FL. 32726
(352) 357-5807 AR0013558

SCALE: AS SHOWN	DATE: 10/6/16
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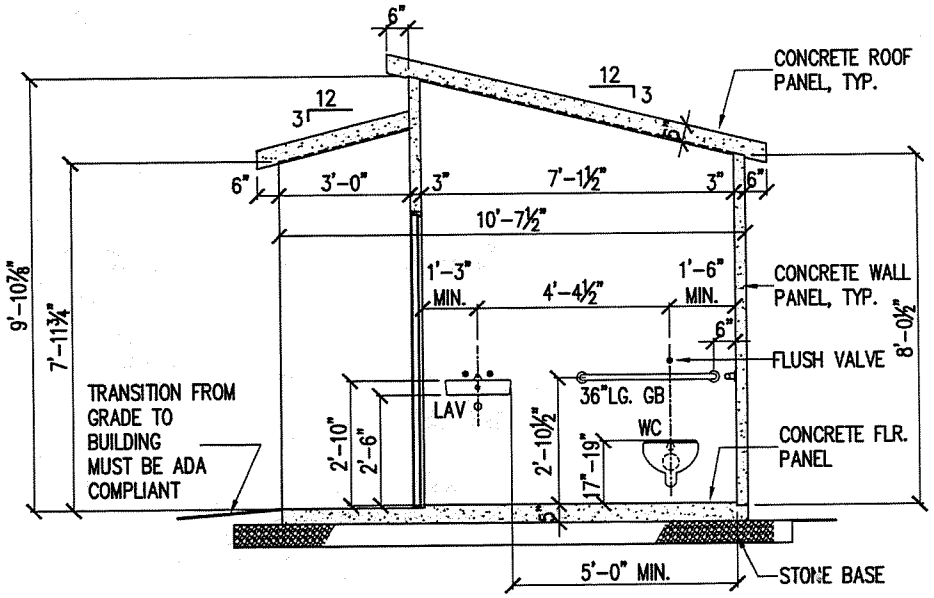
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 5 OF: 42

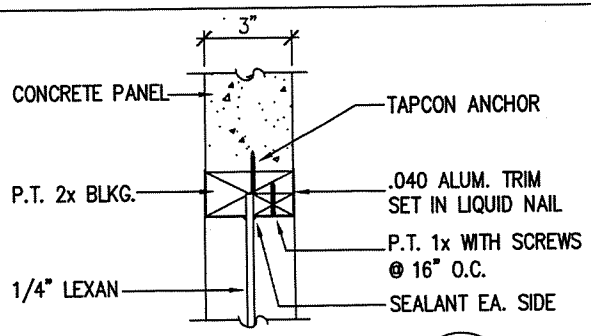
David Arnold 22 Mar 17



BUILDING CROSS SECTION

1 / 2 6

SCALE: 3/8" = 1'-0"



WINDOW DETAIL

2 / 5 6

SCALE: 3" = 1'-0"

REVISIONS

1 DATE: 03/22/2017
CHANGED ALL WIND SPEED REFERENCES FROM 170 TO 175 & RECALCULATED ROOF & WALL PRESSURES

ITEM	QTY	DESCRIPTION
		BURKE BL-2 W/ BL-32 & SHEAR BAR
		PL3
		PL2
		PL3
		BURKE SHIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONCRETE STRENGHT FOR POST TENSIONING: 4,500(CU. YARD:
POST TENSION FORCE: 28,000 PSI
CU. YARD: SQ. FT: WEIGHT:

W. DAVID ARNOLD
ARCHITECT
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EUSTIS, FL. 32726
(352) 357-5807 AR0013558

David Arnold 23 Mar 17

SCALE: AS SHOWN	DATE: 10/6/16
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMETTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 6 OF: 42

GENERAL NOTES:

1. ALL DETAILS, SECTIONS AND NOTES SHOWN ON DRAWINGS ARE INTENDED TO BE TYPICAL SHALL APPLY TO SIMILAR SITUATIONS ELSEWHERE UNLESS NOTED OR SHOWN OTHERWISE.
2. THE MANUFACTURER SHALL VERIFY ALL DIMENSIONS AND CONDITION BEFORE START OF WORK.
3. THE DETAILS ON THESE DRAWINGS SHALL BE USED WHEREVER APPLICABLE UNLESS NOTED OR SHOWN OTHERWISE ON THE DRAWINGS. NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS.
4. ALL WORKMANSHIP AND MATERIALS SHALL CONFORM THE THE REQUIREMENTS OF THE FLORIDA BUILDING CODE, (BUILDING, PLUMBING, MECHANICAL) AND THE NATIONAL ELECTRICAL CODE AS APPLICABLE.
5. ALL CONSTRUCTION OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER OF RECORD BEFORE PROCEEDING WITH ANY WORK INVOLVED.
6. ALL CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE 2014 FLORIDA BUILDING CODE AND NEC 2012.
7. MANUFACTURER SHALL BE RESPONSIBLE FOR SAFETY AND PROTECTION IN AND AROUND JOB SITE AND/OR ADJACENT PROPERTIES.
8. OBSERVATION VISITS TO THE SITE BY A FIELD REPRESENTATIVE OF THE ARCHITECT/ENGINEER OF RECORD SHALL NEITHER BE CONSTRUED AS INSPECTION NOR APPROVAL OF CONSTRUCTION UNLESS STATED OTHERWISE.
9. DURING AND AFTER CONSTRUCTION, MANUFACTURER AND/OR OWNER SHALL KEEP LOADS ON STRUCTURE WITHIN THE LIMITS OF DESIGN LOADS.
10. LIABILITY FOR MOVEMENT OF THIS STRUCTURE WILL BE LIMITED TO THE INITIAL SET AND LOCATION OF THIS STRUCTURE BY THE MANUFACTURER. NO OTHER WARRANTY OR LIABILITY IS EXPRESSED OR IMPLIED.
11. THIS STRUCTURE IS NOT DESIGNED TO BE MOVED WITH LIVE LOADS.
12. DURING THE CONSTRUCTION PERIOD THE MANUFACTURER SHALL BE RESPONSIBLE FOR THE SAFETY OF THE BUILDING. THE CONTRACTOR SHALL PROVIDE ADEQUATE SHORING AND BRACING IN ACCORDANCE WITH ALL NATIONAL STATE, AND LOCAL SAFETY ORDINANCES.
13. ALL CONSTRUCTION PROCEDURE SHALL CONFORM TO OSHA STANDARDS. ANY DEVIATION MUST BE APPROVED BY OSHA PRIOR TO.
14. PROVIDE SUPPORT FOR EQUIPMENT AS PER MANUFACTURES RECOMMENDATIONS.
15. ALL ELECTRICAL CONSTRUCTION BY OTHERS, MUST BE PERFORMED BY A LICENSED ELECTRICAL CONTRACTOR.
16. ALL PLUMBING CONSTRUCTION BY OTHERS, MUST BE PERFORMED BY LICENSED PLUMBING CONTRACTOR.
17. BUILDING IS DESIGNED TO BE IN COMPLIANCE WITH FLORIDA BUILDING CODE 2014. WIND DESIGN IS IN COMPLIANCE WITH SECTION 1609, ASCE 7-10 FOR VULT = 175 MPH ULTIMATE & 132 MPH NOMINAL WIND SPEED, ENCLOSED STRUCTURE, RISK CATEGORY II, EXPOSURE C, GCP = 0.18.±
18. <u>LOADS</u>

    ROOF LIVE LOAD = 20 PSF
    FLOOR LIVE LOAD = 100 PSF
    ROOF DEAD LOAD = 63 PSF
    FLOOR DEAD LOAD = 63 PSF
    WALL DEAD LOAD = 38 PSF

    <u>WIND LOADS FOR COMPONENTS AND CLADDING</u>

    ROOF ZONES
    1. +28.3 PSF, -45 PSF
    2. +28.3 PSF. -78 PSF
    3. +28.3 PSF, -115.5 PSF

    WALL ZONES
    WINDOW (LEXAN) +49 PSF, -53 PSF
    DOOR +47 PSF, -61 PSF

19. 4x4-W4xW4 WELDED WIRE REINFORCEMENT MAY BE SUBSTITUTED FOR #3 @ 12" O.C.
20. BUILDING OCCUPANCY: OCCUPANCY GROUP IS U (UTILITY AND MISCELLANEOUS) PLUS CONFORMING TO SECTION 428. CONSTRUCTION TYPE IS II-B.
21. FIRE RATING: WALLS: 45 MINUTE
    ROOF: 1.5 HR
    NO OPENING PROTECTION PROVIDED

David Arnold
23 Mar 17

JOHN C. PARCHEM
LICENSE
No. 54071
STATE OF FLORIDA
PROFESSIONAL ENGINEER

REVISIONS

1 DATE: 03/22/2017
CHANGED ALL WIND SPEED REFERENCES FROM 170 TO 175 & RECALCULATED ROOF & WALL PRESSURES

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONCRETE STRENGHT FOR POST TENSIONING: 4,500 CU. YARD:
POST TENSION FORCE: 28,000 PSI

CU. YARD:	SQ. FT:	WEIGHT:

W. DAVID ARNOLD
ARCHITECT
106 PORTLAND STREET
EUSTIS, FL. 32726
(352) 357-5807 AR0013558

SCALE:		DATE:
AS SHOWN		10/6/16
DESIGN: WDA	DRAWN: JK/LP	CHECKED: WDA

CLIENT: LEESBURG CONCRETE

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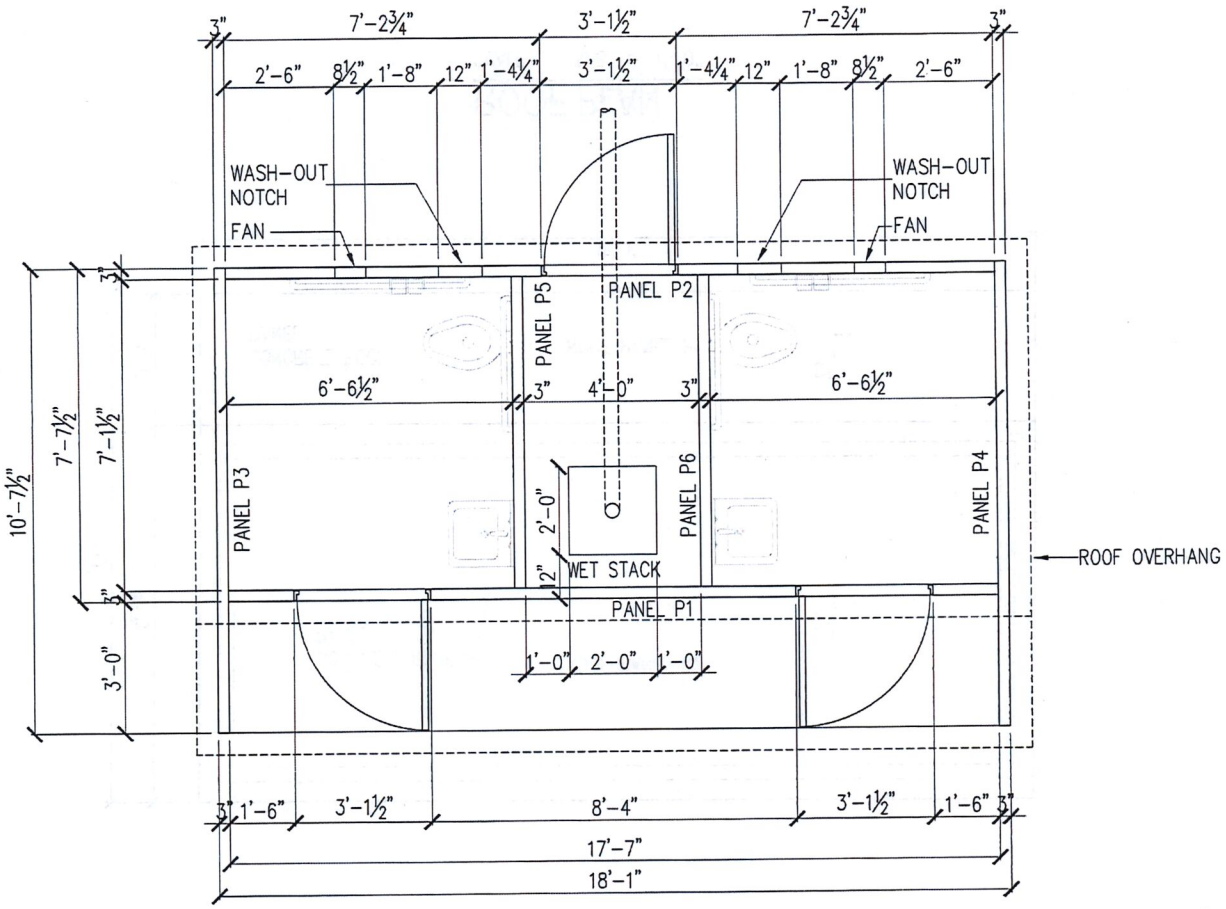
PALMETO BAY, FL. 33157

JOB NO: 16-57



FLOOR PLAN

SCALE: 3/8" = 1'-0"

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb
CU. YARD: SQ. FT: WEIGHT:

Linden Parchment P.E.

8002 Glitter Court
Orlando, FL 32836

(407) 810-3322 P.E. # 54071

W. DAVID ARNOLD
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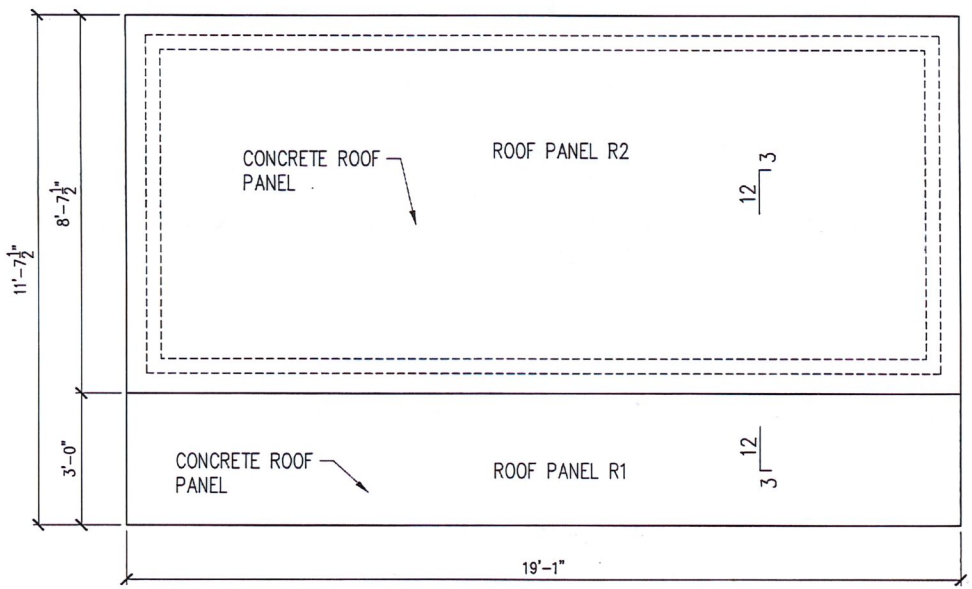
106 PORTLAND STREET
EUSTIS, FL 32726

(352) 357-5807 AR0013558

LINDEN C. PARCHMENT LICENSE No. 54071 STATE OF FLORIDA PROFESSIONAL ENGINEER

3/22/17

SCALE: AS SHOWN	DATE: 3/22/2017
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 8 OF: 42



ROOF PLAN

SCALE: 3/8" = 1'-0"

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb
CU. YARD: SQ. FT: WEIGHT:

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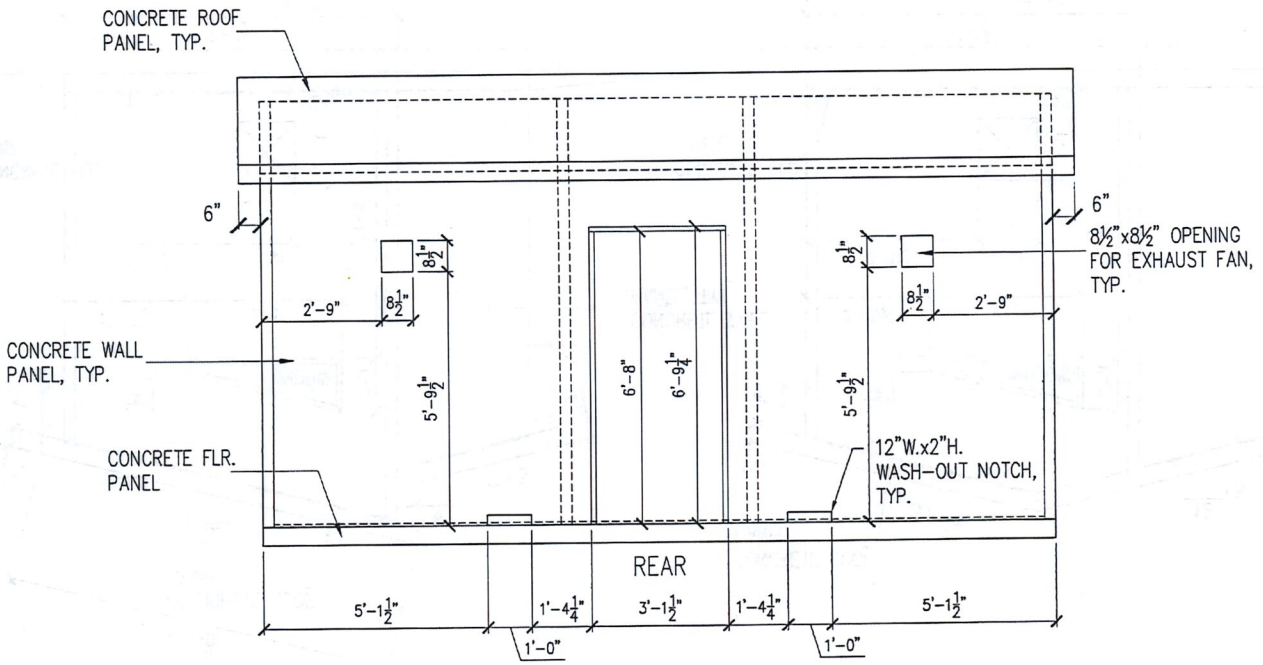
LINDEN C. PARCHMENT LICENSE No. 54071 STATE OF FLORIDA PROFESSIONAL ENGINEER

SCALE: AS SHOWN	DATE: 3/22/2017
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 9 OF: 42



EXTERIOR ELEVATION

SCALE: 3/8" = 1'-0"

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb
CU. YARD: SQ. FT: WEIGHT:

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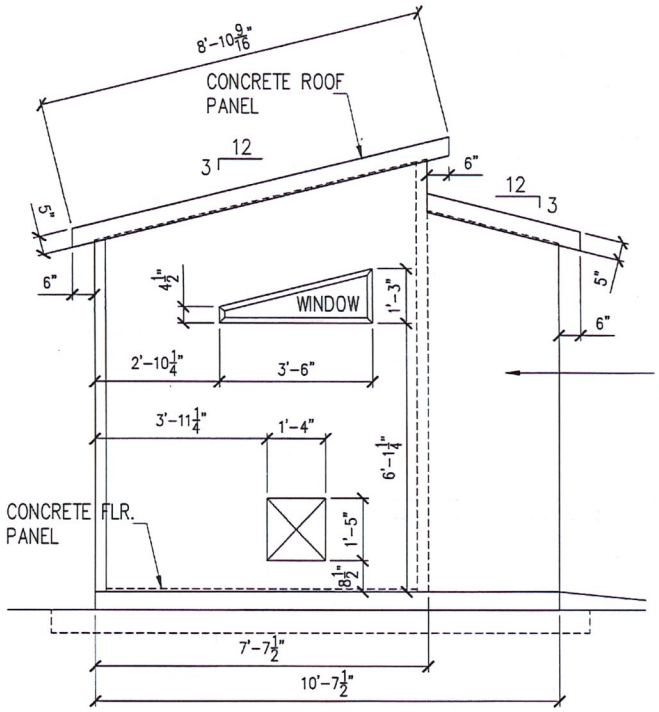
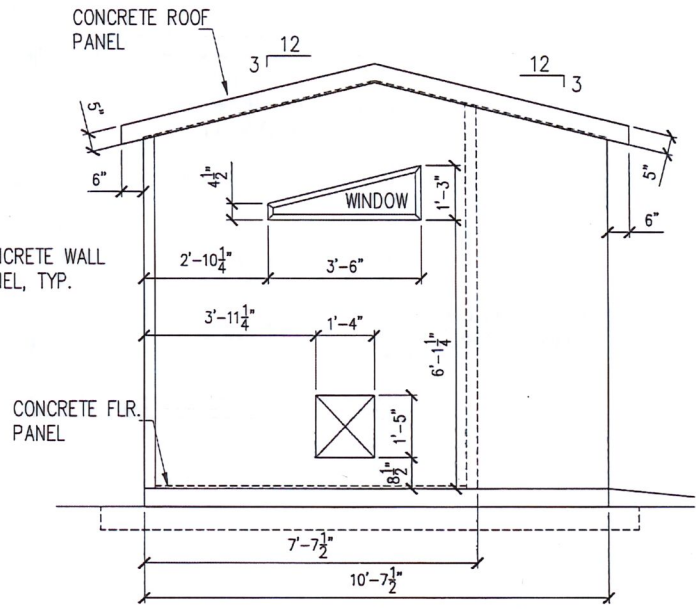
SCALE: AS SHOWN	DATE: 3/22/2017
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMETTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 11 OF: 42



LEFT SIDE



LEFT SIDE (ALTERNATE 1)

EXTERIOR ELEVATIONS

SCALE: 3/8" = 1'-0"

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb
CU. YARD: SQ. FT: WEIGHT:

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DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 12 OF: 42

FOUNDATION REQUIREMENTS

BUILDING DESIGN
THE FLOATING FOUNDATION OF CRUSHED STONE OR SAND SHOULD BE PLACED ON A WELL-DRAINED AND GRADED AREA, TO PRECLUDE THE RETENTION OF STANDING WATER.

SITE PREPARATION REQUIREMENTS
(MANUFACTURER'S RECOMMENDATION)

A. BUILDING SHALL BEAR FULLY ON A CRUSHED STONE BASE THAT IS AT LEAST TWO FEET LARGER THAN THE LENGTH AND WIDTH OF BUILDING.

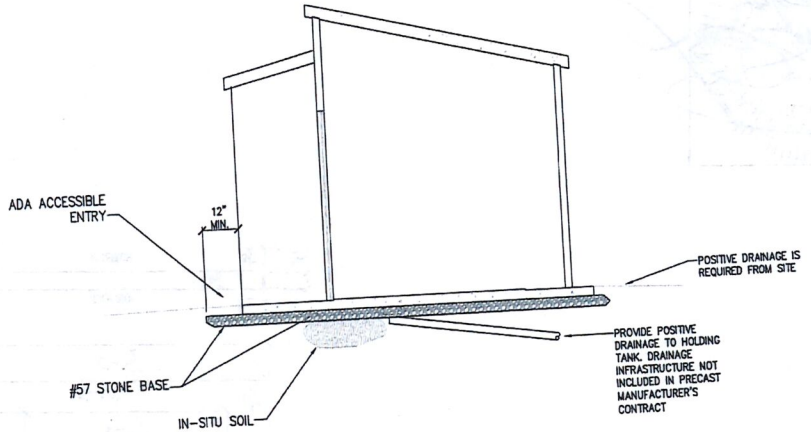
B. STONE SHALL BE A MINIMUM OF 6" THICK OR DOWN TO FIRM SUBGRADE. THE VERTICAL SOIL CAPACITY UNDER STONE SHALL BE COMPACTED TO HAVE MINIMUM BEARING OF 1,500 POUNDS PER SQUARE FOOT. STONE SHALL BE #57 STONE GRADE OR 3/8" MAXIMUM IN SIZE AND MUST BE SCREEDED LEVEL WITHIN ¼" IN BOTH DIRECTIONS. STONE SHALL BE PLACED WITHIN A PERIMETER FORM WITH FLAT AND LEVEL TOP EDGE FOR SCREEDING. FORMING MATERIAL SHALL REMAIN AROUND STONE UNTIL AFTER THE BUILDING IS SET.

C. THE CRUSHED STONE BASE SHALL BE KEPT WITHIN THE CONFINES OF THE SOIL OR PERIMETER FORM. DO NOT ALLOW THE BASE TO BECOME UNCONFINED SO THAT IT MAY WASH, ERODE, OR OTHERWISE BE UNDERMINED.
OR
SUBSTRATE BELOW PAVEMENT OR SLAB MUST HAVE A VERTICAL SOIL CAPACITY OF 1,500 POUNDS PER SQUARE FOOT. PLACE STONE OR SAND TO 1" ABOVE HIGHEST POINT OF AREA WHERE BUILDING WILL BE PLACED AND AT LEAST 1'-0" WIDE ALL AROUND THE BUILDING FOOTPRINT. RETAIN STONE OR SAND WITH A PERIMETER FORM TO PREVENT THE MATERIAL FROM WASHING OUT.

D. PROVIDE POSITIVE DRAINAGE FOR THE FILL AS REQUIRED.
E. ACCESS INTO THE BUILDING SHALL BE ADA COMPLIANT.

ACCESS

A. CONTRACTOR MUST PROVIDE LEVEL UNOBSTRUCTED AREA LARGE ENOUGH FOR A CRANE AND A TRACTOR-TRAILER TO PARK ADJACENT TO THE PAD. CRANE MUST BE ABLE TO PLACE OUTRIGGERS WITHIN 5'-0" OF EDGE OF PAD AND TRUCK AND CRANE MUST BE ABLE TO GET SIDE BY SIDE UNDER THEIR OWN POWER. NO OVERHEAD LINES MAY BE WITHIN 75' RADIUS OF CENTER OF PAD. A MINIMUM OF 24" CLEARANCE IS REQUIRED BETWEEN THIS BUILDING AND ADJACENT BUILDINGS.



FOUNDATION DETAIL

SCALE: NONE

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb
CU. YARD: SQ. FT: WEIGHT:

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3/22/17

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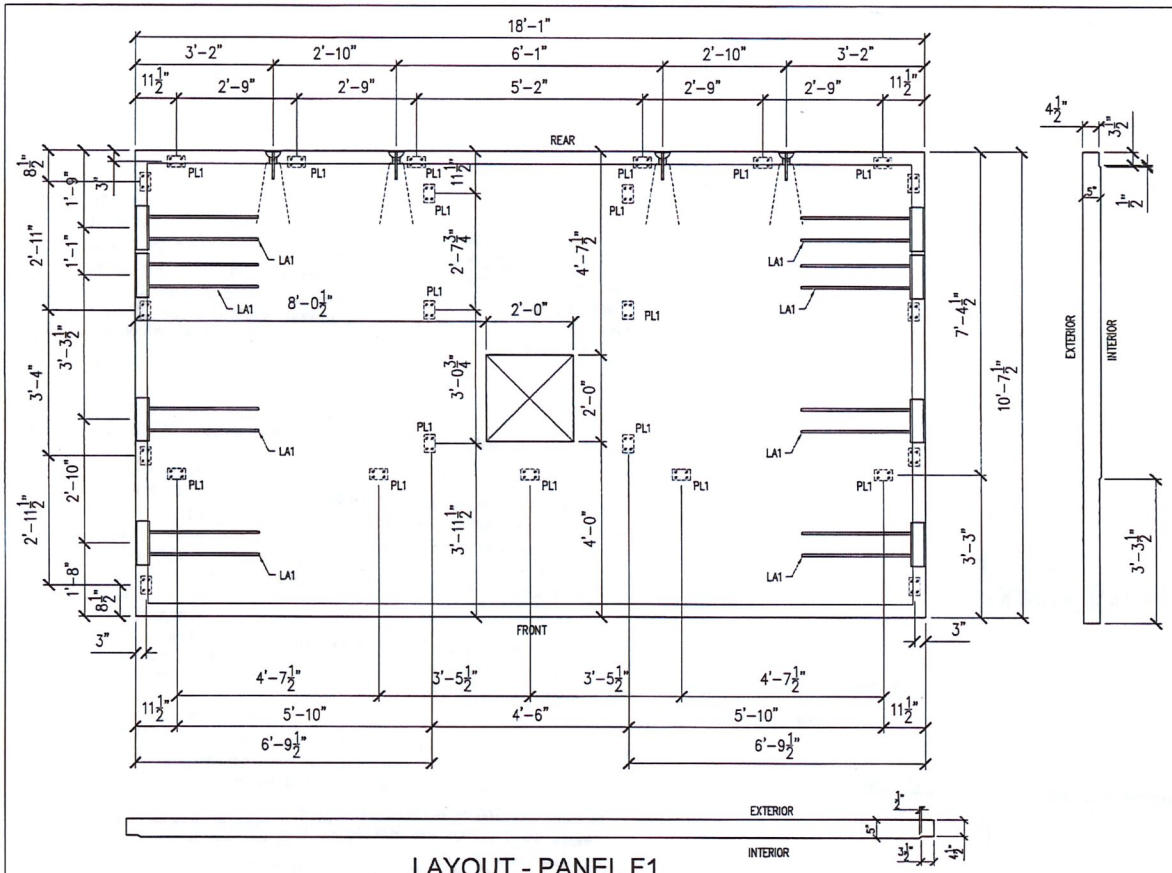
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMETO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 13 OF: 42



LAYOUT - PANEL F1

FORM VIEW

SCALE: ⅜" = 1'-0"

MARK	DESCRIPTION	QUANTITY
	MEADOW BURKE RL-3 GALV. w/ RL-31 SHEAR BAR AND RL-32 TENSION BAR	4
	PL1	25
	PL2	
	PL3	
	SWIFT LIFT (2 TON)	
	LIFT ANCHOR	8
	CONCRETE VOLUME	3 CU YD
	PANEL WEIGHT	12,000 lb

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY

EXT FINISH:

INT FINISH:

MIX DESIGN: 5,000 PSI

RELEASE @ 3000 PSI

CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:

POST TENSION FORCE: 28,000 lb

CU. YARD: | SQ. FT: | WEIGHT:

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3/22/17

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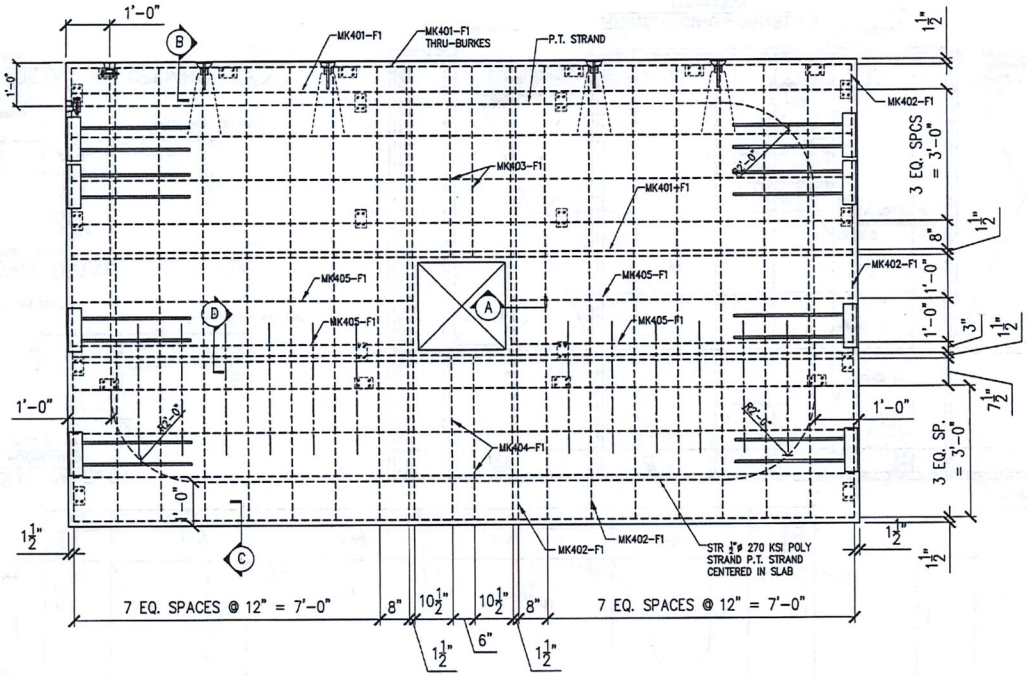
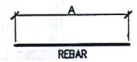
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMETO BAY, FL 33157	
JOB NO: 16-57	SHEET: 14 OF: 42



REINFORCING - PANEL F1

FORM VIEW

SCALE: 3/8" = 1'-0"



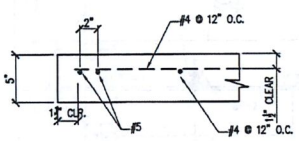
REBAR

MARK	QUANTITY	LENGTH "A"
MK301-F1	12	3'-0"
MK401-F1	13	17'-10"
MK402-F1	20	10'-4 1/2"
MK403-F1	2	4'-4 1/2"
MK404-F1	2	3'-9"
MK405-F1	4	7'-9 1/2"

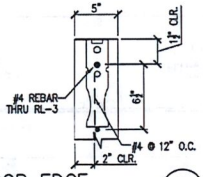
MK402-R1 — PANEL REF.
└ INDICATES REBAR SIZE (4=#4 ETC.)

POST TENSION CASTING ANCHOR AND WEDGE FOR 48'-10" LONG 1/4"ø POLY STRAND TENDON. POST TENSION FORCE = 28,000 lbs

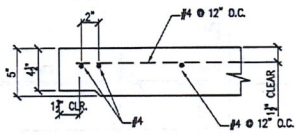
Notes:
1. Orientation of plan is from the form view.
2. Trim plastic sheath at both ends of post tensioned cable to fit tight between anchor plates.
3. Note clearance on drawing from edge of panel to outside of cable.
4. Reinforcing sequence:
Place chairs under mesh
Place mesh and install Burke lift
Install cable. Hold cable at vertical center of the panel
5. Hold reinforcing bars 1 1/2 inch clear from edge of forms.



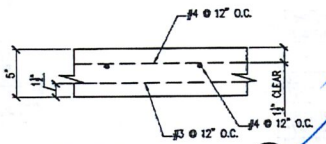
FLOOR PENETRATION (A)
SCALE: NONE



FLOOR EDGE (B)
SCALE: NONE



FLOOR EDGE (C)
SCALE: NONE



TOP SLAB REINF. (D)
SCALE: NONE

LINDEN C. PARCHMENT LICENSE No. 54071 STATE OF FLORIDA PROFESSIONAL ENGINEER
3/22/17

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb
CU. YARD: | SQ. FT: | WEIGHT:

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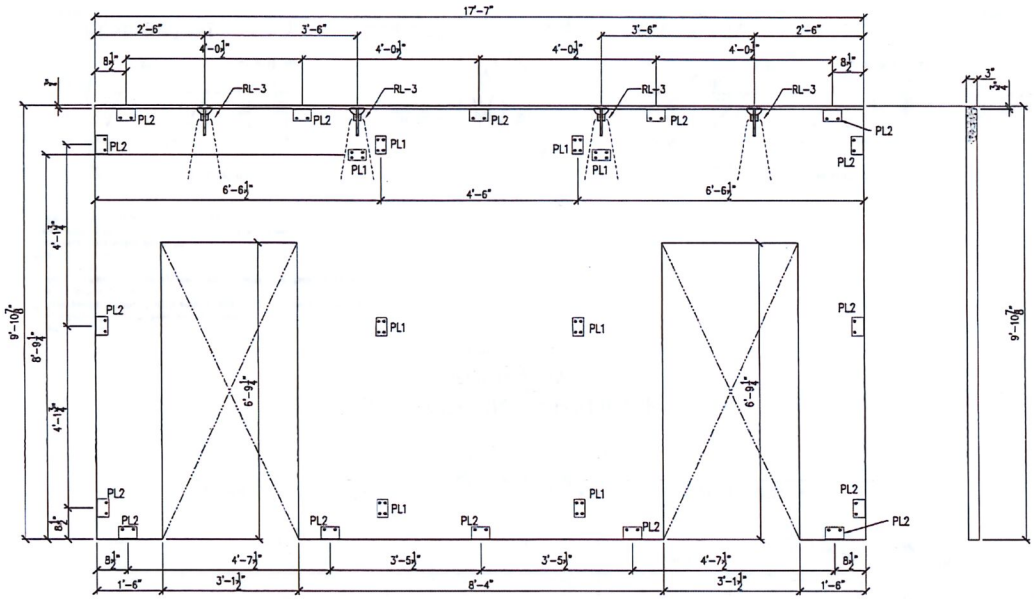
(352) 357-5807 AR0013558

SCALE: AS SHOWN		DATE: 3/22/2017
DESIGN: WDA	DRAWN: JK/LP	CHECKED: WDA

CLIENT: LEESBURG CONCRETE

PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL
17800 OLD CUTLER RD.
PALMENTO BAY, FL. 33157

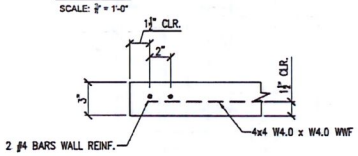
JOB NO: 16-57 | SHEET: 15 | OF: 42



LAYOUT - PANEL P1

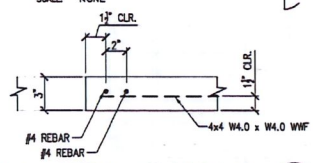
FORM VIEW

SCALE: ¾" = 1'-0"



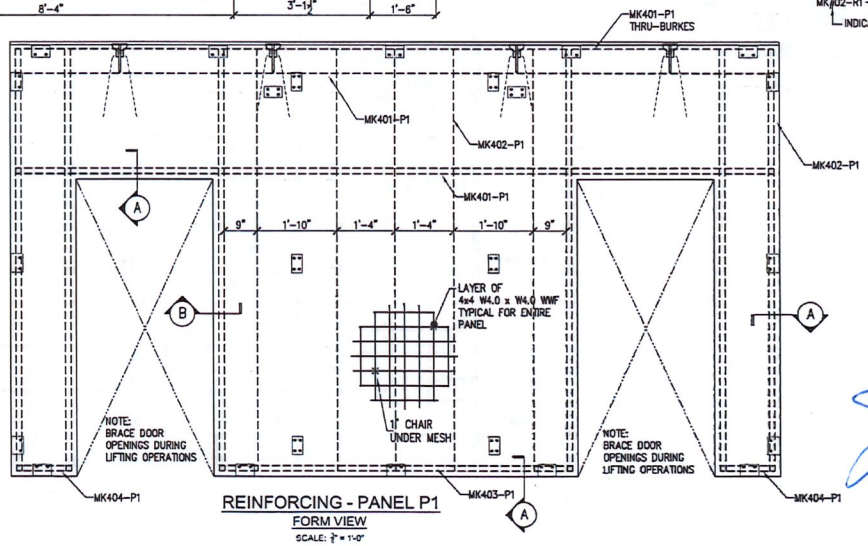
WALL DETAIL (A)

SCALE: NONE



WALL DETAIL AT OPENING (B)

SCALE: NONE



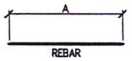
REINFORCING - PANEL P1

FORM VIEW

SCALE: ¾" = 1'-0"

MARK	DESCRIPTION	QUANTITY
	MEADOW BURKE RL-3 GALV. w/ RL-31 SHEAR BAR AND RL-32 TENSION BAR	4
	PL1	8
	PL2	16
	CONCRETE VOLUME	1.2 CU YD
	PANEL WEIGHT	4,920 lb

Notes:
1. Orientation of plan is from the form view.
2. Trim plastic sheath at both ends of post tensioned cable to fit light between anchor plates.
3. Note clearance on drawing from edge of panel to outside of cable.
4. Reinforcing sequence:
Place chairs under mesh
Place mesh and install Burke lift
Install cable. Hold cable at vertical center of the panel
5. Hold reinforcing bars 1 1/2 inch clear from edge of forms.



REBAR

MARK	QUANTITY	LENGTH "A"
MK401-P1	4	17'-4"
MK402-P1	17	9'-7"
MK403-P1	2	8'-1"
MK404-P1	4	1'-3"

MK402-R1 — PANEL REF.
└ INDICATES REBAR SIZE (4=#4 ETC.)

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb
CU. YARD: | SQ. FT: | WEIGHT:

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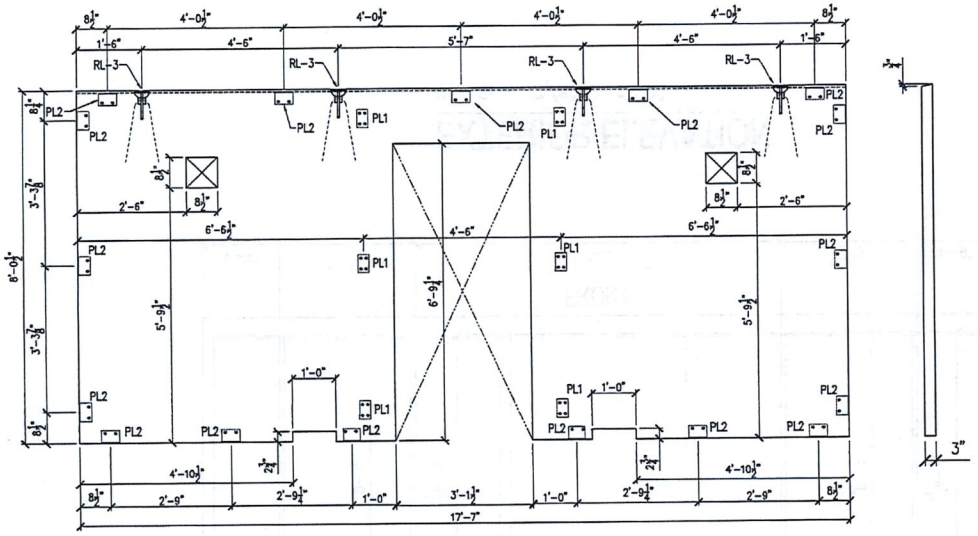
LINDEN C. PARCHMENT LICENSE No. 54071 STATE OF FLORIDA PROFESSIONAL ENGINEER

3/22/17

SCALE: AS SHOWN	DATE: 3/22/2017
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 16 OF: 42



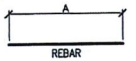
LAYOUT - PANEL P2

FORM VIEW

SCALE: ¼" = 1'-0"

MARK	DESCRIPTION	QUANTITY
	MEADOW BURKE RL-3 GALV. w/ RL-31 SHEAR BAR AND RL-32 TENSION BAR	4
	PL1	6
	PL2	17
	CONCRETE VOLUME	1.1 CU YD
	PANEL WEIGHT	4,500 lb

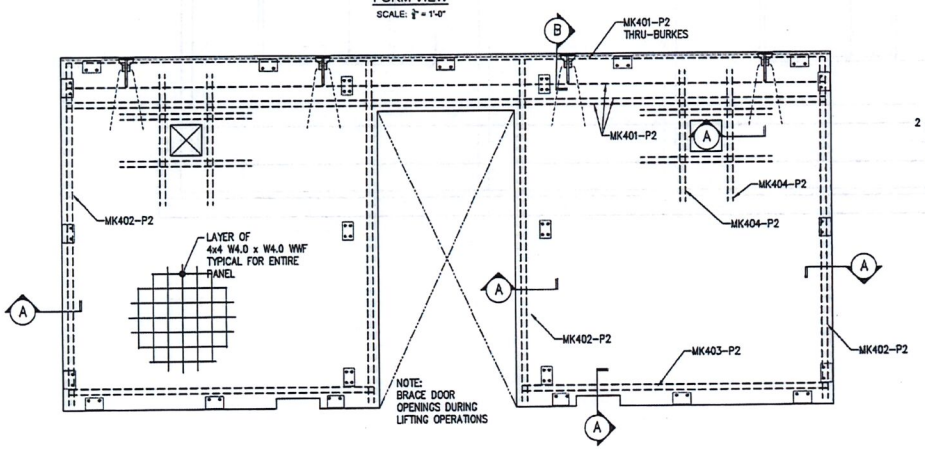
Notes:
1. Orientation of plan is from the form view.
2. Trim plastic sheath at both ends of post tensioned cable to fit tight between anchor plates.
3. Note clearance on drawing from edge of panel to outside of cable.
4. Reinforcing sequence:
Place chairs under mesh
Place mesh and install Burke lift
Install cable. Hold cable at vertical center of the panel
5. Hold reinforcing bars 1 1/2 inch clear from edge of forms.



REBAR

MARK	QUANTITY	LENGTH "A"
MK401-P2	4	17'-4"
MK402-P2	8	7'-9¼"
MK403-P2	4	6'-11½"
MK404-P2	16	3'-6"

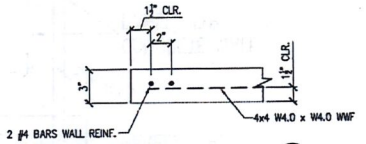
MK402-R1 — PANEL REF.
└ INDICATES REBAR SIZE (4=#4 ETC.)



REINFORCING - PANEL P2

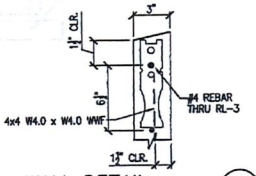
FORM VIEW

SCALE: ¼" = 1'-0"



WALL DETAIL

SCALE: NONE



WALL DETAIL

SCALE: NONE

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb
CU. YARD: | SQ. FT: | WEIGHT:

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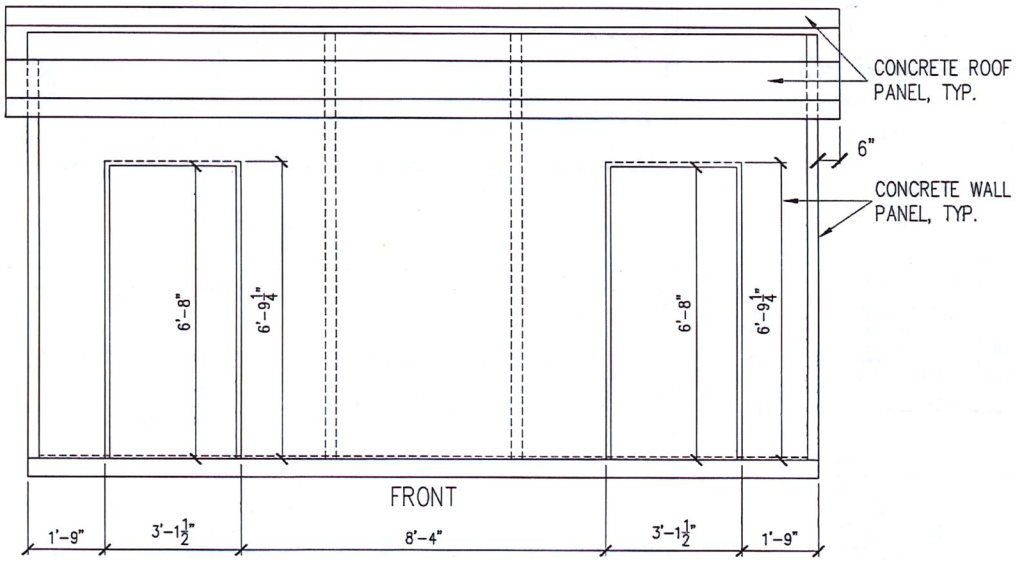
JOB NO: 16-57	SHEET: 17 OF: 42



EXTERIOR ELEVATION

SCALE: 3/8" = 1'-0"

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY

EXT FINISH:

INT FINISH:

MIX DESIGN: 5,000 PSI

RELEASE @ 3000 PSI

CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:

POST TENSION FORCE: 28,000 lb

CU. YARD: | SQ. FT: | WEIGHT:

Linden Parchment P.E.

8002 Glitter Court
Orlando, FL 32836

(407) 810-3322 P.E. # 54071

W. DAVID ARNOLD
ARCHITECT

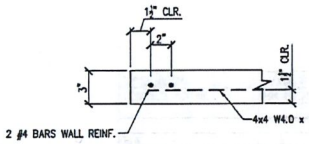
106 PORTLAND STREET
EUSTIS, FL 32726

(352) 357-5807 AR0013558

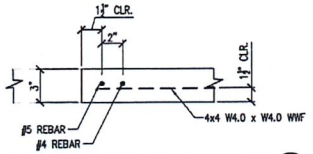
LINDEN C. PARCHMENT LICENSE No. 54071 STATE OF FLORIDA PROFESSIONAL ENGINEER

3/22/17

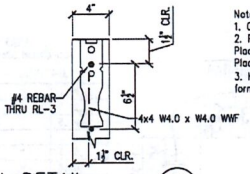
SCALE: AS SHOWN	DATE: 3/22/2017
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 10 OF: 42



WALL DETAIL (A)
SCALE: NONE

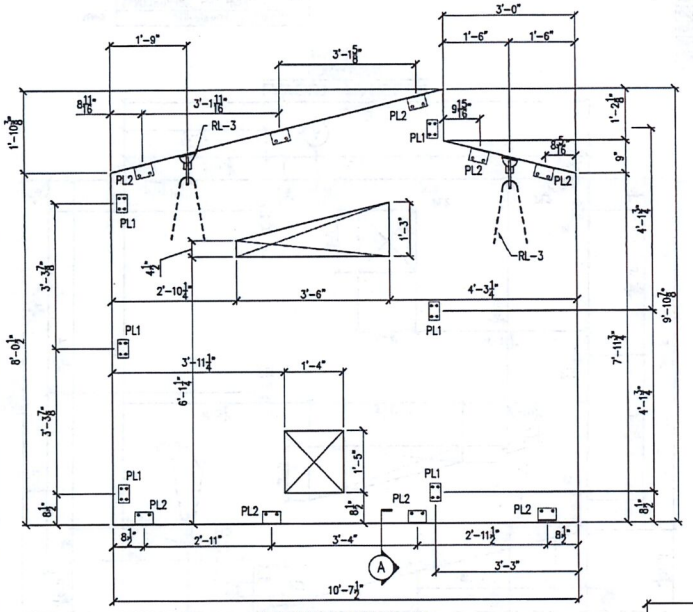


WALL DETAIL AT OPENING (B)
SCALE: NONE

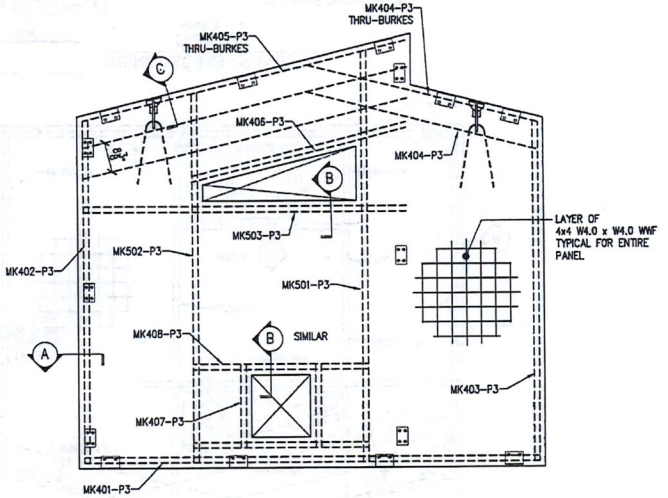


WALL DETAIL (C)
SCALE: NONE

Notes:
1. Orientation of plan is from the form view.
2. Reinforcing sequence:
Place chairs under mesh
Place mesh and install Burke lift
3. Hold reinforcing bars 1 1/2 inch clear from edge of forms.

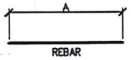


LAYOUT - PANEL P3
FORM VIEW
SCALE: ¼" = 1'-0"



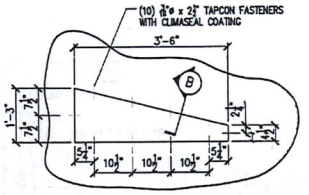
REINFORCING - PANEL P3
FORM VIEW
SCALE: ¼" = 1'-0"

MARK	DESCRIPTION	QUANTITY
	MEADOW BURKE RL-3 GALV. w/ RL-31 SHEAR BAR AND RL-32 TENSION BAR	2
	PL1	6
	PL2	9
	CONCRETE VOLUME	0.84 CU YD
	PANEL WEIGHT	3,520 lb



MARK	QUANTITY	LENGTH "A"
MK401-P3	2	10'-4 ½"
MK402-P3	2	7'-9 ½"
MK403-P3	2	7'-9"
MK404-P3	2	5'-4"
MK405-P3	3	7'-7"
MK406-P3	2	5'-0"
MK407-P3	4	2'-0"
MK408-P3	4	4'-0"
MK501-P3	2	9'-4½"
MK502-P3	2	8'-5"
MK503-P3	2	7'-4"

MK#02-P1 — PANEL REF.
└ INDICATES REBAR SIZE (4=#4 ETC.)



WINDOW DETAIL
SCALE: ½" = 1'-0"

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb
CU. YARD: | SQ. FT: | WEIGHT:

REVISIONS

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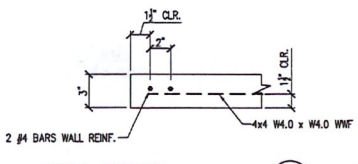
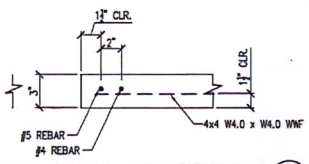
LINDEN C. PARCHMENT LICENSE No. 54071 STATE OF FLORIDA PROFESSIONAL ENGINEER

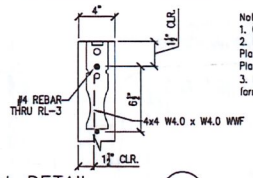
SCALE: AS SHOWN	DATE: 3/22/2017
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 18 OF: 42



WALL DETAIL (A)
SCALE: NONE

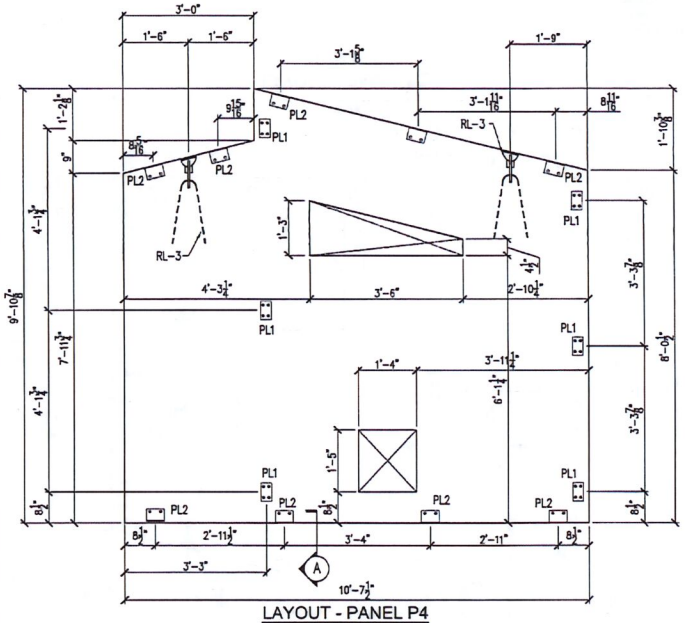


WALL DETAIL AT OPENING (B)
SCALE: NONE

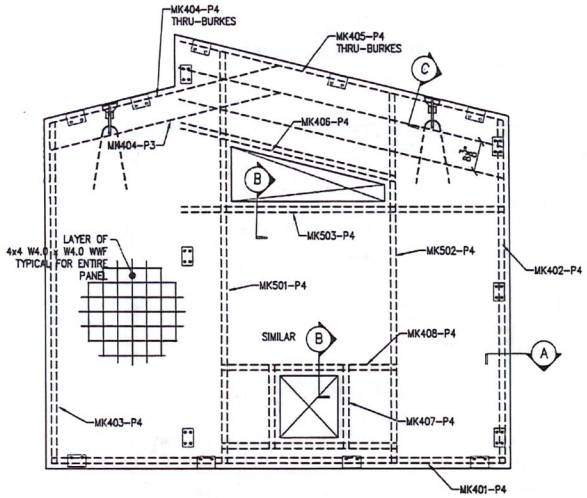


WALL DETAIL (C)
SCALE: NONE

Notes:
1. Orientation of plan is from the form view.
2. Reinforcing sequence:
Place chairs under mesh
Place mesh and install Burke lift
3. Hold reinforcing bars 1 1/2 inch clear from edge of forms.



LAYOUT - PANEL P4
FORM VIEW
SCALE: 3/8" = 1'-0"



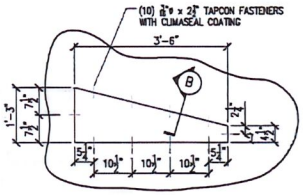
REINFORCING - PANEL P4
FORM VIEW
SCALE: 3/8" = 1'-0"

MARK	DESCRIPTION	QUANTITY
	MEADOW BURKE RL-3 GALV. w/ RL-31 SHEAR BAR AND RL-32 TENSION BAR	2
	PL1	6
	PL2	9
	CONCRETE VOLUME	0.84 CU YD
	PANEL WEIGHT	3,520 lb

A
REBAR

MARK	QUANTITY	LENGTH "A"
MK401-P4	2	10'-4 3/8"
MK402-P4	2	7'-9 3/8"
MK403-P4	2	7'-9"
MK404-P4	2	5'-4"
MK405-P4	3	7'-7"
MK406-P4	2	5'-0"
MK407-P4	4	2'-0"
MK408-P4	4	4'-0"
MK501-P4	2	9'-4 3/8"
MK502-P4	2	8'-5"
MK503-P4	2	7'-4"

MK#02-P1 — PANEL REF.
└ INDICATES REBAR SIZE (4=#4 ETC.)



WINDOW DETAIL
SCALE: 3/4" = 1'-0"

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb
CU. YARD: | SQ. FT: | WEIGHT:

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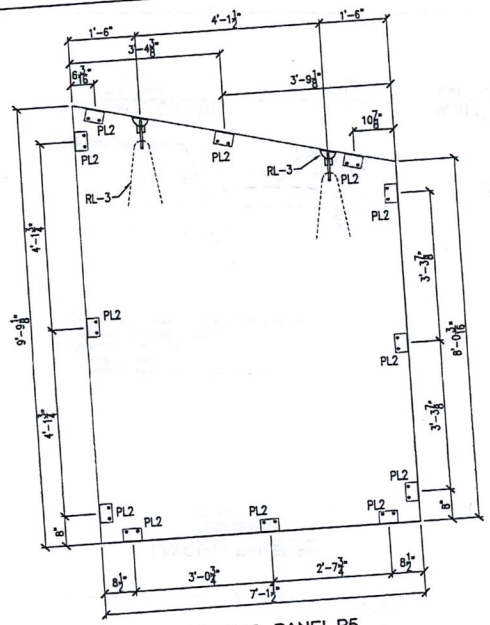
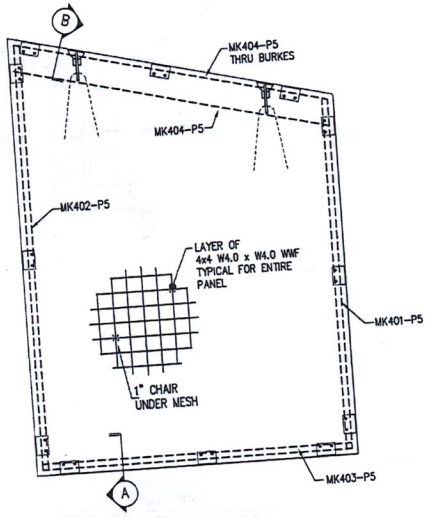
LINDEN C. PARCHMENT LICENSE No. 54071 STATE OF FLORIDA PROFESSIONAL ENGINEER
3/22/17

SCALE: AS SHOWN	DATE: 3/22/2017
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 19 OF: 42



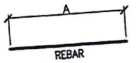
LAYOUT - PANEL P5
FORM VIEW
SCALE: 3/8" = 1'-0"



REINFORCING - PANEL P5
FORM VIEW
SCALE: 3/8" = 1'-0"

MARK	DESCRIPTION	QUANTITY
	MEADOW BURKE RL-3 GALV. w/ RL-31 SHEAR BAR AND RL-32 TENSION BAR	2
	PL1	
	PL2	12
	CONCRETE VOLUME	0.58 CU YD
	PANEL WEIGHT	2,454 lb

Notes:
1. Orientation of plan is from the form view.
2. Trim plastic sheath at both ends of post tensioned cable to fit tight between anchor plates.
3. Note clearance on drawing from edge of panel to outside of cable.
4. Reinforcing sequence:
Place chairs under mesh
Place mesh and install Burke lift
Install cable. Hold cable at vertical center of the panel
5. Hold reinforcing bars 1 1/2 inch clear from edge of forms.

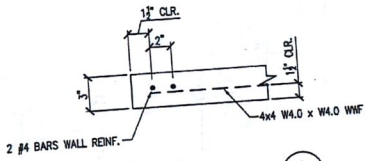


REBAR

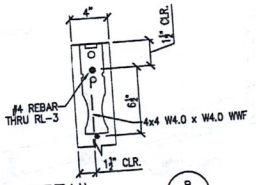
MARK	QUANTITY	LENGTH "A"
MK401-P5	2	7'-9"
MK402-P5	2	9'-5"
MK403-P5	2	6'-10"
MK404-P5	2	7'-0"

MK#02-R1 — PANEL REF.
└ INDICATES REBAR SIZE (4=#4 ETC.)

Notes:
1. Orientation of plan is from the form view.
2. Reinforcing sequence:
Place chairs under mesh
Place mesh and install Burke lift
3. Hold reinforcing bars 1 1/2 inch clear from edge of forms.



WALL DETAIL
SCALE: NONE
A



WALL DETAIL
SCALE: NONE
B

REVISIONS

RECEIVED
BLDG DIV

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb
CU. YARD: | SQ. FT: | WEIGHT:

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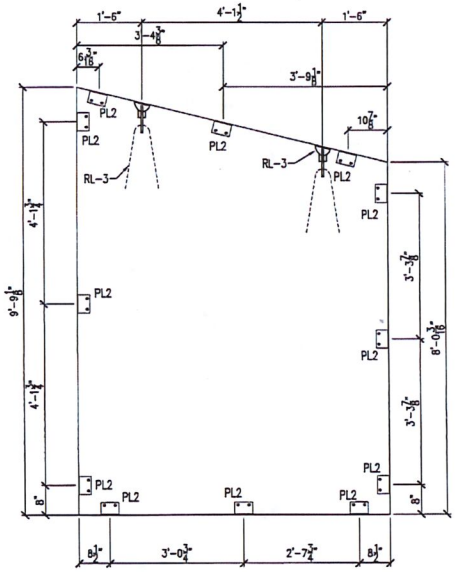
LINDEN C. PARCHMENT
LICENSE
No. 54071
STATE OF FLORIDA
PROFESSIONAL ENGINEER

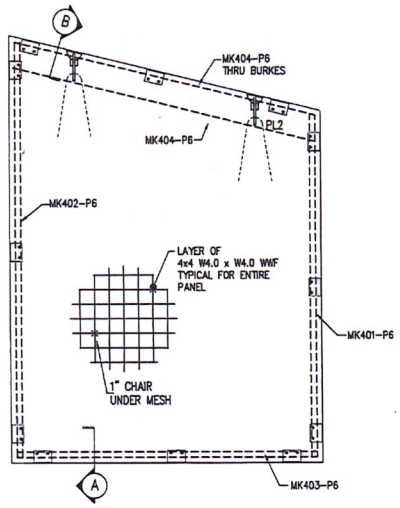
SCALE: AS SHOWN	DATE: 3/22/2017
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 20 OF: 42



LAYOUT - PANEL P6

FORM VIEW

SCALE: ¾" = 1'-0"



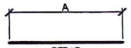
REINFORCING - PANEL P6

FORM VIEW

SCALE: ¾" = 1'-0"

MARK	DESCRIPTION	QUANTITY
	MEADOW BURKE RL-3 GALV. w/ RL-31 SHEAR BAR AND RL-32 TENSION BAR	2
	PL1	
	PL2	12
	CONCRETE VOLUME	0.58 CU YD
	PANEL WEIGHT	2,454 lb

Notes:
1. Orientation of plan is from the form view.
2. Trim plastic sheath at both ends of post tensioned cable to fit tight between anchor plates.
3. Note clearance on drawing from edge of panel to outside of cable.
4. Reinforcing sequence:
Place chairs under mesh
Place mesh and install Burke lift
Install cable. Hold cable at vertical center of the panel
5 Hold reinforcing bars 1 1/2 inch clear from edge of forms.

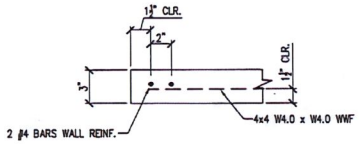


REBAR

MARK	QUANTITY	LENGTH "A"
MK401-P6	2	7'-9"
MK402-P6	2	9'-5"
MK403-P6	2	6'-10"
MK404-P6	2	7'-0"

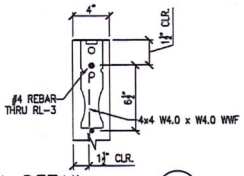
MK402-R1 —— PANEL REF.
└ INDICATES REBAR SIZE (4=#4 ETC.)

Notes:
1. Orientation of plan is from the form view.
2. Reinforcing sequence:
Place chairs under mesh
Place mesh and install Burke lift
3. Hold reinforcing bars 1 1/2 inch clear from edge of forms.



WALL DETAIL A

SCALE: NONE



WALL DETAIL B

SCALE: NONE

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb
CU. YARD: SQ. FT: WEIGHT:

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SCALE: AS SHOWN	DATE: 3/22/2017
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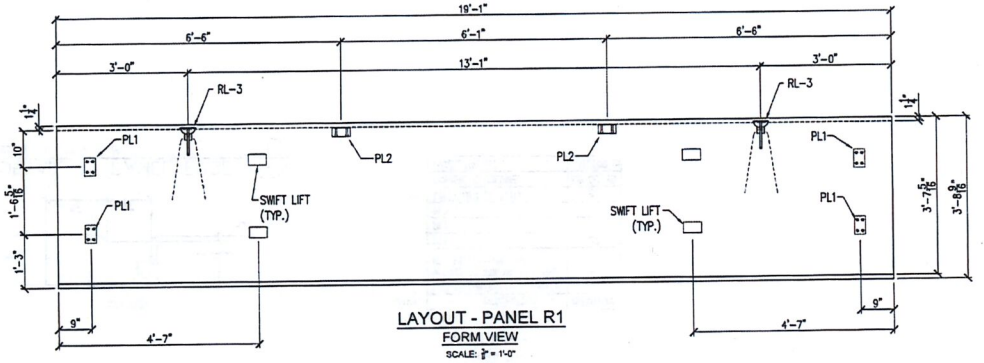
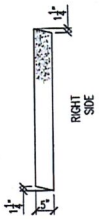
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 21 OF: 42



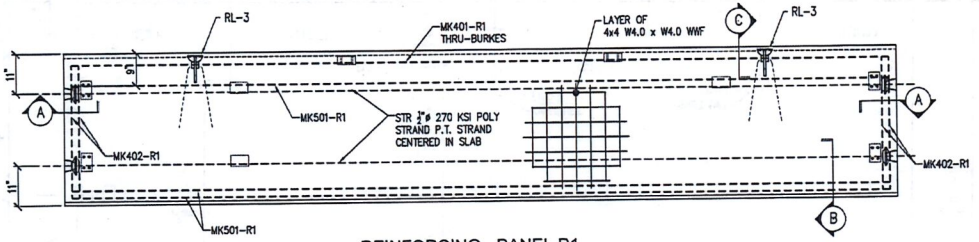
LAYOUT - PANEL R1

FORM VIEW

SCALE: 3/8" = 1'-0"



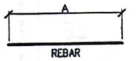
MARK	DESCRIPTION	QUANTITY
	MEADOW BURKE RL-3 GALV. w/ RL-31 SHEAR BAR AND RL-32 TENSION BAR	2
	PL1	4
	PL2	2
	PL3	
	SWIFT LIFT (2 TON)	4
	CONCRETE VOLUME	1.1 CU YD
	PANEL WEIGHT	4,500 lb



REINFORCING - PANEL R1

FORM VIEW

SCALE: 3/8" = 1'-0"



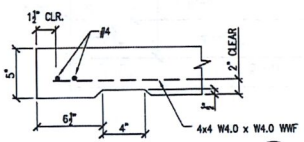
MARK	QUANTITY	LENGTH "A"
MK401-R1	1	18'-10"
MK402-R1	4	3'-5"
MK501-R1	3	18'-10"
STR	2	19'-1

MK#02-R1 —— PANEL REF.

└ INDICATES REBAR SIZE (4=#4 ETC.)

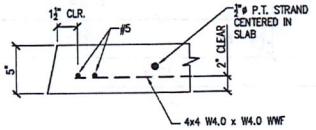
POST TENSION CASTING ANCHOR AND WEDGE FOR 19'-1" LONG 3/4"ø POLY STRAND TENDON. POST TENSION FORCE = 28,000 lbs

Notes:
1. Orientation of plan is from the form view.
2. Trim plastic sheath at both ends of post tensioned cable to fit tight between anchor plates.
3. Note clearance on drawing from edge of panel to outside of cable.
4. Reinforcing sequence:
Place chairs under mesh
Place mesh and install Burke lift
Install cable. Hold cable at vertical center of the panel
5. Hold reinforcing bars 1 1/2 inch clear from edge of forms.



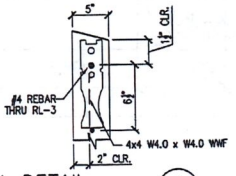
ROOF EDGE DETAIL (A)

SCALE: NONE



ROOF EDGE DETAIL (B)

SCALE: NONE



WALL DETAIL (C)

SCALE: NONE

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY

EXT FINISH:

INT FINISH:

MIX DESIGN: 5,000 PSI

RELEASE @ 3000 PSI

CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:

POST TENSION FORCE: 28,000 lb

CU. YARD: | SQ. FT: | WEIGHT:

Linden Parchment P.E.

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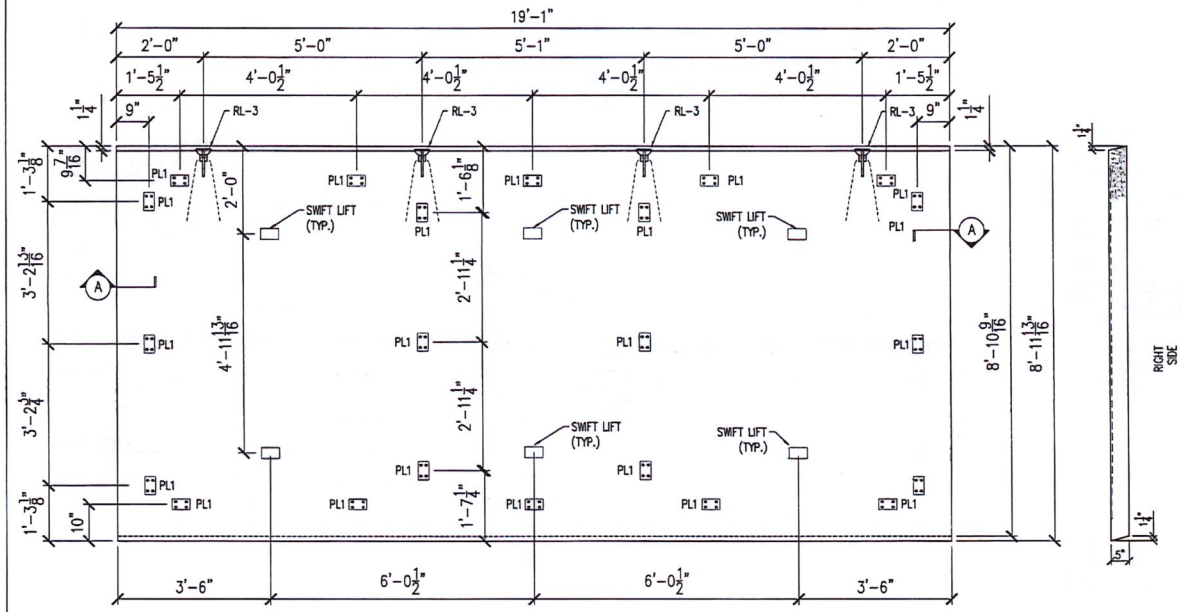
LINDEN C. PARCHMENT LICENSE No. 54071 STATE OF FLORIDA PROFESSIONAL ENGINEER

SCALE: AS SHOWN		DATE: 3/22/2017
DESIGN: WDA	DRAWN: JK/LP	CHECKED: WDA

CLIENT: LEESBURG CONCRETE

PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL
17800 OLD CUTLER RD.
PALMETTO BAY, FL 33157

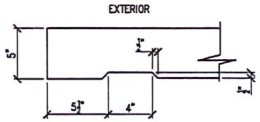
JOB NO: 16-57 | SHEET: 22 OF: 42



LAYOUT - PANEL R2

FORM VIEW

SCALE: ¼" = 1'-0"



DETAIL AT PANEL EDGE (A)

SCALE: 1 1/2" = 1'-0"

MARK	DESCRIPTION	QUANTITY
	MEADOW BURKE RL-3 GALV. w/ RL-31 SHEAR BAR AND RL-32 TENSION BAR	4
	PL1	22
	PL2	
	PL3	
	SWIFT LIFT (2 TON)	6
	CONCRETE VOLUME	2.6 CU YD
	PANEL WEIGHT	10,600 lb

REVISIONS

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		PL1
		PL2
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CONCRETE: 5000 PSI @ 28 DAY

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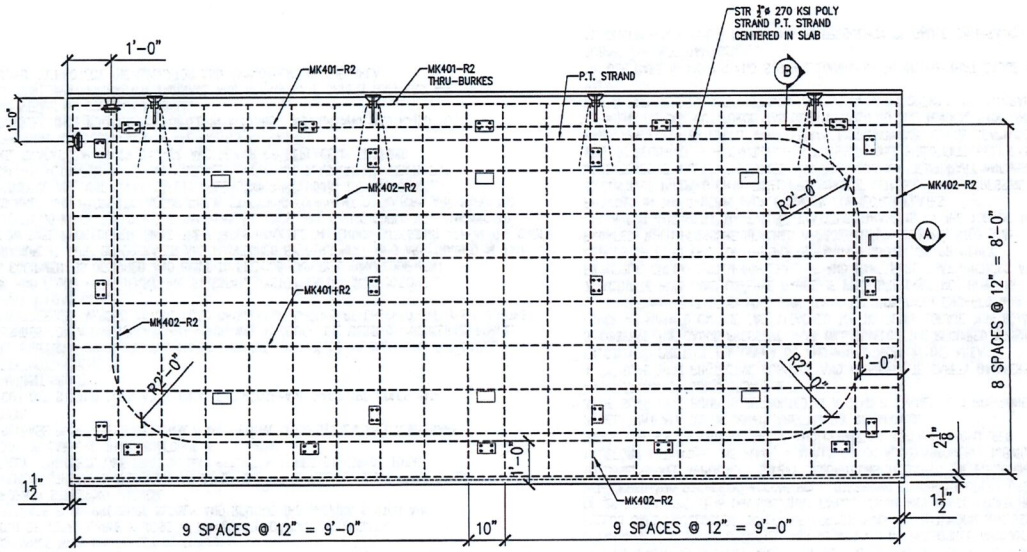
JOB NO: 16-57	SHEET: 23 OF: 42

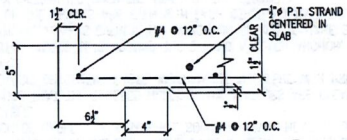


REINFORCING - PANEL R2

FORM VIEW

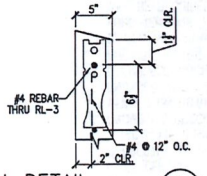
SCALE: 3/8" = 1'-0"

Notes:
1. Orientation of plan is from the form view.
2. Trim plastic sheath at both ends of post tensioned cable to fit light between anchor plates.
3. Note clearance on drawing from edge of panel to outside of cable.
4. Reinforcing sequence:
Place chairs under mesh
Place mesh and install Burke lift
Install cable. Hold cable at vertical center of the panel
5. Hold reinforcing bars 1 1/2 inch clear from edge of forms.



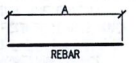
ROOF EDGE DETAIL (A)

SCALE: NONE



WALL DETAIL (B)

SCALE: NONE



MARK	QUANTITY	LENGTH "A"
MK401-R2	10	18'-10"
MK402-R2	20	8'-6"

MK402-R1 — PANEL REF.
└ INDICATES REBAR SIZE (4=#4 ETC.)

POST TENSION CASTING ANCHOR AND WEDGE FOR 47'-4" LONG 3/8"ø POLY STRAND TENDON. POST TENSION FORCE = 28,000 lbs

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
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TENDON STRESSING PROCEDURES FOR ROOFS AND FLOORS

1. REMOVE FORM RAILS. VERIFY CONCRETE STRENGTH WITH CYLINDER BREAKS. CONCRETE SHALL BE A MINIMUM OF 3,750 PSI BEFORE CABLE CAN BE STRESSED.
2. REMOVE RECESSED PLASTIC POCKET-FORMERS AT THE POST-TENSIONING ANCHOR.
3. REMOVE THE PLASTIC SHEATH FROM THE STRAND ALL THE WAY FROM THE BASE OF THE ANCHOR TO THE END OF THE STRAND. CLEAN OFF GREASE FROM EXPOSED PORTION OF CABLE. USE A SUITABLE SOLVENT. NEVER HEAT OR USE FLAME ON THE CABLE.
4. INSURE THAT THE TAPERED HOLES IN THE ANCHORS ARE CLEANED AND FREE OF ANY DEBRIS OR CEMENT PASTE, WHICH MIGHT INTERFERE WITH PROPER WEDGE SEATING.
5. INSTALL WEDGE SETS AROUND THE STRAND AT EACH ANCHOR. PRESS THE WEDGES IN WITH THE FINGERS EVENLY. TAP WEDGES IN SECURELY. MAKE SURE THE ENDS OF THE WEDGES ARE EVEN WITH EACH OTHER SO THAT THE STRESS WILL BE EVENLY DISTRIBUTED OVER THE WIRES OF THE STRAND.
6. SLIDE THE WEDGE-SETTER OVER THE CABLE AND PLACE AGAINST THE WEDGES INSTALLED IN THE CAST-IN ANCHORS (SEE DETAIL 4 ON SHEET 26).
7. INSTALL THE TEMPORARY REUSABLE ANCHOR ON THE STRAND. THE PLATES OF THE JACK WILL PUSH AGAINST THIS ANCHOR TO ELONGATE AND, THEREFORE, STRESS THE STRAND. SET THE TEMPORARY STRESSING ANCHOR APPROXIMATELY 5" BEYOND THE REAR OF THE WEDGE-SETTER (SEE DETAIL 6 ON SHEET 26).
8. INSTALL THE PLATES OF THE TWIN RAM STRESSING JACK OVER THE STRAND BETWEEN THE WEDGE-SETTER AND THE REUSABLE STRESSING ANCHOR.
9. STANDING BACK FROM THE JACK, USING A REMOTE SWITCH FOR THE PUMP, STRESS THE STRANDS UNTIL THE PUMP PRESSURE GAUGE INDICATES A PRESSURE LISTED ON THE CALIBRATION CHART OF YOUR STRESSING PUMP, THAT CORRESPONDS TO 28 K REQUIRED TENSION ON THE ½" CABLE. WHENEVER STRESSING BE SURE TO OBSERVE SAFETY PRECAUTIONS AND OPERATOR POSITIONING IN ANTICIPATION OF UNEXPECTED FAILURES. NOTE: AS A DOUBLE-CHECK TO INSURE PROPER ELONGATION OF THE STRAND, MEASURE THE DISTANCE THE JACK HAS STRETCHED THE CABLE. THIS DIMENSION SHOULD BE IN ACCORDANCE WITH THE FOLLOWING FORMULA:
LENGTH OF TOTAL CABLE X .0795 = THE LENGTH OF CABLE ELONGATION.
10. RELEASE PUMP PRESSURE SLOWLY AND REMOVE THE JACKING SYSTEM AND TEMPORARY STRESSING ANCHOR.
11. REPEAT STRESSING OPERATION AT OTHER END OF CABLE. TO INSURE THAT CABLE IS FULLY TENSIONED. LINE FRICTION MAY RESULT IN LOWER STRESS OF TOTAL LENGTH OF CABLE IF THIS PROCEDURE IS NOT DONE.
12. RELEASE PUMP PRESSURE SLOWLY AND REMOVE JACK SYSTEM AND TEMPORARY ANCHOR.
12. CUT OFF STRAND TAILS AT 1" FROM THE WEDGE AND PACK THE HOLES WITH PERMANENT GROUT.

SAFETY INSTRUCTIONS:
NOTE: STRESSING UNITS ARE CALIBRATED AGAINST LOAD CELLS TO DETERMINE THE GAUGE PRESSURES FOR A PARTICULAR UNIT WHICH WILL PRODUCE THE DESIRED ENGINEERING FORCE. GAUGE PRESSURE VERSUS ACTUAL LOAD CALIBRATION SHOULD BE CHECKED EVERY SIX MONTHS AGAINST A LOAD CELL.
ROOF AND FLOOR POST-TENSIONING STRESSING SYSTEM SAFETY INSTRUCTIONS
THIS EQUIPMENT IS DESIGNED AND BUILT TO PROVIDE SAFETY DURING EQUIPMENT OPERATION. IT MUST BE OPERATED AND MAINTAINED BY PERSONNEL WHO ARE TRAINED IN AND FOLLOW SAFE PROCEDURES. SINCE THE FORCES INVOLVED IN TENDON STRESSING ARE IN THE ORDER OF 28 TO 30 KIPS, IT IS OBVIOUS THAT FAILURE FROM ANY CAUSE COULD BE DAMAGING TO PERSONNEL AND PROPERTY. ACCORDINGLY, STRESSING EQUIPMENT OPERATORS ARE EXPECTED TO OBSERVE ALL APPLICABLE SAFETY PRECAUTIONS INCLUDING THE FOLLOWING:
1. BEFORE ACTIVATING THE PUMP, VISUALLY CHECK ALL ELASTOMERIC TUBING FOR NICKS, CRACKS, OR OTHER DAMAGE AND REPAIR OR REPLACE IF REQUIRED.
2. SEE THAT ALL TUBING JOINTS ARE TIGHTLY CONNECTED.
3. BE SURE THAT PUMP IS SUPPORTED IN A STABLE MANNER AND POSITIONED TO AVOID STRAIN ON CONNECTION LINES.
4. BE SURE THAT PROTECTIVE DUNNAGE AND APPROPRIATE SAFETY BARRIERS ARE ERECTED TO PROTECT THE OPERATOR AND ANY OTHERS IN THE AREA.
5. OBSERVE THE GAUGE PRESSURE THAT WILL DEVELOP THE REQUIRED TENSILE FORCE AS MARKED ON THE TAG ATTACHED TO THE PUMP. LOCATE THIS READING ON GAUGE FACE.
6. BE CERTAIN THAT TENSIONING TWIN-RAMS ARE POSITIVELY ENGAGED AND CORRECTLY ALIGNED.
7. ACTIVATE PUMP AND OBSERVE GAUGE. IF PRESSURE DOES NOT BUILD UP PROMPTLY, CHECK SYSTEM FOR LEAKAGE AND MAKE SURE THAT ANCHORS AND WEDGES ON BOTH ENDS OF THE POLYSTRAND® TENDONS ARE PROPERLY ENGAGED. WHEN PRESSURE REACHES APPROXIMATELY 1000 PSI, STOP AND CHECK FOR ALIGNMENT. MAKE SURE THAT THE STRAND IS AT THE TOP OF THE SLOT PROVIDED IN THE TENSION JACK CYLINDER CONNECTOR PLATES.
8. CONTINUE PUMPING UNTIL FINAL PRESSURE IS ATTAINED. WHEN STRESSING LONG TENDONS, MULTIPLE STROKES AND RESETTING THE TEMPORARY ANCHOR WILL BE REQUIRED. - - CAUTION - - PRESSURE WILL BUILD EXTREMELY FAST IF RAMS ARE FULLY EXTENDED. MONITOR RAM EXTENSION AND GAUGE READINGS CONTINUOUSLY.
9. WHEN THE CORRECT FORCE HAS BEEN APPLIED TO THE TENDON, STOP THE PUMP MOTOR AND RELEASE THE PRESSURE ON THE PACK BY OPENING THE HYDRAULIC VALVE.
10. USE CARE IN MOVING AND STORING EQUIPMENT TO INSURE THAT GAUGE AND TUBING ARE NOT DAMAGED.
11. OBSERVE SITE RULES PERTAINING TO FREQUENCY OF GAUGE CALIBRATION.

SAFETY INSTRUCTIONS:
NOTE: STRESSING UNITS ARE CALIBRATED AGAINST LOAD CELLS TO DETERMINE THE GAUGE PRESSURES FOR A PARTICULAR UNIT WHICH WILL PRODUCE THE DESIRED ENGINEERING FORCE. GAUGE PRESSURE VERSUS ACTUAL LOAD CALIBRATION SHOULD BE CHECKED EVERY SIX MONTHS AGAINST A LOAD CELL. THE SAMPLE TABLE ON A1.08 LISTS THE CURRENT EQUIPMENT IN SERVICE. NEVER USE EQUIPMENT WHICH HAS NOT BEEN TESTED IN THE LAST 6 MONTHS. THE QUALITY CONTROL DEPARTMENT HAS THE RESPONSIBILITY TO INSURE THE STRESSING UNIT IS CALIBRATED SEMI-ANNUALLY.
ROOF AND FLOOR POST-TENSIONING STRESSING SYSTEM SAFETY INSTRUCTIONS
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REVISIONS

ITEM	QTY	DESCRIPTION
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		PL1
		PL2
		PL3
	4	BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
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POST TENSION FORCE: 28,000 lb
CU. YARD: SQ. FT: WEIGHT:

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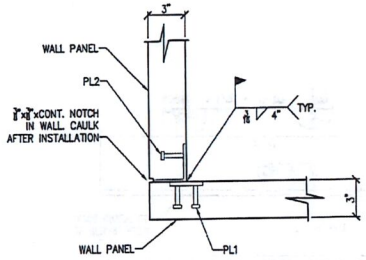
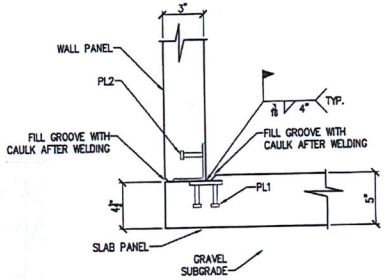
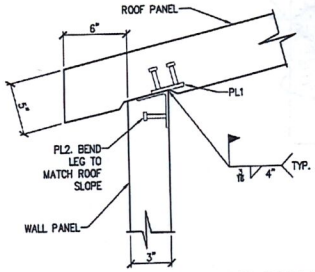


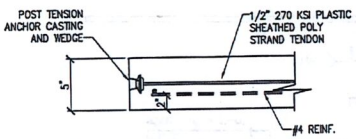
TYPICAL WALL TO WALL CONN. (1)
SCALE: 1 1/2" = 1'-0"



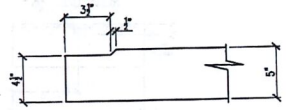
TYPICAL WALL TO FLOOR CONN. (2)
SCALE: 1 1/2" = 1'-0"



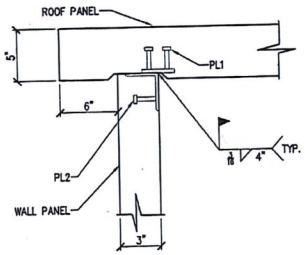
TYPICAL WALL TO ROOF CONN. (3)
SCALE: 1 1/2" = 1'-0"



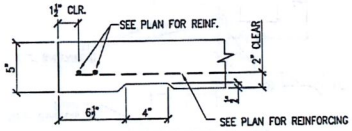
POST TENSION ANCHOR DETAIL (4)
SCALE: 1 1/2" = 1'-0"



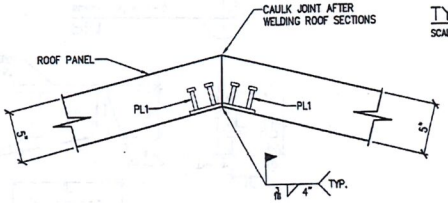
FLOOR RECESS DETAIL (5)
SCALE: 1 1/2" = 1'-0"



TYPICAL WALL TO ROOF CONN. (8)
SCALE: 1 1/2" = 1'-0"



ROOF EDGE DETAIL (6)
SCALE: NONE



ROOF TO ROOF CONN. AT ALTERNATE (7)
SCALE: 1 1/2" = 1'-0"

REVISIONS

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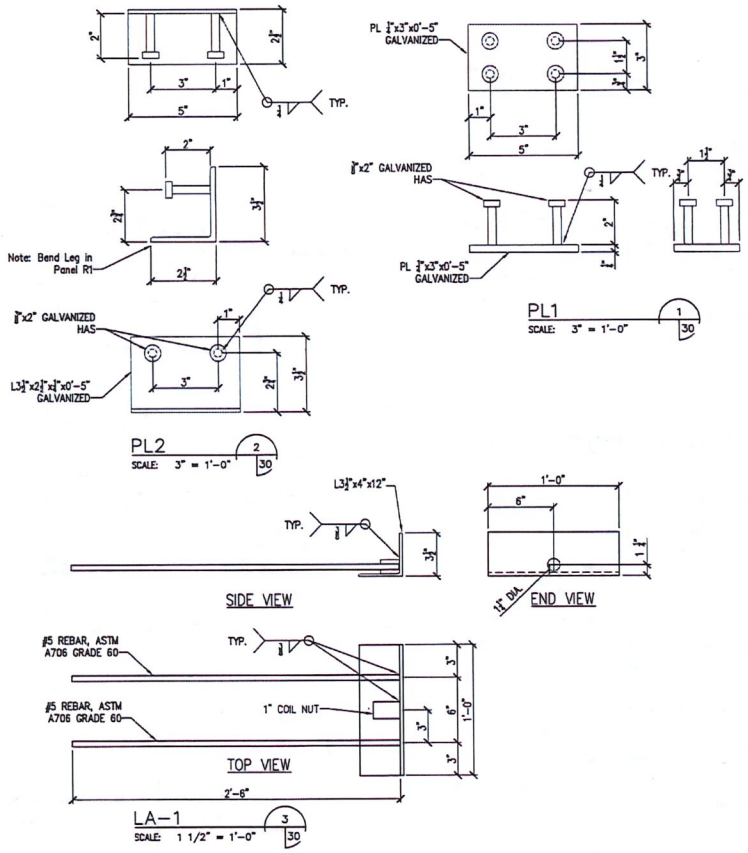

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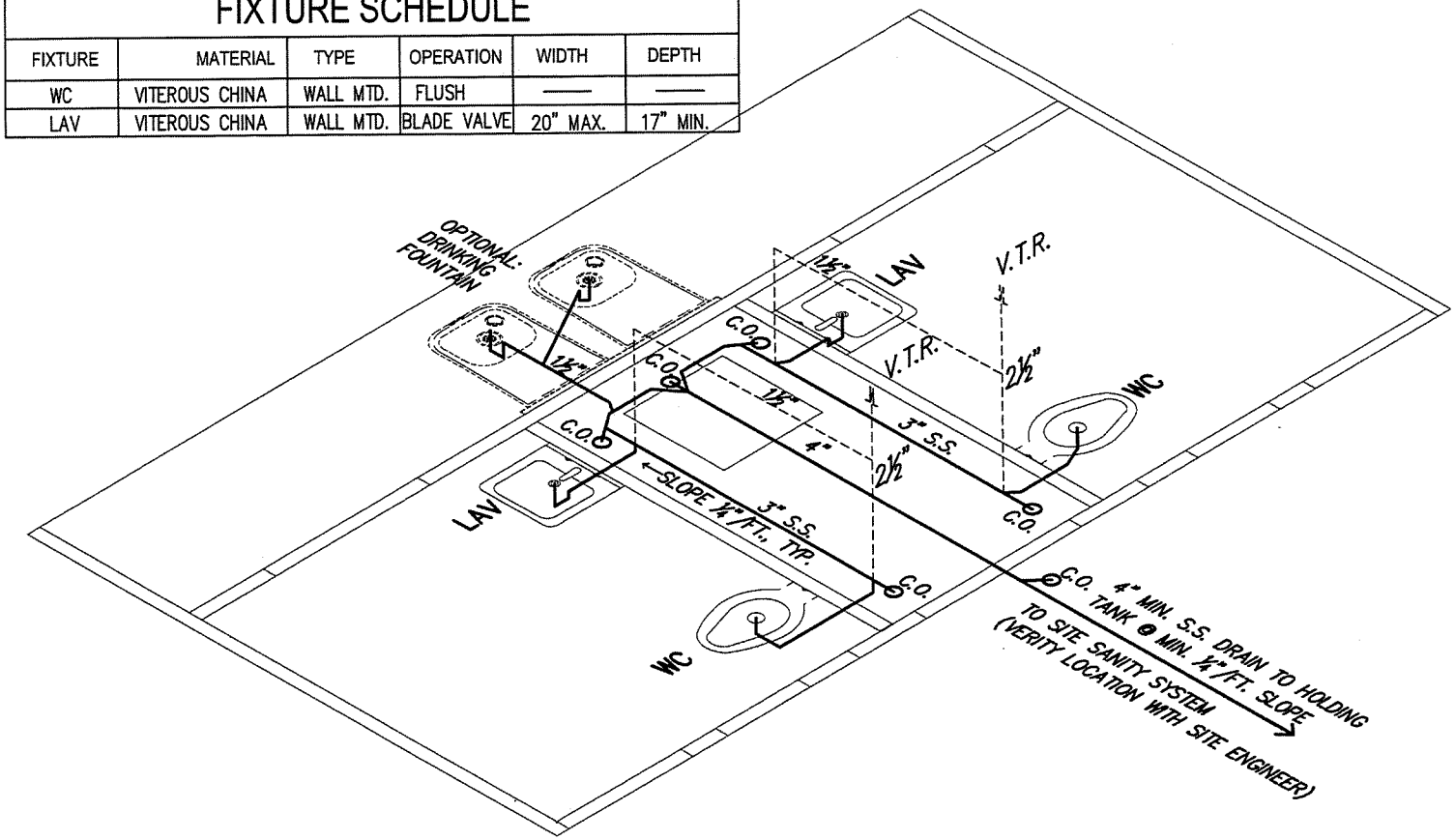
JOB NO: 16-57

SHEET: 27 OF: 42

FIXTURE SCHEDULE

FIXTURE	MATERIAL	TYPE	OPERATION	WIDTH	DEPTH
WC	VITEROUS CHINA	WALL MTD.	FLUSH	——	——
LAV	VITEROUS CHINA	WALL MTD.	BLADE VALVE	20" MAX.	17" MIN.



WET VENT WASTE RISER

SCALE: NONE

David Arnold
23 Mar 17

REVISIONS

1 DATE: 03/22/2017
CHANGED ALL WIND SPEED REFERENCES FROM 170 TO 175 & RECALCULATED ROOF & WALL PRESSURES

ITEM	QTY	DESCRIPTION
		BURKE BL-2 W/ BL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 8000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONCRETE STRENGHT FOR POST TENSIONING: 4,500/CU. YARD:
POST TENSION FORCE: 28,000 PSI

CU. YARD:	SQ. FT:	WEIGHT:

W. DAVID ARNOLD
ARCHITECT
106 PORTLAND STREET
EUSTIS, FL. 32726
(352) 357-5807 AR0013558

SCALE:	DATE:
AS SHOWN	10/6/16

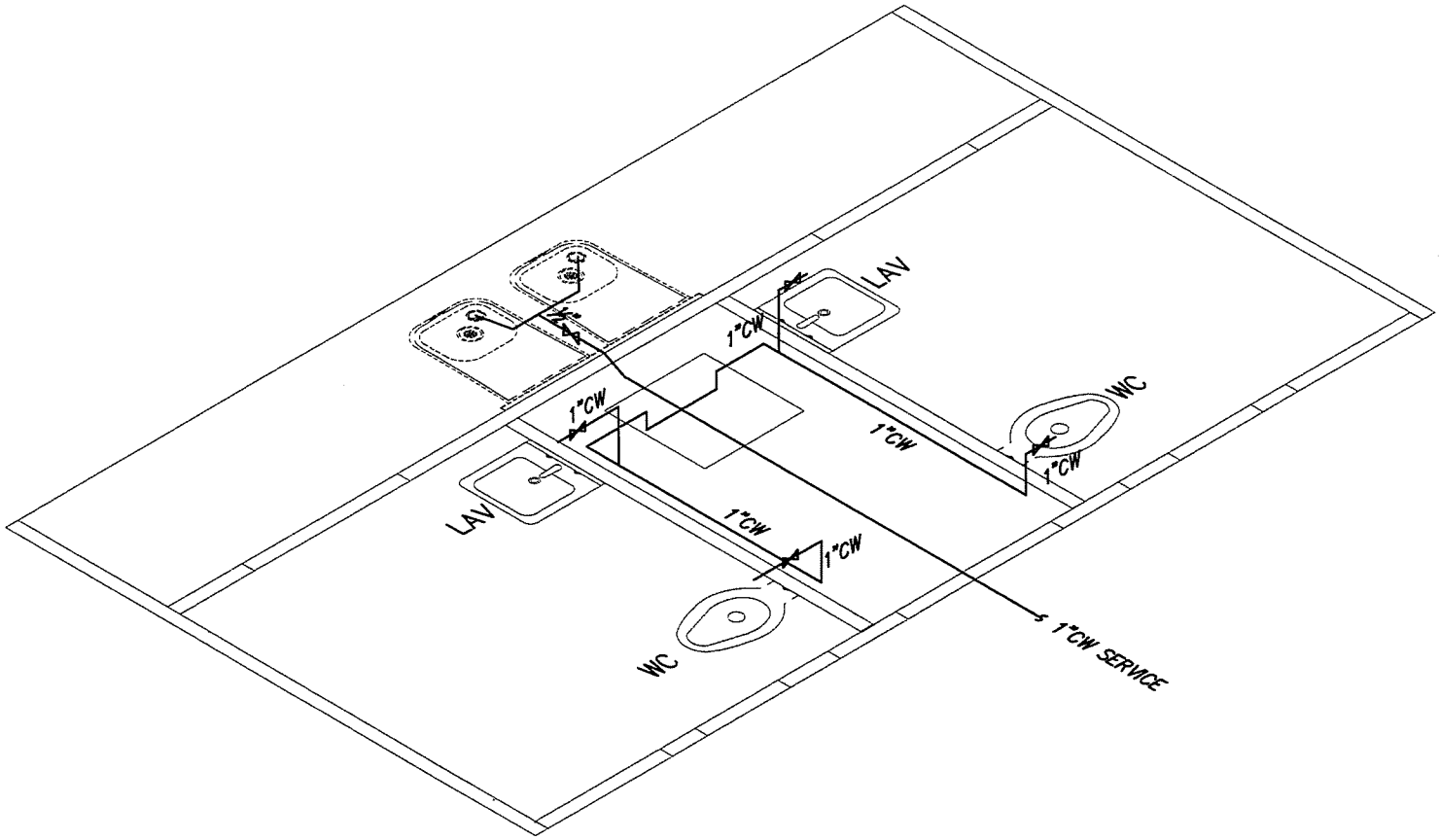
DESIGN:	DRAWN:	CHECKED:
WDA	JK/LP	WDA

CLIENT: LEESBURG CONCRETE

PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL
17800 OLD CUTLER RD.
PALMENTO BAY, FL. 33157

JOB NO: 16-57

SHEET: 30 OF: 42



SUPPLY RISER - CW

SCALE: NONE

REVISIONS

1 DATE: 03/22/2017
CHANGED ALL WIND SPEED REFERENCES FROM 170 TO 175 & RECALCULATED ROOF & WALL PRESSURES

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL3
		PL3
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONCRETE STRENGHT FOR POST TENSIONING: 4,000CCL YARD:
POST TENSION FORCE: 30,000 PSI
CU. YARD: SQ. FT: WEIGHT:

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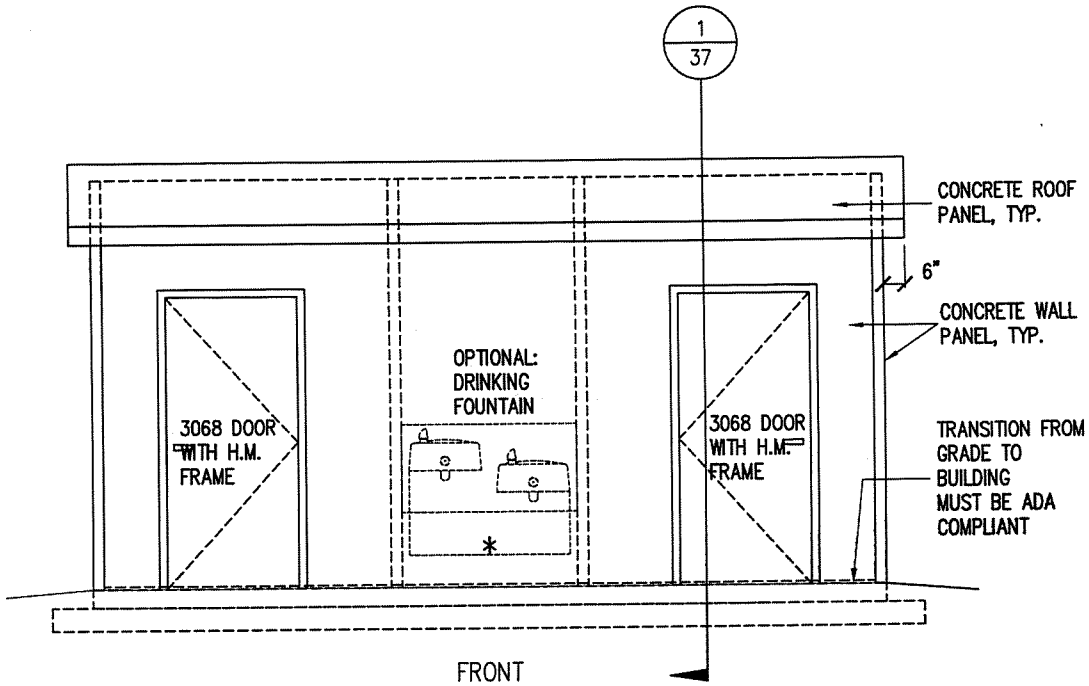
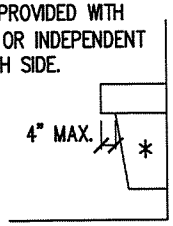
David Arnold
23 Mar 17

SCALE: AS SHOWN	DATE: 10/6/16
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 31 OF: 42



* CANE PROTECTION REQUIRED. MAYBE SHROWD PROVIDED WITH HI/LO FOUNTAIN OR INDEPENDENT BARRIER ON EACH SIDE.



ALTERNATE BID

EXTERIOR ELEVATION

SCALE: 3/8" = 1'-0"

REVISIONS

1 DATE: 03/22/2017
CHANGED ALL WIND SPEED REFERENCES FROM 170 TO 175 & RECALCULATED ROOF & WALL PRESSURES

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 6000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONCRETE STRENGHT FOR POST TENSIONING: 4,000(CU. YARD:
POST TENSION FORCE: 28,000 PSI
CU. YARD: SQ. FT: WEIGHT:

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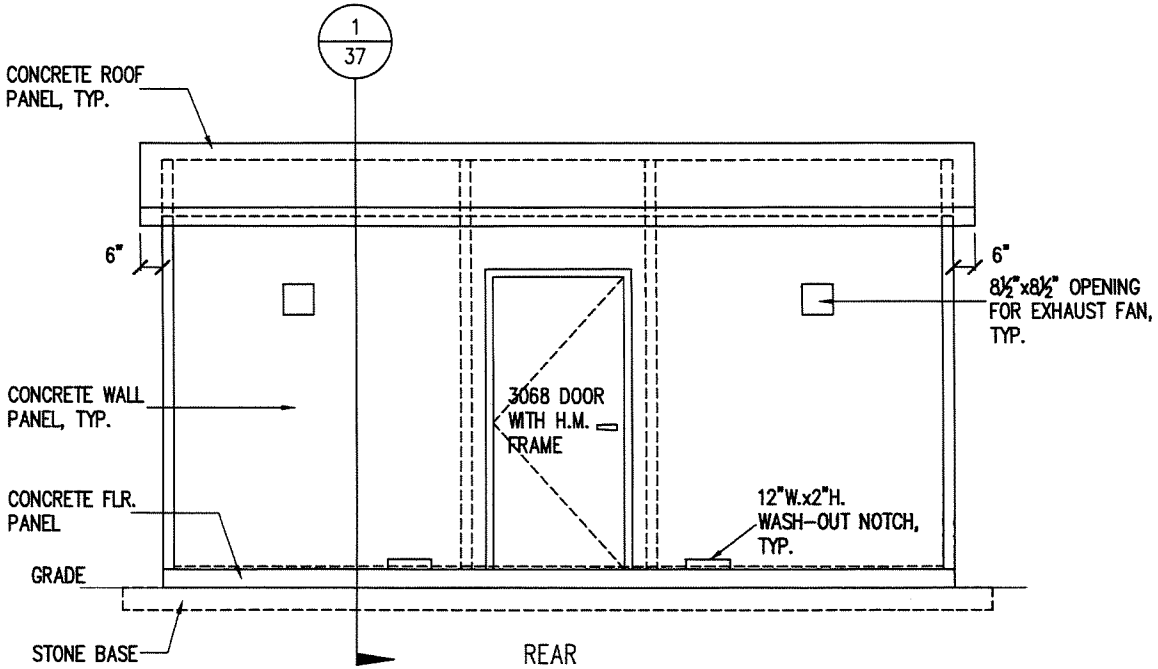
David Arnold 23 Mar 17

SCALE: AS SHOWN	DATE: 10/6/16
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 34 OF: 42



ALTERNATE BID

EXTERIOR ELEVATION

SCALE: 3/8" = 1'-0"

REVISIONS

1 DATE: 03/22/2017
CHANGED ALL WIND SPEED REFERENCES FROM 170 TO 175 & RECALCULATED ROOF & WALL PRESSURES

ITEM	QTY	DESCRIPTION
		BURNE RL-2 W/ RL-39 & SHEAR BAR
		PL1
		PL2
		PL3
		BURNE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONCRETE STRENGHT FOR POST TENSIONING: 4,000CCL YARD:
POST TENSION FORCE: 30,000 PSI

CU. YARD:	SQ. FT:	WEIGHT:

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David Arnold
23 Mar 17

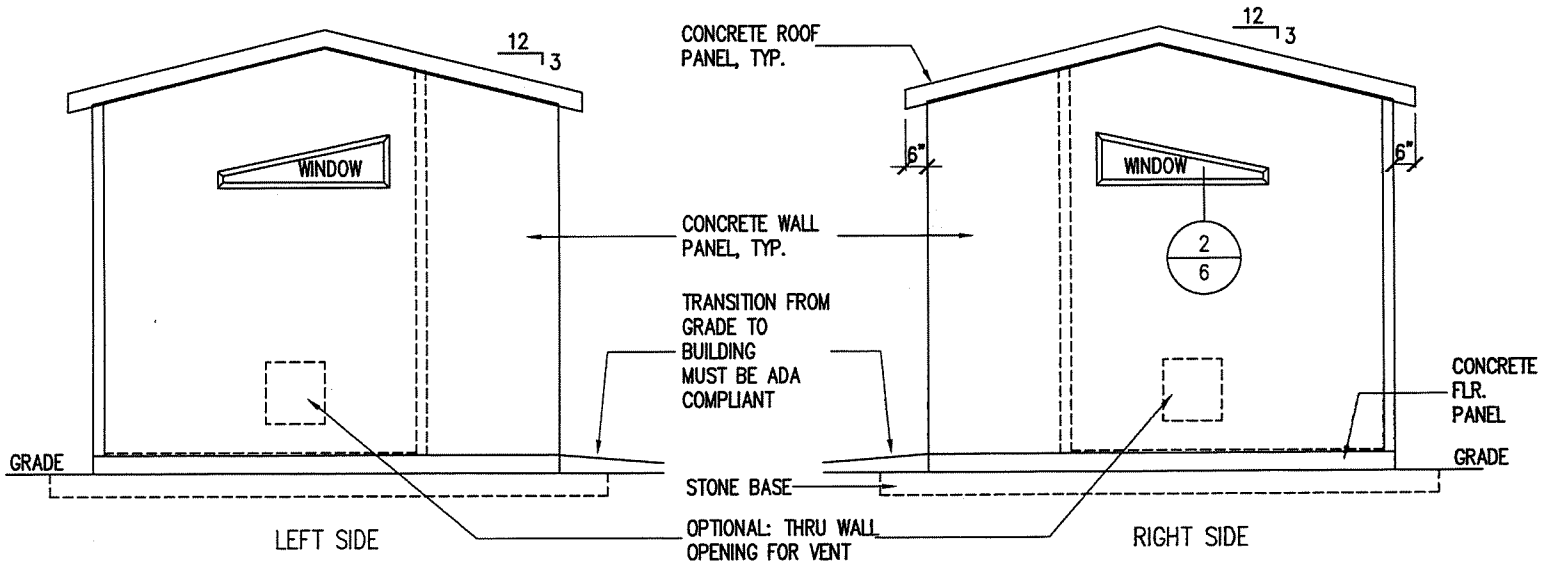
SCALE: AS SHOWN		DATE: 10/6/16
DESIGN: WDA	DRAWN: JK/LP	CHECKED: WDA

CLIENT: LEESBURG CONCRETE

PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL
17800 OLD CUTLER RD.
PALMENTO BAY, FL. 33157

JOB NO: 16-57

SHEET: 35 OF: 42



ALTERNATE BID

EXTERIOR ELEVATIONS

SCALE: 3/8" = 1'-0"

REVISIONS

1 DATE: 03/22/2017
CHANGED ALL WIND SPEED REFERENCES FROM 170 TO 175 & RECALCULATED ROOF & WALL PRESSURES

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 6000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONCRETE STRENGHT FOR POST TENSIONING: 4,500CUL YARD
POST TENSION FORCE: 30,000 PSI

CU. YARD:	SQ. FT:	WEIGHT:

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David Arnold 23 Mar 17

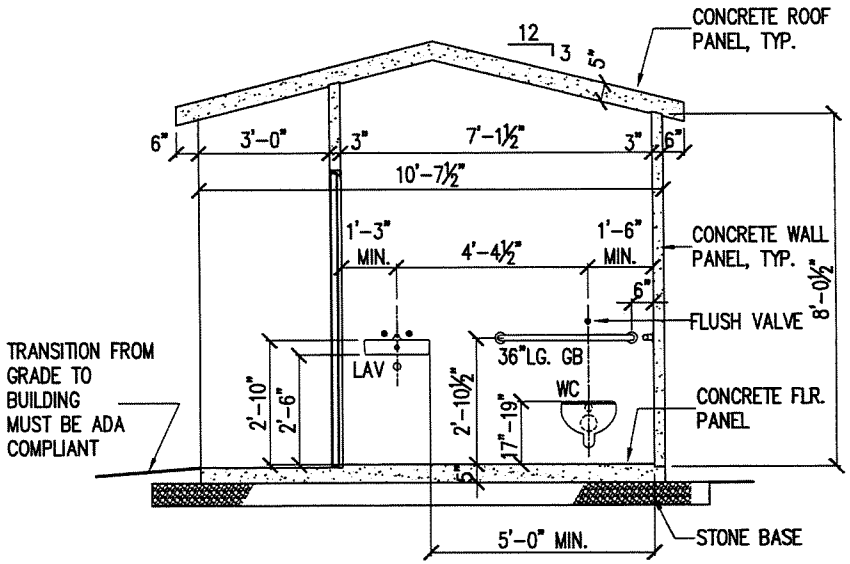
SCALE: AS SHOWN		DATE: 10/6/16
DESIGN: WDA	DRAWN: JK/LP	CHECKED: WDA

CLIENT: LEESBURG CONCRETE

PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL
17800 OLD CUTLER RD.
PALMETTO BAY, FL 33157

JOB NO: 16-57

SHEET: 36 OF: 42



ALTERNATE BID

BUILDING CROSS SECTION

SCALE: 3/8" = 1'-0"

1
34 | 37

35

REVISIONS

1 DATE: 03/22/2017
CHANGED ALL WIND SPEED REFERENCES FROM 170 TO 175 & RECALCULATED ROOF & WALL PRESSURES

ITEM	QTY	DESCRIPTION
		SURE BL-2 W/ BL-3B & SHEAR BAR
		FL1
		FL2
		FL3
		SURE SHIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 3000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONCRETE STRENGHT FOR POST TENSIONING: 4,500CCU. YARD:
POST TENSION FORCE: 36,000 PSI

CU. YARD:	SQ. FT:	WEIGHT:

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(352) 357-5807 AR0013558

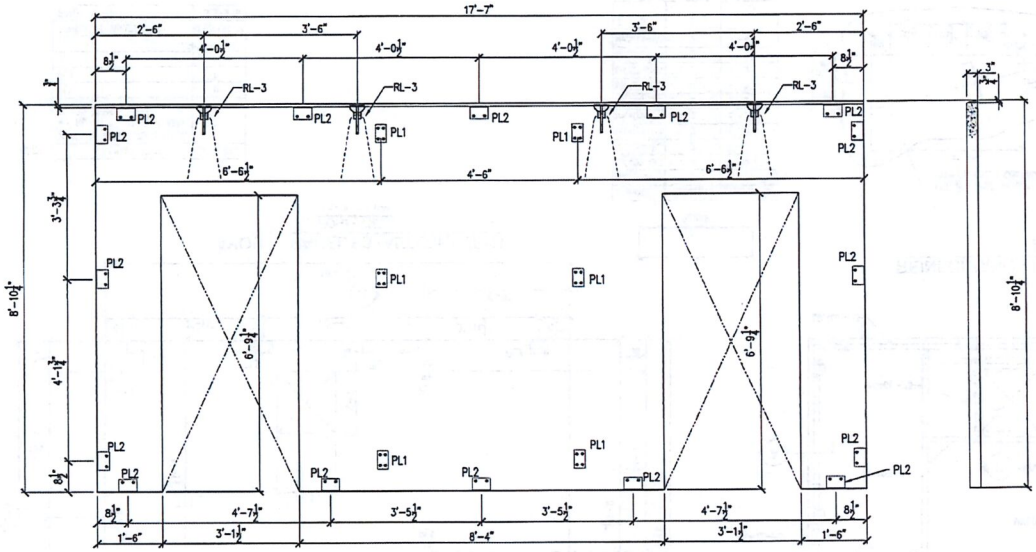
David Arnold
23 Mar 17

SCALE: AS SHOWN		DATE: 10/6/16
DESIGN: WDA	DRAWN: JK/LP	CHECKED: WDA

CLIENT: LEESBURG CONCRETE

PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL
17600 OLD CUTLER RD.
PALMETTO BAY, FL. 33157

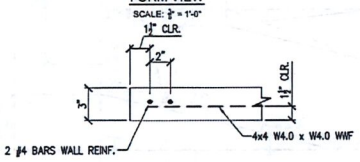
JOB NO: 16-57 SHEET: 37 OF: 42



LAYOUT - PANEL P1 (ALTERNATE 1)

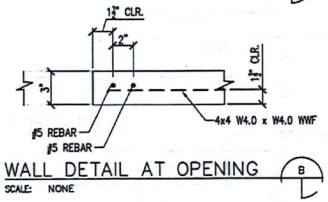
FORM VIEW

SCALE: ¾" = 1'-0"



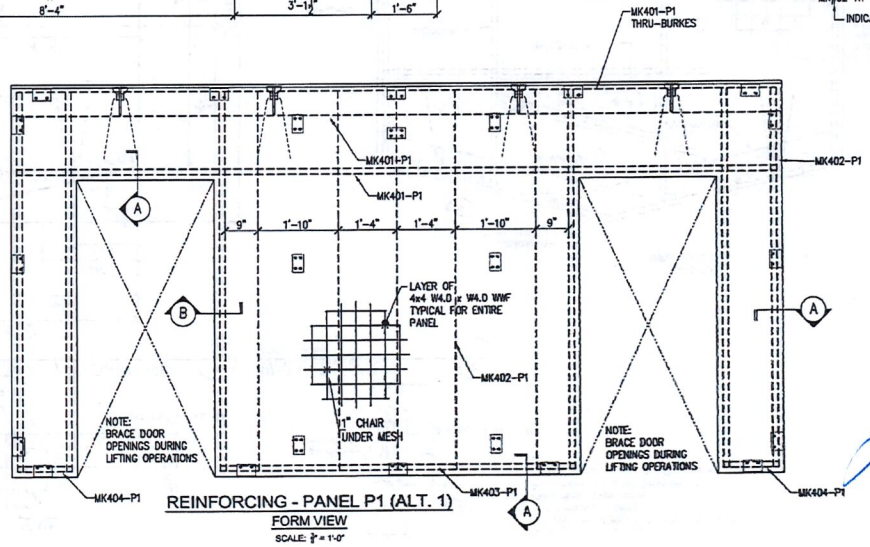
WALL DETAIL

SCALE: NONE



WALL DETAIL AT OPENING

SCALE: NONE



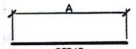
REINFORCING - PANEL P1 (ALT. 1)

FORM VIEW

SCALE: ¾" = 1'-0"

MARK	DESCRIPTION	QUANTITY
	MEADOW BURKE RL-3 GALV. w/ RL-31 SHEAR BAR AND RL-32 TENSION BAR	4
	PL1	6
	PL2	16
	CONCRETE VOLUME	1.04 CU YD
	PANEL WEIGHT	4,390 lb

Notes:
1. Orientation of plan is from the form view.
2. Trim plastic sheath at both ends of post tensioned cable to fit tight between anchor plates.
3. Note clearance on drawing from edge of panel to outside of cable.
4. Reinforcing sequence:
Place chairs under mesh
Place mesh and install Burke lift
Install cable. Hold cable at vertical center of the panel
5. Hold reinforcing bars 1 1/2 inch clear from edge of forms.



REBAR

MARK	QUANTITY	LENGTH "A"
MK401-P1	4	17'-4"
MK402-P1	17	8'-8"
MK403-P1	2	8'-1"
MK404-P1	4	1'-3"

MK#02-R1 — PANEL REF.
└ INDICATES REBAR SIZE (4=#4 ETC.)

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb
CU. YARD: SQ. FT: WEIGHT:

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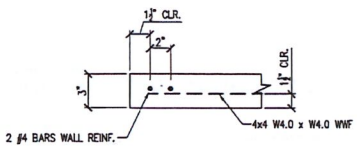
Orlando, FL 32836
(407) 810-3322 P.E. # 54071

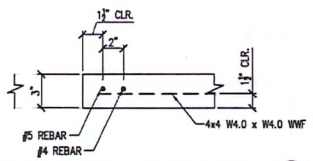
W. DAVID ARNOLD
ARCHITECT
106 PORTLAND STREET
EUSTIS, FL 32726
(352) 357-5807 AR0013558

LINDEN C. PARCHMENT III LICENSE No. 54071 STATE OF FLORIDA PROFESSIONAL ENGINEER

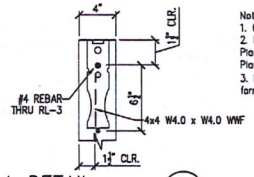
SCALE: AS SHOWN	DATE: 3/22/2017
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL 33157	
JOB NO: 16-57	SHEET: 38 OF: 42



WALL DETAIL (A)
SCALE: NONE

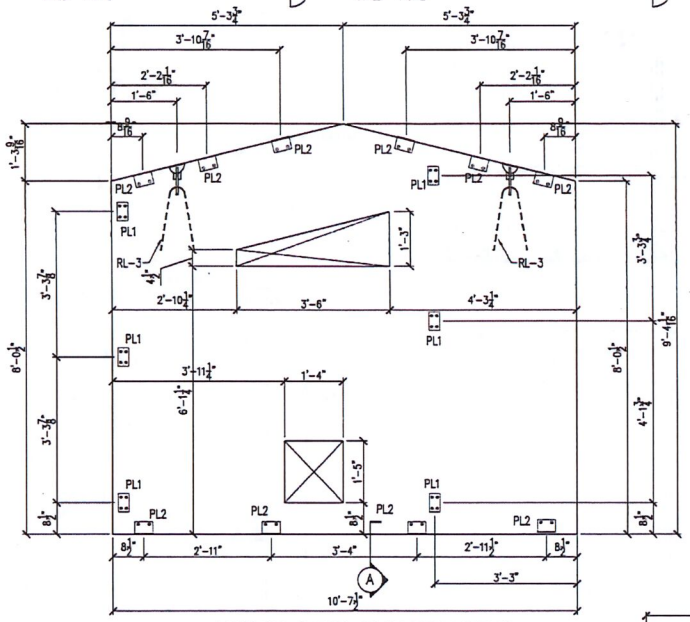


WALL DETAIL AT OPENING (B)
SCALE: NONE

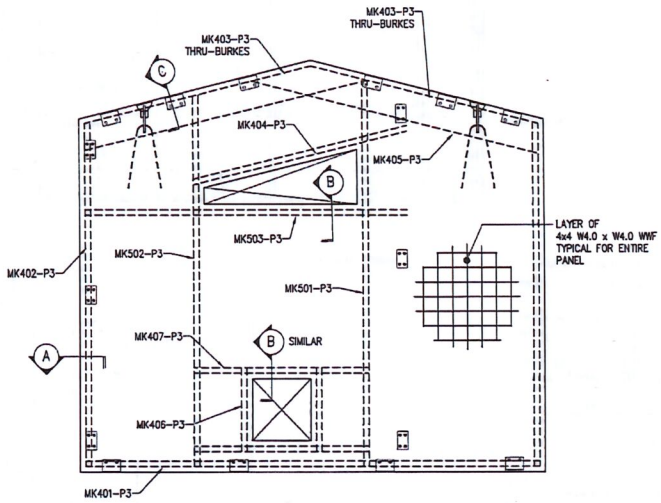


WALL DETAIL (C)
SCALE: NONE

Notes:
1. Orientation of plan is from the form view.
2. Reinforcing sequence:
Place chairs under mesh
Place mesh and install Burke lift
3. Hold reinforcing bars 1 1/2 inch clear from edge of forms.

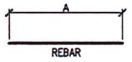


LAYOUT - PANEL P3 (ALTERNATE 1)
FORM VIEW
SCALE: ⅛" = 1'-0"



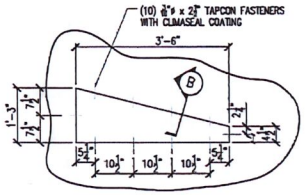
REINFORCING - PANEL P3 (ALTERNATE 1)
FORM VIEW
SCALE: ⅛" = 1'-0"

MARK	DESCRIPTION	QUANTITY
	MEADOW BURKE RL-3 GALV. w/ RL-31 SHEAR BAR AND RL-32 TENSION BAR	2
	PL1	6
	PL2	9
	CONCRETE VOLUME	0.83 CU YD
	PANEL WEIGHT	3,380 lb



MARK	QUANTITY	LENGTH "A"
MK401-P3	2	10'-4 ⅞"
MK402-P3	4	7'-9 ⅞"
MK403-P3	2	5'-4"
MK404-P3	2	5'-0"
MK405-P3	2	6'-7"
MK406-P3	4	2'-0"
MK407-P3	4	4'-0"
MK501-P3	2	8'-9"
MK502-P3	2	8'-5"
MK503-P3	2	7'-4"

MK#02-P1 —— PANEL REF.
└ INDICATES REBAR SIZE
(4=#4 ETC.)



WINDOW DETAIL
SCALE: 3/4" = 1'-0"

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-3 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb
CU. YARD: | SQ. FT: | WEIGHT:

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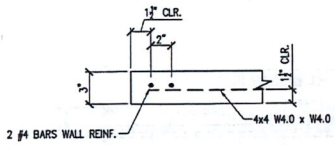
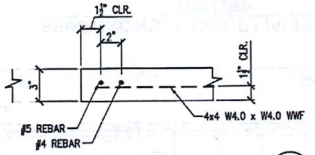
EUSTIS, FL. 32726
(352) 357-5807 AR0013558

LINDEN PARCHMENT LICENSE No. 54071 STATE OF FLORIDA PROFESSIONAL ENGINEER

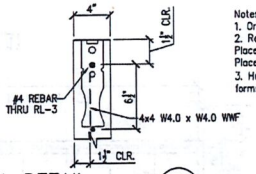
SCALE: AS SHOWN	DATE: 3/22/2017
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 39 OF: 42



WALL DETAIL (A)
SCALE: NONE

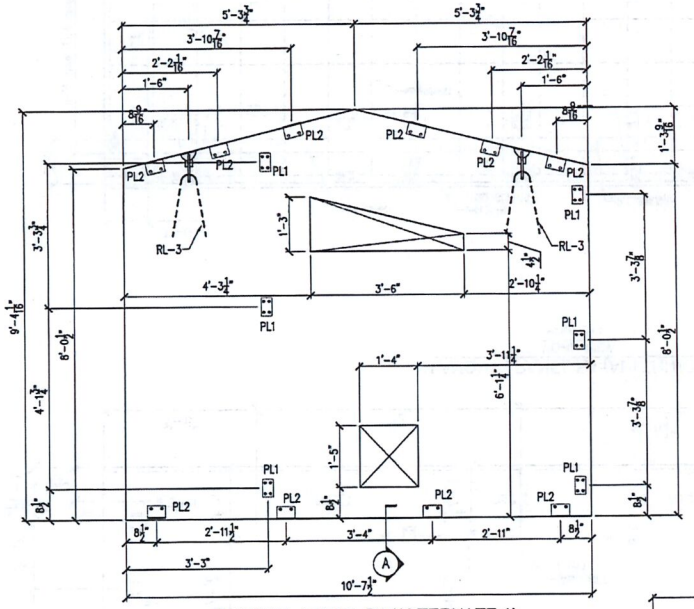


WALL DETAIL AT OPENING (B)
SCALE: NONE

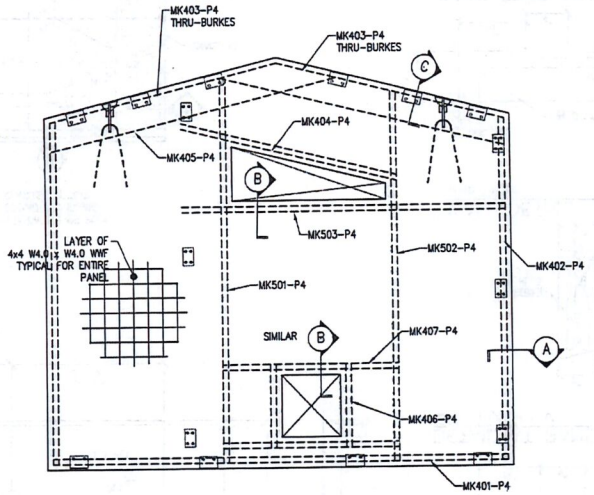


WALL DETAIL (C)
SCALE: NONE

Notes:
1. Orientation of plan is from the form view.
2. Reinforcing sequence:
Place chairs under mesh
Place mesh and install Burke lift
3. Hold reinforcing bars 1 1/2 inch clear from edge of forms.



LAYOUT - PANEL P4 (ALTERNATE 1)
FORM VIEW
SCALE: ¼" = 1'-0"



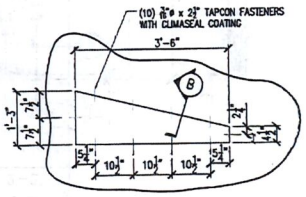
REINFORCING - PANEL P4 (ALTERNATE 1)
FORM VIEW
SCALE: ¼" = 1'-0"

MARK	DESCRIPTION	QUANTITY
	MEADOW BURKE RL-3 GALV. w/ RL-31 SHEAR BAR AND RL-32 TENSION BAR	2
	PL1	6
	PL2	9
	CONCRETE VOLUME	0.83 CU YD
	PANEL WEIGHT	3,380 lb

A
REBAR

MARK	QUANTITY	LENGTH "A"
MK401-P4	2	10'-4 ½"
MK402-P4	4	7'-9 ½"
MK403-P4	2	5-4"
MK404-P4	2	5'-0"
MK405-P4	2	6'-7"
MK406-P4	4	2'-0"
MK407-P4	4	4'-0"
MK501-P4	2	8'-9"
MK502-P4	2	8'-5"
MK503-P4	2	7'-4"

MK#02-P1 — PANEL REF.
└ INDICATES REBAR SIZE (4=#4 ETC.)



WINDOW DETAIL
SCALE: 3/4" = 1'-0"

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY
EXT FINISH:
INT FINISH:
MIX DESIGN: 5,000 PSI
RELEASE @ 3000 PSI
CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:
POST TENSION FORCE: 28,000 lb
CU. YARD: SQ. FT: WEIGHT:

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W. DAVID ARNOLD
ARCHITECT
106 PORTLAND STREET
EUSTIS, FL 32726
(352) 357-5807 AR0013558

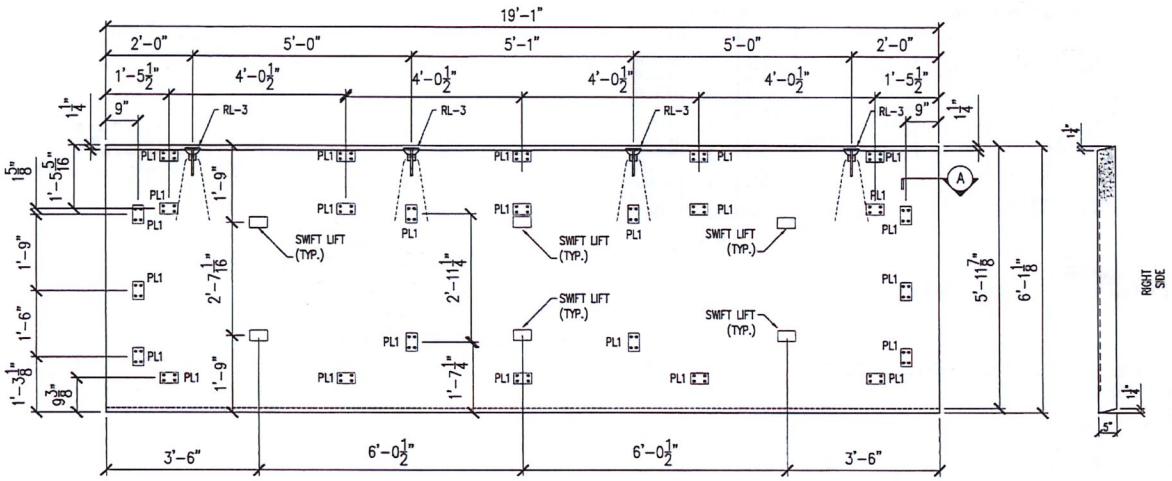
LINDEN C. PARCHMENT LICENSE No. 54071 STATE OF FLORIDA PROFESSIONAL ENGINEER

SCALE: AS SHOWN		DATE: 3/22/2017
DESIGN: WDA	DRAWN: JK/LP	CHECKED: WDA

CLIENT: LEESBURG CONCRETE

PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL
17800 OLD CUTLER RD.
PALMETTO BAY, FL 33157

JOB NO: 16-57 SHEET: 40 OF: 42

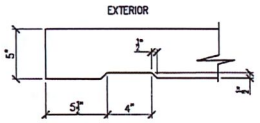


LAYOUT - PANEL R1 (ALTERNATE 1)

FORM VIEW

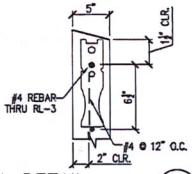
SCALE: ¾" = 1'-0"

MARK	DESCRIPTION	QUANTITY
	MEADOW BURKE RL-3 GALV. w/ RL-31 SHEAR BAR AND RL-32 TENSION BAR	4
	PL1	25
	PL2	
	PL3	
	SWIFT LIFT (2 TON)	6
	CONCRETE VOLUME	1.8 CU YD
	PANEL WEIGHT	7,550 lb



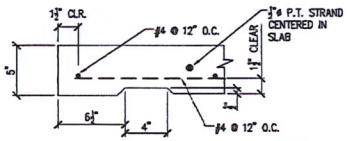
DETAIL AT PANEL EDGE (A)

SCALE: 1 1/2" = 1'-0"



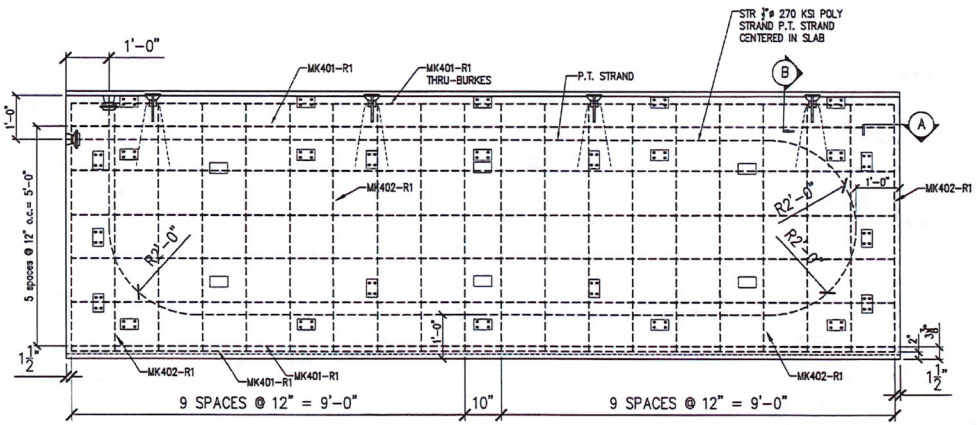
WALL DETAIL (B)

SCALE: NONE



ROOF EDGE DETAIL (C)

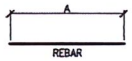
SCALE: NONE



REINFORCING - PANEL R1 (ALTERNATE 1)

FORM VIEW

SCALE: ¾" = 1'-0"



MARK	QUANTITY	LENGTH "A"
MK401-R1	8	18'-10"
MK402-R1	20	5'-7¾"

MK402-R1 — PANEL REF.

INDICATES REBAR SIZE (4=#4 ETC.)

POST TENSION CASTING ANCHOR AND WEDGE FOR 41'-7" LONG ¾"ø POLY STRAND TENDON. POST TENSION FORCE = 28,000 lbs

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY

EXT FINISH:

INT FINISH:

MIX DESIGN: 5,000 PSI

RELEASE @ 3000 PSI

CONC. STRENGTH FOR POST TENSIONING: 4,5000 psi CU.YD:

POST TENSION FORCE: 28,000 lb

CU. YARD:	SQ. FT:	WEIGHT:

Linden Parchment P.E.

8002 Glitter Court
Orlando, FL 32836

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AS SHOWN	3/22/2017

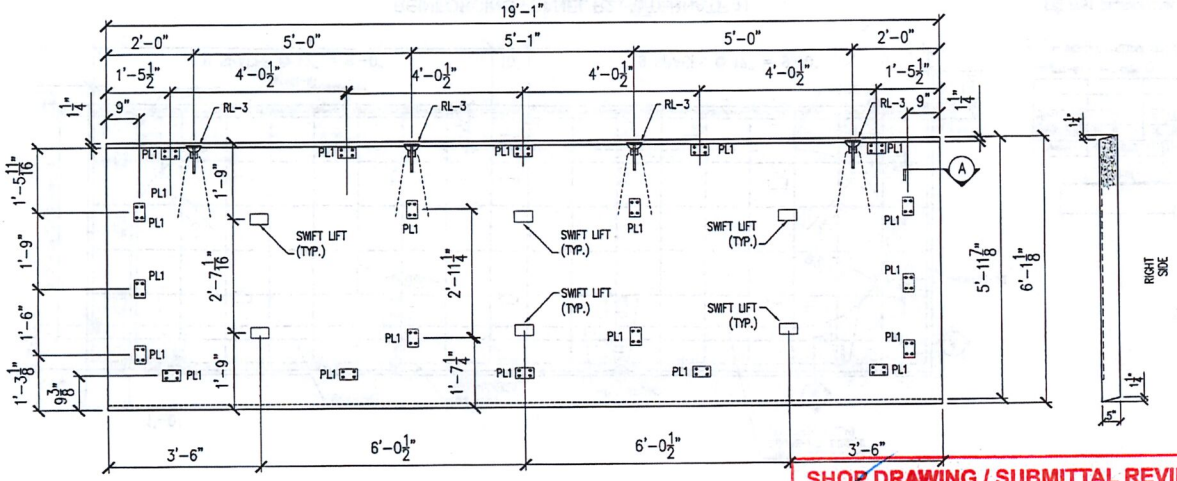
DESIGN:	DRAWN:	CHECKED:
WDA	JK/LP	WDA

CLIENT: LEESBURG CONCRETE

PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL
17800 OLD CUTLER RD.
PALMENTO BAY, FL. 33157

JOB NO: 16-57

SHEET: 41 OF: 42

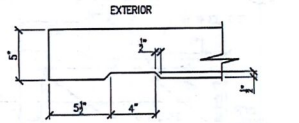


LAYOUT - PANEL R2 (ALTERNATE 1)

FORM VIEW

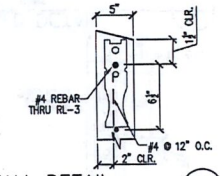
SCALE: ¾" = 1'-0"

MARK	DESCRIPTION	QUANTITY
	MEADOW BURKE RL-3 GALV. w/ RL-31 SHEAR BAR AND RL-32 TENSION BAR	4
	PL1	20
	PL2	
	PL3	
	SWIFT LIFT (2 TON)	6
	CONCRETE VOLUME	1.8 CU YD
	PANEL WEIGHT	7,524 lb



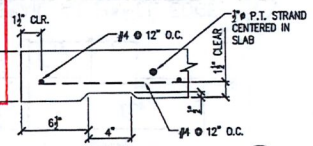
DETAIL AT PANEL EDGE (A)

SCALE: 1 1/2" = 1'-0"



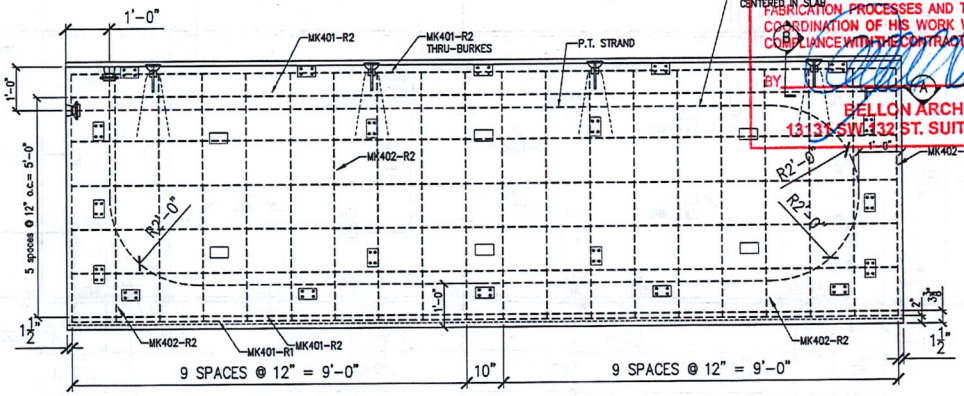
WALL DETAIL (B)

SCALE: NONE



ROOF EDGE DETAIL (C)

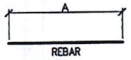
SCALE: NONE



REINFORCING - PANEL R2 (ALTERNATE 1)

FORM VIEW

SCALE: ¾" = 1'-0"



MARK	QUANTITY	LENGTH "A"
MK401-R2	8	18'-10"
MK402-R2	20	5'-7¾"

MK402-R1 — PANEL REF.

└ INDICATES REBAR SIZE (4=#4 ETC.)

POST TENSION CASTING ANCHOR AND WEDGE FOR 41'-7" LONG ¾"ø POLY STRAND TENDON. POST TENSION FORCE = 28,000 lbs

SHOP DRAWING / SUBMITTAL REVIEW

☑ APPROVED ☐ APPROVED WITH CHANGES NOTED

☐ REVISE AND RESUBMIT ☐ REJECTED:

SUBMITTAL WAS REVIEWED FOR DESIGN CONFORMITY AND GENERAL CONFORMANCE TO CONTRACT DOCUMENTS ONLY. THE CONTRACTOR IS RESPONSIBLE FOR CONFIRMING AND CORRELATING DIMENSIONS AND QUANTITIES FOR TOLERANCES, CLEARANCES, QUANTITIES, FABRICATION PROCESSES AND TECHNIQUES OF CONSTRUCTION, COORDINATION OF HIS WORK WITH OTHER TRADES AND FULL COMPLIANCE WITH THE CONTRACT DOCUMENTS.

BY: [illegible] DATE: 8-10-17

BELLON ARCHITECTURE, INC.
13131 SW 132 ST. SUITE 104, MIAMI, FL 33196

REVISIONS

ITEM	QTY	DESCRIPTION
		BURKE RL-2 W/ RL-32 & SHEAR BAR
		PL1
		PL2
		PL3
		BURKE SWIFT LIFT 2 TON CAPACITY
		LA-1 LIFT ANCHOR

CONCRETE: 5000 PSI @ 28 DAY

EXT FINISH:

INT FINISH:

MIX DESIGN: 5,000 PSI

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POST TENSION FORCE: 28,000 lb

CU. YARD: | SQ. FT: | WEIGHT:

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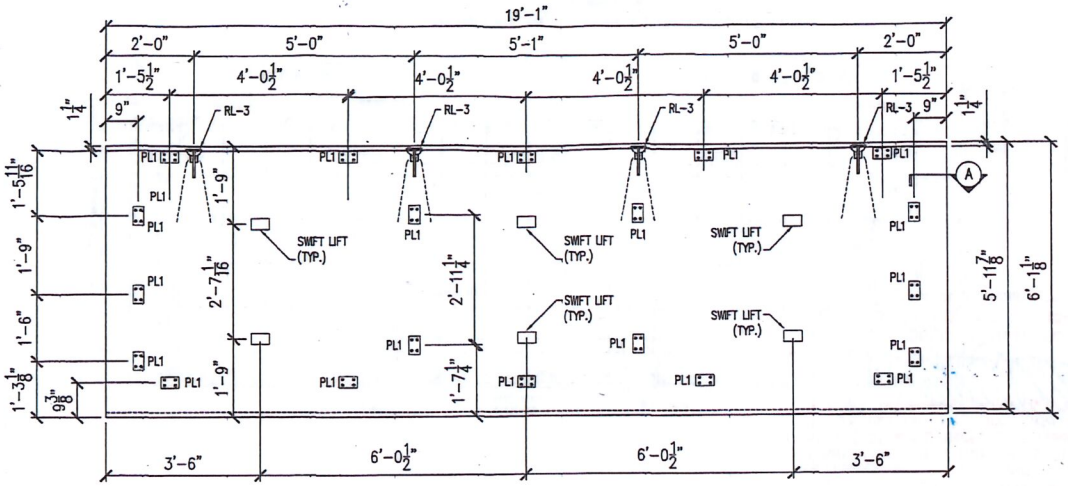
SCALE: AS SHOWN	DATE: 3/22/2017
DESIGN: WDA / DRAWN: JK/LP	CHECKED: WDA
CLIENT: LEESBURG CONCRETE	
PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL, 17800 OLD CUTLER RD., PALMETTO BAY, FL. 33157	
JOB NO: 16-57	SHEET: 42 OF: 42



LAYOUT - PANEL R2 (ALTERNATE 1)

FORM VIEW

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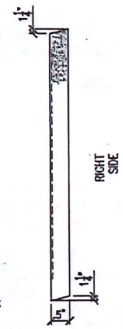

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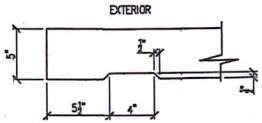

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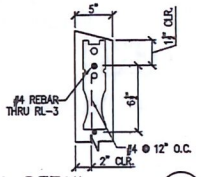
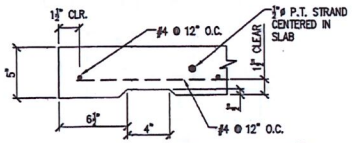


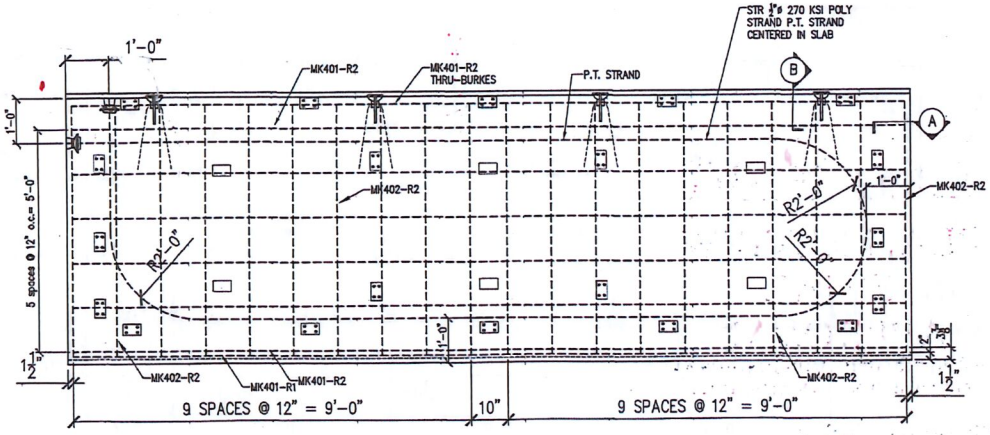
WALL DETAIL (B)

SCALE: NONE



ROOF EDGE DETAIL (C)

SCALE: NONE



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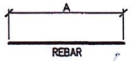

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LINDEN C. PARCHMENT LICENSE No. 54071 STATE OF FLORIDA PROFESSIONAL ENGINEER

SCALE: AS SHOWN		DATE: 10/6/16
DESIGN: WDA	DRAWN: JK/LP	CHECKED: WDA

CLIENT: LEESBURG CONCRETE

PROJECT: OUTBACK SIERRA RESTROOM for MONTESSORI SCHOOL 17800 OLD CUTLER RD. PALMENTO BAY, FL. 33157

JOB NO: 16-57 | SHEET: 42 | OF: 42

BLD2017-4965
14400 Old Cutler Rd

VILLAGE OF PALMETTO BAY				
THIS COPY OF PLAN MUST BE AVAILABLE ON BUILDING SITE OR NO INSPECTION WILL BE GIVEN				
	APPROVED		DISAPPROVED	
SECTION	BY	DATE	BY	DATE
ZONING		3/14/17		
P & Z				
LANDSCAPING				
PLOOD				
PUBLIC WORKS				
BUILDING	DMG	8/17/17	DMG	3/29/17
HANDICAP				
STRUCTURAL	MC	3/29/17	MC	3/14/17
ELECTRICAL		3/13/17		
MECHANICAL		3-10-17		
PLUMBING	RW	8-8-17		8-10-17
UTILITIES				
BUILDING OFFICIAL				

Subject to compliance with all Federal, State and County Law, rules and regulations. The Village of Palmetto Bay assumes no responsibility for accuracy of all results of these plans

NOTICE: In addition to the requirements of these permits there may be additional restrictions applicable to this in the public record of this county and village.

8/16/17 ok mc

8-11-17 Express Review Please

3/17/17 Plans Back in office 3pm sheet Separately

MIAMI-DADE FIRE RESCUE

Process No:
Municipal Insp No:
Project Name:
Address: 14400 Old Cutler Rd

INITIAL REVIEW

DATE	Approved	Not Applicable
4-27-17		
5-15-17		

Revision Process No:

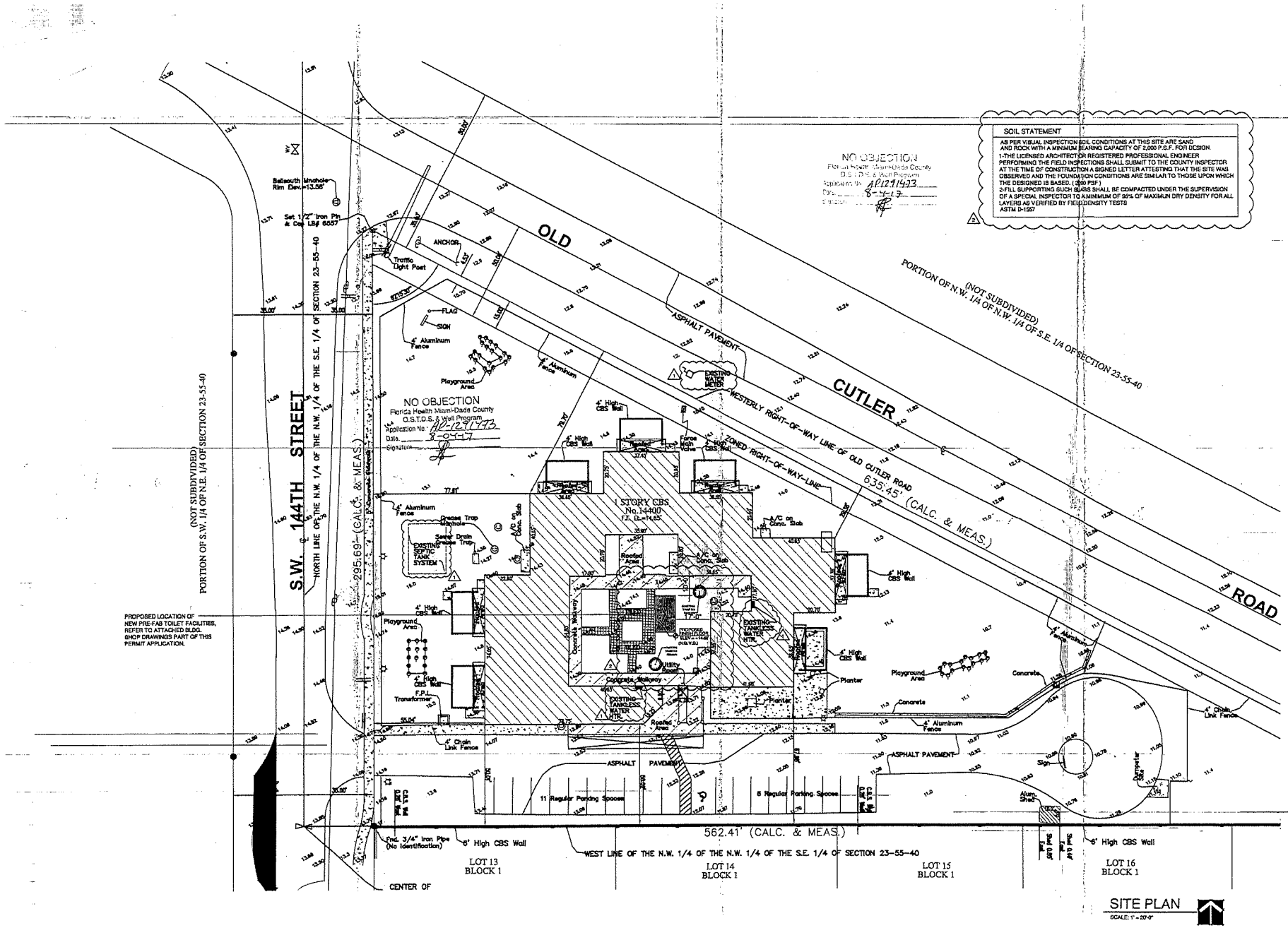
DATE	Approved	Disapp	Not Applicable

0000233097

M201700902I

DERM
PLAN REVIEW
FINAL
APPROVAL
M2017009021
DIVISION OF ENVIRONMENTAL RESOURCES MANAGEMENT

NAME (PRINT): DVS
SIGNATURE: DATE: 5/16/17



SITE PLAN
SCALE: 1" = 20'-0"

BELLON architecture
13131 S.W. 152 STREET
SUITE 104
MIAMI, FLORIDA 33186
T. (305) 276-7770
F. (305) 276-7473
WWW.BELLONWILANDS.COM
AA-26002616
ARCHITECTURE
LAND PLANNING
INTERIORS
CONSTRUCTION MANAGEMENT

CONSULTANTS

ALEXANDER MONTESSORI SCHOOL

ADDITION OF TOILET FACILITIES
17800 OLD CUTLER ROAD
PALMETTO BAY, FLORIDA

MARK	DATE	DESCRIPTION
1	3-20-17	
2	5-15-17	

PROJECT No. L2006
DRAWN BY: JP
CHECKED BY: LB
DATE: 09-20-16

STATE OF FLORIDA LEOPOLDO BELLON
LEOPOLDO BELLON, AIA (AR-6737)

SHEET TITLE
SITE PLAN

SP-1

SHEET OF